

NorthMet Project

Water Management Plan - Mine

Version 5 - Certified

Issue Date: July 11, 2016

This document was prepared for Poly Met Mining Inc. by Barr Engineering Co.



Date: July 11, 2016	NorthMet Project Water Management Plan - Mine
Version: 5	Certifications

I hereby certify that this report, with the exception of the sections listed below, was prepared by me or under my direct supervision and that I am a duly Licensed Professional Engineer under the laws of the state of Minnesota.

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Acronyms, Abbreviations and Units

Acronym	Stands For
ac-ft	acre-feet
AWMP	Adaptive Water Management Plan
ВМР	best management practice
cfs	cubic feet per second
CPS	Central Pumping Station
CRE	Contingency Reclamation Estimate
East EQ Basin	East Equalization Basin
fps	feet per second
FSP	Field Sampling Plan
FTB	Flotation Tailings Basin
gpm	gallons per minute
HDPE	high-density polyethylene
HRC	haul road central
HRE	haul road east
HRN	haul road north
HRW	haul road west
LCRS	leak collection and recovery system
LTVSMC	LTV Steel Mining Company
MDNR	Minnesota Department of Natural Resources
mg/L	milligram per liter
mi ²	square mile
mm	millimeter



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Acronym	Stands For
MPCA	Minnesota Pollution Control Agency
MSFMF	Mine Site Fueling and Maintenance Facility
MSHA	Mine Safety and Health Administration
N/A	not applicable
NPDES	National Pollutant Discharge Elimination System
No.	Number
NWL	normal water level
OSLA	Overburden Storage and Laydown Area
OSP	Ore Surge Pile
PRB	Permeable Reactive Barrier
PTM	Permit to Mine
MD-	Mine drainage (prefix)
QAPP	Quality Assurance Project Plan
RTH	Rail Transfer Hopper
S	Mine drainage sump (prefix)
SAP	Sampling and Analysis Plan
SCS	Soil Conservation Service
SDS	State Disposal System
SOP	Standard Operating Procedure
SPCC	Spill Prevention Control & Counter Measures
SWPPP	Storm Water Pollution Prevention Plan
TSS	total suspended solids



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Acronym	Stands For	
TWP	Treated Water Pipeline	
USGS	United States Geological Survey	
West EQ Basin	West Equalization Basin	
WWTF	Mine Site Waste Water Treatment Facility	
XP-SWMM	Software package used to model stormwater, sanitary water and river systems	



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1.0 Introduction

This document presents the Water Management Plan - Mine for Poly Met Mining Inc.'s (PolyMet) NorthMet Project (Project) and describes the management of mine drainage (note this was formerly referred to as "process water" throughout the environmental review process) and stormwater at the Mine Site. It includes design of mine drainage and stormwater infrastructure associated with the Project, estimated quantity of mine drainage to be pumped to the Flotation Tailings Basin (FTB), estimated water quality at appropriate water quality compliance points, operating plans, water quality and quantity monitoring plans, reporting requirements, and adaptive management approaches. Information from this report will become part of the Minnesota Department of Natural Resources (MDNR) Permit to Mine (PTM) application and Water Appropriation Permit application and Minnesota Pollution Control Agency (MPCA) National Pollutant Discharge Elimination System (NPDES) / State Disposal System (SDS) Permit application and is summarized in the NorthMet Project Mine Plan (Reference (1)). This and all other Management Plans will evolve through the environmental review, permitting, operating, reclamation, and long-term closure phases of the Project.

In addition to the management of water at the Mine Site, this document also briefly describes the quantity of water removed from the upper reaches of the Partridge River by the Project and the quantity of water that will be discharged from the Mine Site Waste Water Treatment Facility (WWTF) in long-term closure, as modeled in the Water Modeling Data Package Volume 1 – Mine Site (Reference (2)).

Several other Management Plans contain information that relates to the water management at the Mine Site. The NorthMet Project Rock and Overburden Management Plan (Reference (3)) includes design details for stockpile drainage containment/liner systems. The NorthMet Project Adaptive Water Management Plan (AWMP, Reference (4)) contains details of adaptive engineering controls (WWTF and Category 1 Waste Rock Stockpile cover) that will ensure compliance with applicable water quality standards at appropriate evaluation points.

The Project is described in the Project Description (Reference (5)). Detailed reclamation plans for the mine drainage and stormwater management systems are described in this document. The overall reclamation plan and cost estimate is described in the NorthMet Project Reclamation Plan (Reference (6)).

1.1 Objective and Overview

The objective of the Water Management Plan - Mine is to describe a safe and reliable system for managing the water at the Mine Site in a manner that results in compliance with applicable surface water and groundwater quality standards at appropriate Mine Site compliance points and with water appropriations/augmentation limits as demonstrated by modeling outcomes discussed in Reference (2).



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In order to manage the water at the Mine Site, an understanding of the overall Mine Plan is necessary. As described in Section 1.1 of Reference (1), ore will be mined from the East Pit from Mine Years 1 to 11 and from the West Pit from Mine Years 2 to 11. During that period, the more reactive waste rock will be placed in temporary stockpiles, and the least reactive waste rock will be placed in a permanent stockpile. Ore will be mined from the West and Central Pits from Mine Years 11 to 16. As mining of the Central Pit progresses, it will be joined to the East Pit, and the combined pits will be referred to as the East Pit. Ore will be mined only from the West Pit from Mine Years 17 to 20. Beginning in Mine Year 11, the more reactive waste rock mined from the West and Central Pits will be placed directly in the East Pit, after mining is completed in that pit. The waste rock in the temporary stockpiles will be relocated to the East Pit beginning in Mine Year 11. As the least reactive waste rock is mined, it will be placed in the permanent stockpile or the East Pit. As the East Pit is backfilled, water will be pumped to the pit to submerge the backfilled rock. By the end of operations (Mine Year 20), the East Pit will be backfilled with waste rock mined from the West and Central Pits, waste rock and overburden from the temporary stockpiles, and water, resulting in permanent subaqueous disposal of these materials.

1.2 Outline

The outline of this document is:

- Section 1.0 Introduction, objective and overview, and description of the Mine Site baseline data and existing conditions
- Section 2.0 Description of the design of the mine drainage management systems and stormwater management infrastructure at the Mine Site
- Section 3.0 Description of key outcomes of the design, including quantity of water treated and pumped to the FTB or East Pit and water quality at compliance points
- Section 4.0 Description of operational water management plans for mine drainage, stormwater, spills, and overflows
- Section 5.0 Overview of the approach for monitoring water quantity and quality The specifics of monitoring, including specific locations, nomenclature, frequency, and parameters will be finalized during the NPDES/SDS and Water Appropriation permitting processes and have been incorporated into each of those permit applications.
- Section 6.0 Description of monthly and annual reporting requirements including comparison to modeled outcomes and compliance, adaptive management plans, and available mitigations
- Section 7.0 Description of the reclamation and long-term closure plan for the Mine Site water management systems including the Contingency Reclamation Plan (assumes closure in the upcoming year) for Mine Years 0 and 1



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Because this document is intended to evolve through the environmental review and permitting (NPDES/SDS, Water Appropriations, and PTM) phases of the Project, a Revision History is included at the end of the document.

1.3 Baseline Data

Section 4 of Reference (2) describes the baseline climate, land use, geology, surface water, and groundwater data used in water quantity and quality modeling at the Mine Site. This section provides a summary of the baseline surface water and groundwater data from Reference (2).

1.3.1 Surface Water Baseline Data

As described in Section 4.4 of Reference (2), the Mine Site is located within the Partridge River watershed, approximately 17 river miles upstream of Colby Lake (Large Figure 1). Above Colby Lake, the Partridge River watershed covers approximately 103 square miles. Tributaries to the Partridge River above Colby Lake and downstream of the Mine Site and Transportation and Utility Corridors include an Unnamed Creek downstream of the future West Pit Overflow, Wetlegs Creek, Longnose Creek, and Wyman Creek. Colvin Creek and the south branch of the Partridge River are also tributaries to the Partridge River; however, these streams will not be directly or indirectly impacted by the Project.

Daily flow data is available for the Partridge River from the U.S. Geological Survey (USGS) gaging station 04015475 – Partridge River above Colby Lake at Hoyt Lakes, Minnesota, from water years 1978 through 1987. During this period, hydrology was affected by the periodic and variable dewatering of the Peter Mitchell Pits located at the headwaters of the Partridge River. The hydrology data has been validated and adjusted for use on this Project, as described in Reference (2).

Recent (2011-present) daily flow data near the Mine Site is available from MDNR gage H03155002, located on the Partridge River at the Dunka Road crossing (surface water monitoring location PM-3/SW003). This data is not directly comparable to the USGS gage 04015475 data due to the large difference in tributary watershed size and location. Based on its location, the MDNR gage H03155002 is more heavily influenced by Peter Mitchell Pit dewatering than the USGS gage 04015475.

Several locations within the Partridge River watershed have been monitored for water quality between 2004 and 2014. These locations are shown in Large Figure 1 and include seven monitoring locations on the Partridge River above Colby Lake, two locations along Wyman Creek, three locations along tributaries to the Partridge River, and four locations in Colby Lake and Whitewater Reservoir. The results of baseline monitoring of the Partridge River, upstream of Colby Lake, and select tributaries is presented in Large Table 10 of Reference (2). Baseline monitoring data from water collected in Colby Lake and Whitewater Reservoir is presented in Large Table 9 of Reference (2). The frequency and extent (i.e.,



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number of constituents) of monitoring varies by location. Monitoring conducted from 2004 through 2008 generally includes a wider list of constituents to characterize the baseline conditions within the watershed. Monitoring from 2008 through 2011 generally focused on a smaller list of constituents and locations to resolve specific issues (e.g., ratio of dissolved to total aluminum, inadequate thallium detection limits) with the data. More comprehensive baseline monitoring at select locations along the Partridge River and its tributaries was resumed in 2012 with a wider list of constituents.

1.3.2 Groundwater Baseline Data

As described in Sections 4.3 and 4.4 of Reference (2), baseflow in the Partridge River near the Mine Site can be considered a proxy for overall discharge through the surficial aquifer at the Mine Site because the river represents the primary sink for shallow groundwater flow. In the Mine Site area, the average 30-day low flow (considered a proxy for baseflow) in the Partridge River is estimated to be 3.8 cubic feet per second (cfs), corresponding to a contributing watershed area of approximately 95 square miles (mi²), which represents an estimated aquifer yield of 0.04 cfs/mi², or 0.55 inches per year.

Based on groundwater elevations at the Mine Site surficial aquifer monitoring wells (Reference (2)) and estimated Partridge River elevations downgradient of the wells, the average hydraulic gradient across the area is on the order of 0.01. Using the approximate geometric mean of the hydraulic conductivity estimates from slug tests completed at the Mine Site (0.3 feet/day; Reference (7)) and assuming a porosity of 0.3, a representative groundwater velocity in the unconsolidated aquifer at the Mine Site is approximately 0.01 feet/day. Locally, actual velocities likely range over several orders of magnitude, depending on the gradient and hydraulic conductivity of the aquifer material present.

As described in Section 4.3 of Reference (2), the Mine Site contains 33 monitoring wells, including:

- 24 wells located in the surficial deposits (identified on Large Figure 2 with the prefix "MW")
- 5 wells within the upper 100 feet of bedrock (identified on Large Figure 2 with the prefix "OB")
- 4 wells in bedrock at depths ranging from 485 to 610 feet below grade (identified on Large Figure 2 with the prefix "P")

The locations of these wells are shown on Large Figure 2. Three of the monitoring wells in the surficial deposits were installed in 2005 and have been sampled intermittently since installation. The additional 21 wells in the surficial deposits were installed between October 2011 and February 2012. A monthly groundwater sampling program of these surficial monitoring wells was initiated in November 2011 and continued through August 2012. A



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quarterly (excluding first quarter) sampling program was initiated in October 2012 and has continued through October 2014. The five monitoring wells within the upper 100 feet of the bedrock have each been sampled nine or ten times since installation in 2006. The four larger diameter deep bedrock wells were installed in 2006 and have been sampled during aquifer testing in 2006 and 2007. Groundwater monitoring data from the monitoring wells in the surficial deposits and bedrock wells is summarized in Large Table 3 through Large Table 6 in Reference (2).

1.4 Existing Conditions

Existing subwatersheds at and near the Mine Site are shown on Large Figure 3. Under existing conditions, runoff from the northernmost area of the Mine Site generally drains north into the One Hundred Mile Swamp and associated wetlands along the Partridge River. These wetlands form the headwaters of the Partridge River, which meanders around the east end of the Mine Site before turning southwest. Runoff from the majority of the Mine Site naturally drains to the south through culverts under Dunka Road and the adjacent rail line, into the Partridge River downstream of the Dunka Road crossing.

In addition to subwatershed boundaries, Large Figure 3 shows the 100-year flood levels and average water levels at selected locations along the Partridge River. The flood boundary was developed for the 24-hour storm event, which was the critical event for the Partridge River. The 100-year, 10-day snowmelt event was previously modeled to evaluate the peak flows in the Partridge River, but the 24-hour storm event produced higher flows and flood levels due to the quick runoff delivery in the upper watershed.

As shown by these flood levels, the Partridge River is very flat in the upstream reach in the vicinity of the One Hundred Mile Swamp; however, there is still an increase of over 10 feet in normal and flood water levels through the wetland from the east end of the Mine Site to the west end. Between the headwaters and Dunka Road, the Partridge River has a maximum slope of approximately 0.6%. The flood levels downstream of Dunka Road are more than 20 feet lower than most of the adjacent Mine Site perimeter ground elevations. There is very little risk from Partridge River flooding on the east and south sides of the Mine Site.

The increase in flood elevation from the 100-year event to the 500-year event on the Partridge River is relatively minor, varying from 0.1 to 0.5 feet on the north and east sides of the Mine Site to 1.0 foot upstream of the railroad crossing in the southeast corner of the Mine Site.



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2.0 Mine Drainage and Stormwater Management Systems Design

The water at the Mine Site will be managed by keeping the stormwater separate from the mine drainage through a system of ditches, dikes, and ponds. Each of these terms is defined specifically for this Project, as follows:

- Stormwater is the result of precipitation and runoff that contact natural or reclaimed surfaces, including reclaimed stockpiles, and surface runoff that has not been exposed to mining activities. Stormwater is expected to meet water quality standards after sedimentation ponds remove total suspended solids (TSS) prior to being discharged off-site.
- Mine drainage includes precipitation, runoff, and collected groundwater (pit
 dewatering water) that has contacted surfaces disturbed by mining activities, such as
 drainage collected on stockpile liners and runoff contacting exposed ore and waste
 rock and Mine Site haul road surfaces. Due to commitments made during the
 environmental review process, runoff from the Overburden Storage and Laydown
 Area (OSLA) will also be managed as mine drainage.

Construction water will be managed as either mine drainage or stormwater depending on its anticipated water quality. Once operations begin in Mine Year 1, the following guidelines will apply:

- Runoff from construction areas with no excavation will be managed as construction stormwater;
- Runoff from construction areas where the majority of the material being excavated is Unsaturated Overburden or Peat will be managed as construction stormwater; and
- Runoff and groundwater from construction areas of mainly Saturated Overburden (i.e., dewatering) or exposed ore will be managed as mine drainage.

The mine drainage system including sumps, ponds, and the piping network for Mine Years 1, 11, and 20 are shown on Large Figure 4 to Large Figure 6. The stormwater system including dikes, ditches, culverts, and sedimentation ponds for Mine Years 1, 11, and 20 are shown on Large Figure 7 to Large Figure 9.



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2.1 Mine Drainage

Mine drainage includes runoff¹ and groundwater that has contacted surfaces disturbed by mining activities such as active stockpiles, water from the Category 1 Waste Rock Stockpile Groundwater Containment System, and pit dewatering. This water may not meet water quality discharge limits for metals or other constituents and as a result, may require treatment at the WWTF prior to being pumped through the Treated Water Pipeline (TWP) to the FTB for use as plant make-up water or for East Pit flooding in later years. Mine drainage will be intercepted by ditches, dikes, and stockpile foundation liners/containment system to keep it separate from the stormwater collection and conveyance systems. Design drawings and flow diagrams of the mechanical infrastructure, which includes the TWP, Central Pumping Station (CPS), and mine drainage systems, are provided in Attachment A.

Drawing ME-003 of Attachment A provides a flow diagram of the mine drainage collection and conveyance system from each source to the FTB at the Plant Site or the East Pit. Mine drainage sources include mine pits, waste rock and ore stockpiles, the OSLA, and other mine infrastructure such as haul roads, the Rail Transfer Hopper (RTH), and the Mine Site Fueling and Maintenance Facility (MSFMF).

There are three types of stockpiles that will generate mine drainage:

- overburden stockpiles in the OSLA
- waste rock stockpiles (Category 1, 2/3, and 4)
- the Ore Surge Pile (OSP)

Precipitation coming in contact with each of these stockpiles will be managed as mine drainage until the stockpiles are reclaimed. Runoff from the OSLA will be considered mine drainage due to the concern regarding Peat drainage potentially containing elevated levels of mercury. As described in Section 5.2 of Reference (1), the Category 1 Waste Rock Stockpile is the only permanent stockpile and will be reclaimed. Once reclaimed, surface runoff from the Category 1 Waste Rock Stockpile will be managed as stormwater. The Category 2/3 and 4 waste rock stockpiles are temporary, and the footprints will be reclaimed after the material is relocated to the mined-out East Pit for subaqueous disposal and the liner systems are removed. The ore in the OSP will be removed by the end of Mine Year 20, the liner will be removed, and the footprint will be reclaimed.

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¹Runoff is defined in this document as the total volume of stormwater or mine drainage that collects above ground. According to this definition, runoff from active stockpiles is mine drainage and runoff from reclaimed stockpiles is stormwater. Runoff from active stockpiles includes the total yield from surface runoff, liner drainage, and leakage through the liner. Runoff from reclaimed stockpiles includes flows from the top of the cover and interflow that infiltrates into the cover and exits the stockpile without contacting the waste rock.



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As described in Section 3.1 of Reference (4), incremental reclamation of the Category 1 Waste Rock Stockpile is planned beginning in Mine Year 14. The timing of cover placement will have a large impact on the water flows. The total flow from the reclaimed stockpile will include:

- Infiltration through the cover that drains through the waste rock and is stored in the stockpile. This mine drainage will not be seen in any collection system.
- Infiltration through the cover that drains through the waste rock and is collected by the groundwater containment system and routed to the WWTF. Design of the groundwater containment system is provided in Section 2.1.2 of Reference (3).
- Infiltration through the cover that drains through the waste rock, bypasses the containment system, and flows via groundwater to the pits for collection as mine drainage during operations or to the West Pit lake or East Pit wetland during reclamation and long-term closure. Modeling and capture efficiency of the groundwater containment system is provided in Section 2.1.2.2 and 2.1.2.3 of Reference (3).
- Infiltration through the cover that drains through the waste rock, bypasses the groundwater containment system, and is not captured in the groundwater containment system or the pits. Modeling and capture efficiency of the groundwater containment system is provided in Section 2.1.2.2 and 2.1.2.3 of Reference (3).
- Surface runoff from the stockpile cover (stormwater) that will be collected by the stormwater ditch surrounding the stockpile and routed through sedimentation ponds prior to off-site discharge or routed to the West Pit lake during reclamation.

2.1.1 Design Criteria for the Mine Drainage Systems

Design criteria for the mine drainage design features are provided in Table 2-1 with preliminary sizing of the components listed on Drawing ME-004 of Attachment A. Mine drainage system components at the Mine Site have been designed to route mine drainage by gravity flow to sumps or mine drainage ponds that are designed to contain water from a component-specific "design event". The design event chosen for each component was based on the expected quality of water handled by the component and the overflow potential of the component. This allows matching the level of protection applied to the component to the expected water quality handled by the component and the potential for overflows by choosing larger design events as necessary. Water from the sumps and mine drainage ponds will be pumped to the WWTF, if needed, and then to the CPS pond. The CPS will pump water from the pond through the TWP to the FTB or the East Pit during pit flooding.



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The following sections describe the design of the major components of the Mine Drainage System, which includes the collection and conveyance of water from the pits, the waste rock stockpiles, the OSLA, the OSP, and applicable construction areas.

Table 2-1 Design Criteria for Mine Drainage Infrastructure

Infrastructure Draining	Mine Drainage Structure Name ⁽¹⁾	Design Event	Overflow Pond Design Event	
Category 1 Waste Rock Stockpile	Groundwater Containment System (Section 2.1.2 of Reference (3))	100-year, 24- hour ⁽²⁾	Not applicable (N/A); overflow to mine pits	
Category 2/3 Waste Rock Stockpile			100-year, 24-hour less sump capacity ⁽²⁾	
		Sump: 10-year, 24-hour ²	100-year, 24-hour less sump capacity ⁽²⁾	
Ore Surge Pile	SOSP; MD-SOSP	Sump: 10-year, 24-hour ⁽²⁾	100-year, 24-hour less sump capacity ⁽²⁾	
Rail Transfer Hopper	MD-RTH	Pond: 100-year, 24-hour ^{(2), (3)}	N/A	
Haul Roads	MD-HRE, MD-HRN, MD-HRC, MD-HRW	Pond: 100-year, 24-hour ⁽²⁾	N/A	
Overburden Storage and Laydown Area	MD-OSLA	25-year, 24- hour ⁽²⁾	N/A	
Pit Pumps and Pipes	Varies	Annual snowmelt event (removal within 3 days)	N/A	
Other Pumps / Pipes	Varies	Annual snowmelt event (removal within 30 days)	N/A	

⁽¹⁾ Mine drainage sumps are named with the prefix S followed by an abbreviation of the infrastructure the drainage is coming from. Mine drainage ponds are named with the prefix MD followed by an abbreviation of the infrastructure the drainage is coming from.

2.1.2 Mine Site Water Balance

The details of the Mine Site water balance can be found in Section 6.1 of Reference (2). The details include quantification and breakdown of stormwater, groundwater, and mine drainage, including the water balance associated with the stockpiles.

⁽²⁾ Mine drainage sumps and ponds include a safety factor in the form of freeboard (typically three feet) in addition to the design storm volume.

⁽³⁾ MD-RTH was sized based on available area with a larger pump capacity to meet the design storm volume.



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2.1.3 Pit Dewatering

The estimated average annual inflow rates and peak inflow rates for the three pits were modeled as part of Reference (2) and are listed in Table 2-2. Mine pit inflows will be directed to sumps within the pits where the water can be collected and pumped to the WWTF. The mine pit pump capacities have been designed to minimize delay to mining operations during the typical spring snowmelt event.

Table 2-2 Mine Pit Inflows

		Mine Year 1 Inflows ⁽¹⁾ Average Annual (gallons per minute [gpm]) 90th Percentile (gpm)		Mine Year 11 Inflows ^{(1),(2)}		Mine Year 20 Inflows ⁽¹⁾	
Mine Pit	Inflow Component			Average Annual (gpm)	90th Percentile (gpm)	Average Annual (gpm)	90th Percentile (gpm)
	Groundwater ⁽³⁾	Not Applicable		81	104	44	58
West Pit	Runoff			224	278	236	298
	Total ⁽⁴⁾			303	367	280	346
	Groundwater	Not Applicable		30	40	4.9	6.4
Centra I Pit	Runoff			7.2	8.9	68	81
	Total ⁽⁴⁾			37	47	73	86
_	Groundwater(3),(5)	101 134		738	977	161	210
East Pit	Runoff	114	114 144		153	217	258
- "	Total ⁽⁴⁾	205 252		863	1,096	378	448

- (1) Source of data: Section 6.1 of Reference (2)
- (2) The Central Pit exists for only a portion of Mine Year 11; the values shown are for the latter third of the year when the pit is operational. The East Pit begins to be backfilled in Mine Year 11, but backfilling does not significantly change the natural inflows to the pit; the values shown are for the entire year.
- (3) Groundwater flows to the West and East Pits include seepage from the Category 1 Waste Rock Stockpile. See Section 2.1.2 of Reference (3).
- (4) Groundwater and runoff values do not sum to totals due to probabilistic model (i.e., high groundwater and high runoff conditions do not necessarily occur simultaneously).
- (5) East Pit groundwater inflows are significantly higher than the West and Central Pit inflows due to its proximity to the Virginia Formation. The hydraulic conductivity of the Virginia Formation is almost 3 orders of magnitude higher than the Duluth Complex. The East Pit intersects the Virginia Formation, and the West and Central Pits do not.

Water management within the pit will occur as part of mine development, with the pit floors sloped toward collection sumps. The sumps will be excavated as part of mine operations. Pumps in the sumps will either be submersible pumps or pumps on a raft floating in the sump. These pumping systems could include one single large pump or several smaller pumps, depending on an optimization analysis. Hoses will connect the pumps to pipes which



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may connect to additional pumps at the rim of the pits conveying the water to the WWTF. The alignment of the pit dewatering system is based on the future pit development, thus minimizing the need to frequently move the pipes. In locations where a pipe will intersect a road, the pipes will be placed inside a culvert or a larger pipe buried under the road. Hoses may be used in some places, where design allows, providing operational flexibility and simplicity.

Inflows to the pits include contributions from groundwater and runoff within the pit. The size and location of the sumps and pumps will change as the pits expand in size and depth, requiring periodic evaluation of the pumping system. Pump capacities are based on peak annual flows from the snowmelt event, assuming a rapid spring snowmelt (40% of the snowmelt occurring within one day). The pumping systems are designed to handle groundwater inflows and the average annual runoff volumes from a snowmelt event, removing approximately 100% of the groundwater inflows and 40% of the annual snowmelt runoff (1.28 inches) within 3 days; the volume from this snowmelt event is approximately equivalent to the runoff volume expected in the pits during the 5-year, 24-hour storm event. The sumps are designed with capacity to hold the remaining volume from this snowmelt runoff event.

In the event that a storm exceeds the sump and pump capacity, the lowest level of the pit will be used to store the excess water, with mining operations relocated to higher levels or delayed until water levels are pumped down. During extreme storm events, pit dewatering may temporarily be stopped to allow the WWTF to handle the increased volumes from other mine drainage sources to minimize overflow of mine drainage sumps and ponds across the Mine Site.

The pipes associated with these pumps are sized to maintain average velocities less than 5 feet per second to minimize friction losses and surge pressures (i.e., water hammer) in the pipes. The pump sizes were evaluated for each Mine Year, because, as the pits deepen, larger pumps will be needed to overcome the change in static head.

The number and size of pumps will be evaluated on a regular basis due to changes in head, pumping distances, and availability of electrical power sources.

The preliminary pit sump, pump, and pipe sizes for pit dewatering are listed on Drawing ME-004 of Attachment A. Pipe configurations for pit dewatering are shown on Drawings MD-001, MD-002, and ME-003 of Attachment A for Mine Years 1 and 11.

2.1.4 Stockpile Drainage

The design of the stockpile liner and underdrain system for the Category 2/3 and Category 4 waste rock stockpiles and the OSP, and the design of the Category 1 Waste Rock Stockpile Groundwater Containment System are described in Section 2.1 of Reference (3). This section discusses the evaluation of leakage through the liners, the collection of water on top of the



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liners, and the routing of the mine drainage away from the temporary stockpiles, and the containment system for collection of drainage from the Category 1 Waste Rock Stockpile.

Table 2-3 presents the range of total annual mine drainage volumes and flow rates estimated from the temporary stockpiles based on modeling results. These annual volumes assume that mine drainage from the stockpiles will begin within the first year and that all mine drainage is conveyed to the sumps.

Table 2-3 Temporary Stockpile Drainage

	Mine Year 1 ⁽¹⁾		Mine Ye	Mine Year 11 ^{(1),(2)}		Mine Year 20 ^{(1),(3)}	
Stockpile	Average Annual Inflow (gpm)	90th Percentile Inflow (gpm)	Average Annual Inflow (gpm)	90th Percentile Inflow (gpm)	Average Annual Inflow (gpm)	90th Percentile Inflow (gpm)	
Category 2/3 Waste Rock	44	53	120	140	9.9	12	
Category 4 Waste Rock	20	24	34	41	Not Ap	plicable	
Ore Surge Pile	20	24	20	24	20	24	

⁽¹⁾ Source of data: Section 6.1 of Reference (2)

2.1.4.1 Temporary Stockpile Drainage Collection Systems

As described in Section 2.1.3 of Reference (3), the temporary stockpiles, which include the Category 2/3 and 4 waste rock stockpiles and the OSP, have drainage systems with underdrains in the foundation that will flow by gravity to underdrain sumps in addition to the stockpile liner drainage systems that will flow by gravity to mine drainage sumps and overflow ponds. The water will be pumped from the mine drainage sumps to the WWTF for treatment before being sent to the CPS to be pumped through the TWP to the FTB or to the East Pit for pit flooding. This section describes the design of the stockpile sumps and the overflow ponds that collect the water from the temporary stockpile liner system. See Section 2.1.3 of Reference (3) for design of the foundation underdrain sumps.

2.1.4.1.1 Temporary Stockpile Overliner Sump and Overflow Pond Design

Mine drainage sumps will be located along the perimeter of the temporary stockpiles to collect overliner runoff, as shown in Large Figure 4 to Large Figure 6. The number of mine drainage sumps associated with each stockpile depends on the stockpile foundation design (Section 2.1.3 of Reference (3)), as follows:

⁽²⁾ The Category 4 Waste Rock Stockpile exists through the first half of Mine Year 11; the stockpile is removed in the latter half of the year.

⁽³⁾ All mass is removed from the Category 2/3 Waste Rock Stockpile by the end of Mine Year 19. The Mine Year 20 values represent the water collected on the liner as it is being removed and the stockpile is being reclaimed.



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- The Category 2/3 Waste Rock Stockpile will have 3 sumps, S23-1, S23-2, and S23-3, located on the south side of the stockpile, between the stockpile and Dunka Road. Overflow mine drainage ponds include MD-S23-1, which provides overflow capacity for S23-1 and S23-2, and MD-S23-3, which provides overflow capacity for S23-3.
- The Category 4 Waste Rock Stockpile will have one sump, S4, located on the south side of the stockpile, with one overflow pond MD-S4.
- The OSP will have one sump, SOSP, located on the southwest side of the OSP, with one overflow pond MD-SOSP.

The sumps will be designed to contain mine drainage from active stockpiles during a 10-year, 24-hour rainfall event with the flood level below the stockpile liner discharge pipe elevation. To minimize uncontrolled overflows from the sumps, the volume generated by the 100-year 24-hour storm event in excess of the sump capacity will flow by gravity to overflow ponds adjacent to each sump. Dikes will be constructed around the perimeter of each sump and pond with a combined capacity for the 100-year, 24-hour mine drainage yield plus a safety factor in the form of freeboard (typically three feet). Further discussion of overflows is included in Section 4.4. Preliminary sump and pond footprints for the temporary stockpiles are listed in Table 2-4.

The temporary stockpile mine drainage sumps will be constructed with a double composite liner system consisting of an upper high-density polyethylene (HDPE) primary liner underlain by a geonet leak collection and recovery system (LCRS) which is underlain by a secondary HDPE liner that overlies a one-foot thick soil liner as shown in Detail 2 of Drawing MD-014 of Attachment A, or a composite liner (without LCRS) but using leak location liner for equivalent protection. Overflow ponds will be constructed with a single liner system overlying a one-foot thick soil liner as shown on Detail 1 of Drawing MD-014 of Attachment A. Temporary stockpile mine drainage sumps and ponds are designed with an average depth between 6 and 12 feet depending on the depth to bedrock, depth to groundwater, and stockpile outlet pipe elevation. Drawings MD-003 to MD-007 of Attachment A show the layout of each of these sumps and associated overflow ponds.

The sump and pond dikes and slopes will be vegetated or riprapped to limit erosion. The design will be finalized once the foundation grading design is completed, and sump and pond elevations can be established. This will be dependent on site-specific investigations of depth to bedrock and depth to groundwater. The design elevations will allow runoff from the temporary stockpiles to be conveyed by gravity into the sumps with gravity overflow into the overflow ponds. The outlet for both the sumps and ponds will be a pump and piping system to convey this mine drainage to the WWTF.



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Table 2-4 Stockpile Sump and Pond Dimensions – Approximate

Stockpile	Sump/Pond Name	Area (acres)	Required Capacity (acre-feet)	Design Volume (acre-feet) ⁽¹⁾
	S23-1	2.4	14.9	14.9
	S23-2	2.5	11.9	12.0
Category 2/3 Waste Rock	S23-3	1.5	6.6	6.6
Tradio Trodic	MD-S23-1	4.3	21.4	21.5
	MD-S23-3	1.5	5.3	5.3
Category 4 Waste	S4	2.5	10.1	12.5
Rock	MD-S4	2.0	8.1	9.9
Ora Curgo Dila	SOSP	2.1	8.5	8.7
Ore Surge Pile	MD-SOSP	1.6	4.8	5.3

⁽¹⁾ The design volume does not account for the freeboard (typically three feet) planned as part of the design.

2.1.4.1.2 Ore Surge Pile (OSP) Sump and Sump Liner

The temporary OSP is different from the temporary waste rock stockpiles because it will likely have periods with very little material on the liner throughout the mine operations. Due to the potential for small quantities of material to be on the liner of the OSP, the sump SOSP has been designed with more overall capacity than the temporary waste rock stockpile sumps. This was achieved by increasing the yield coefficients used in sizing the sumps to 100% of precipitation for the OSP in order to reflect the potential for these periods of small quantities of cover material, which will increase the quantity and timing of runoff within the footprint. This sump was designed to contain the entire precipitation volume from an open liner during the 10-year 24-hour event. The combined capacity of the sump and overflow pond MD-SOSP will contain the 100-year 24-hour precipitation volume.

2.1.4.1.3 Construction of Lined Sumps and Ponds

In general, sumps and overflow ponds will be excavated below the natural ground, designed to optimize the pond bottom with the expected groundwater and bedrock while draining the stockpile liners by gravity. Construction of a lined sump or pond requires adequate foundation drainage to prevent excessive pore pressure from developing under the liner. Due to the high groundwater and high bedrock outcrops in this area and low overliner discharge pipes from the stockpiles, the lined sumps and overflow ponds may have to be designed with the pond bottom below the groundwater level. Additional geotechnical and hydrologic investigation is needed to determine the actual depth of groundwater and bedrock in these locations prior to construction level design. These investigations will be done after the Project has completed environmental review. If the sumps and ponds must be constructed with the pond bottom below the



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groundwater level, the following options will be evaluated to prevent excessive pore pressures from building up below the liners:

- The stockpile underdrain sumps could be extended below the sump and pond bottom to allow for pumping to maintain dry foundations. The advantage of this is that it would minimize the number of pumps on-site; however, the disadvantage is that it would increase the amount of water pumped and managed.
- A separate underdrain system could be installed below the sump and pond bottom to allow for pumping to maintain dry foundations. The advantage of this is that it is separate from the stockpile underdrain system and could potentially be discharged off-site (clean groundwater); however, this would increase the number of pumps required, increasing capital and maintenance costs.
- A clay liner could be used instead of the geomembrane liners. The advantage of this would be that an underdrain system with a separate pump and piping system would not be needed below the sumps and ponds; however, use of a clay liner would increase the amount of water pumped due to increased leakage rates into the sump and pond to maintain inward drainage to prevent leakage out of the sumps and ponds to groundwater. The disadvantage of this is that it would increase the amount of mine drainage pumped from the sumps and ponds.
- The ballast, or weight on top of the liner, in the sump and pond could be increased to counteract the buoyancy forces of groundwater. The advantage of this option is that there would be no additional pumping or piping systems required and no extra water to manage and treat. However, the ballast used to hold down the liner would reduce the capacity of the sumps/ponds, so increased volumes and potentially larger sump and pond footprints would be required.

These options will be evaluated after the additional geotechnical and hydrologic investigation are performed.

2.1.4.2 Category 1 Waste Rock Stockpile Groundwater Containment System

A groundwater containment system will be constructed to capture stockpile drainage from below the Category 1 Waste Rock Stockpile and convey this water to sumps for collection and pumping to the WWTF. Drainage through the stockpile is significantly reduced once portions of the stockpile are reclaimed. See Section 2.1.2 of Reference (3) for more details of this design.

2.1.5 Mine Drainage Ponds for Other Infrastructure

Mine drainage ponds provide storage for gravity flow of mine drainage volumes during large rainfall or snowmelt events and during short power outages. Apart from the temporary stockpile ponds, there will be six other mine drainage ponds constructed at the Mine Site, as shown on



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Large Figure 4 to Large Figure 6 and Drawing MD-001 and MD-002 of Attachment A. These include:

- MD-OSLA will collect drainage from the OSLA
- MD-HRE, MD-HRW, MD-HRN, and MD-HRC will collect mine drainage from the haul roads
- MD-RTH will collect mine drainage from the RTH

The mine drainage ponds for the haul roads and RTH are designed to contain runoff volumes from the 100-year, 24-hour storm. The mine drainage pond for the OSLA is designed to handle the 25-year, 24-hour storm. Preliminary sizing for the mine drainage ponds is listed in Table 2-5. The mine drainage ponds will have the added benefit of reducing TSS, which will limit the amount of sediment in the pumping and piping system.

Table 2-5 Mine Drainage Pond Dimensions – Approximate

Infrastructure	Sump/Pond Name	Area (acres)	Required Capacity (acre-feet)	Design Volume (acre-feet)
Overburden Storage and Laydown Area	MD-OSLA	7.1 ⁽¹⁾	10.7	14.5 ⁽¹⁾
Haul Road	MD-HRE	2.2	10.7	10.7
	MD-HRN	1.4	4.4	4.6
	MD-HRC	1.7	6.1	6.9
	MD-HRW	1.7	3.7	4.0
Rail Transfer Hopper	MD-RTH ⁽²⁾	0.4	0.7	0.7

⁽¹⁾ MD-OSLA was oversized to allow for storage of Peat within the pond, as described in Section 2.1.5.1.

⁽²⁾ MD-RTH was sized based on available area with a larger pump capacity to meet the design storm volume.



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Table 2-6 presents the range of annual mine drainage volumes and flow rates estimated from mine drainage infrastructure ponds based on modeling results.

Table 2-6 Mine Drainage Pond Drainage

	Mine '	Mine Year 1 ⁽¹⁾		Mine Year 11 ⁽¹⁾		Mine Year 20 ⁽¹⁾	
Infrastructure	Average Annual Inflow (gpm)	90th Percentile Inflow (gpm)	Average Annual Inflow (gpm)	90th Percentile Inflow (gpm)	Average Annual Inflow (gpm)	90th Percentile Inflow (gpm)	
Overburden Storage and Laydown Area ⁽²⁾	5.4	6.6	5.4	6.6	5.4	6.6	
Haul Roads ⁽³⁾	53	67	53	67	53	67	
Ore Surge Pile ⁽⁴⁾	20	24	20	24	20	24	

- (1) Source of data: Section 6.1 of Reference (2)
- (2) The OSLA footprint will be fully developed in Mine Year 1 and not reclaimed until after Mine Year 20.
 (3) Haul roads were modeled at their largest extent; inflows represent maximum extent with no change over time.
- (4) The Ore Surge Pile was modeled as fully developed in Mine Year 1 and not reclaimed until after Mine Year 20.

The liner system for these mine drainage ponds has been chosen based on the quality of the water that it will be collecting. The MD-RTH drainage is expected to be similar to that collected from the OSP or Category 4 Waste Rock Stockpile; therefore it will be constructed with the same liner as designed for the Category 4 Waste Rock Stockpile sumps, as described in Section 2.1.4.1. The haul road mine drainage ponds will be constructed with a single HDPE geomembrane over a one-foot thick soil liner, and the OSLA pond will be constructed without a liner.

In general, ponds will be partially excavated and partially filled above the natural ground, designed to optimize the pond bottom with the expected groundwater and bedrock information. As described in Section 2.1.4.1, construction of a lined pond requires adequate foundation drainage to prevent excessive pore pressure from developing under the liner. The pond dikes and slopes will be vegetated or riprapped to limit erosion. The pond dike design will be finalized once the foundation grading design is completed and pond elevations can be established. The pond elevations will allow runoff from disturbed surfaces to be conveyed by gravity into the ponds. The outlet for the haul road ponds and the RTH pond will be a pump and piping system to convey this mine drainage to the WWTF. The outlet for the OSLA pond will be a pump and piping system to convey the mine drainage directly to the CPS pond unless monitoring shows that treatment is necessary.

2.1.5.1 Overburden Storage and Laydown Area (OSLA) Drainage

This section describes the collection and conveyance of runoff from the OSLA including design of the mine drainage pond.



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As described in Section 2.2.3 of Reference (1), the OSLA is a temporary storage area used to screen, sort, and temporarily store Unsaturated Overburden and Peat that may be used for future construction or reclamation purposes. As described in Reference (1), the area will be graded to provide a relatively even, well drained site that directs surface runoff to mine drainage pond MD-OSLA in the southwest corner of the area. Pond MD-OSLA was designed to accommodate runoff from the 25-year, 24-hour storm event with three feet of freeboard. As shown on Table 2-5, this pond was oversized to allow for storage of Peat within the pond to maintain wetland characteristics for future restoration.

Surface runoff from the OSLA is managed as mine drainage because there is concern about the potential release of mercury from Peat storage. Surface runoff from the OSLA will drain to a mine drainage pond for storage and reduction of TSS. The water in Pond MD-OSLA is expected to exhibit water quality similar to construction stormwater and is not expected to require treatment for dissolved substances; however, water quality will be monitored throughout the life of the mine, as described in Section Water Quantity and Quality Monitoring. The water will be pumped from the mine drainage pond directly to the CPS and on to the FTB or to the East Pit during pit filling. See Section 6.9 of Reference (8) and Section 6.5 of Reference (2) for a discussion of mercury in Plant Site and Mine Site discharges, respectively, in long-term closure.

Storage of peat in the OSLA begins prior to the start of mining when peat is removed from areas to be used for stockpiles and the area encompassed by the East Pit. Peat removal from the West and Central Pits will be completed between Mine Years 2 and 11; additions of peat to the OSLA are not expected to occur after Mine Year 11. During operations, peat stored in the OSLA will have the potential to decompose and release mercury.

For newly placed material in the OSLA, the decomposition process of younger peat from the surface oxygenated zone of a wetland may release a pulse of mercury that could occur relatively rapidly based on data from natural peatlands (Reference (9)) and soil laboratory studies (e.g., Reference (10)), with a much slower release rate as time progresses due to a number of factors that include organic materials more resistant to decomposition (Reference (11)). Older peat from below the water table that is placed on the stockpile surface will be exposed to oxygen similar to the oxygenated zone of a peatland (Reference (12)) or an upland forest (Reference (13)), but the decomposition rate is likely to be slow because this older peat has already been subject to decomposition during its longer residence time in the wetland (Reference (11)); therefore, the probability of a pulse of mercury being released from the older peat is likely lower than for younger peat. Because older peat represents the largest mass of material in a peatland (Reference (11)), it will also represent the largest mass of material in the peat stockpile, and because the mixing of younger and older peat will occur, the overall decomposition rate of the newly stored peat is uncertain as is the potential for the release of mercury from the newly stored peat.

The placement of peat in the OSLA will occur over time and will result in previously stored peat being covered (buried) with newly stored peat. As more peat material is added to the stockpile and the peat compresses, oxygen is likely to be limited at depth in the pile (anoxic conditions),



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and decomposition would be limited as typically occurs in the anoxic zone of undisturbed wetlands (Reference (12)). The limited oxygen (anoxic conditions) with depth in the stockpile likely limits the decomposition of the peat and release of mercury similar to the reduced mercury release from the decomposition of organic matter in new reservoirs where oxygen depletion (anoxic conditions) limits organic matter decomposition (Reference (14)).

Mercury released from the decomposition of peat is likely to be Hg(II) (Reference (15)), but has the potential to be: a) converted to elemental mercury (Reference (16)) and volatilized to the atmosphere similar to what occurs in existing upland/peatland watersheds (Reference (15)); b) converted to elemental mercury and adsorbed to organic matter (Reference (16)); or c) re-adsorbed by organic matter and humic acids that have a high affinity for mercury (Reference (16)). Mercury released from organic matter decomposition will have the potential to move with precipitation that falls on the OSLA. However, as shown by upland forest systems that have received atmospheric inputs of mercury for decades and where organic matter is added and decomposed annually, a large share of the mercury is found associated with organic matter (forest floor and upper mineral soil layer enriched with organic matter) (Reference (17); (Reference (18)); Reference (19)). The affinity of the organic matter within the stockpile for mercury may result in less mercury release from the stored peat than might otherwise be expected to occur. Drainage from the OSLA is considered to be mine drainage and is collected in the mine drainage pond MD-OSLA. Settling of solids associated with runoff from the OSLA likely further reduces the amount of mercury that may be associated with water in Pond MD-OSLA. Additionally, TSS in the pond water may provide a medium to adsorb mercury from the water column. Settling of these solids may provide another potential mechanism to reduce the mercury concentration in the pond water.

In Mine Years 1 to 11, water from the OSLA will be routed to the FTB. Any mercury in the water routed to the FTB has the opportunity to mix with Flotation Tailings and be sequestered with the tailings, thereby limiting any release to the environment. In Mine Years 12 to 20, some water from the OSLA is expected to be routed to backfill the East Pit. Mercury in the water routed from the OSLA to the East Pit will mix with the NorthMet waste rock and other fill materials to form a slurry, and similar to the processes occurring in taconite tailings basins and the FTB, mercury is expected to associate with solids and be sequestered at depth in the East Pit. By the time the flooding of the West Pit begins in Year 21, the OSLA will no longer be in operation. In addition, any contributions of water after Mine Year 21 from the East Pit to the West Pit will reflect water from the East Pit and its associated watershed runoff and will not reflect mine drainage from the OSLA, because the OSLA will be reclaimed, as described in Section 7.2.3 of Reference (3).

Because peat removal from the areas to be mined will be completed by Mine Year 11, any potential pulse of mercury from stored peat materials will have occurred, or be ending, by the time water is routed from the OSLA to the East Pit beginning in Mine Year 12. Therefore, the potential release of mercury from the decomposition of peat at the OSLA is not included in the West Pit mercury evaluation in Section 6.5 of Reference (2).



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2.1.5.2 Haul Road Drainage

The quality of the water coming off the haul roads will be related to the amount and type of waste rock and ore spillage occurring on the roadways and is considered mine drainage. Drainage from the haul roads will be collected in mine drainage ditches and directed to one of four lined haul road ponds. Haul roads will generally be kept clear of material for safe travel of the vehicles and as part of best management practices at the Mine Site.

As shown on Table 2-5, haul road drainage will be directed to four separate mine drainage ponds, MD-HRE, MD-HRN, MD-HRC and MD-HRW, as shown on Large Figure 4 to Large Figure 6 and on Drawings MD-001 and MD-002 of Attachment A. MD-HRE is located on the south side of the haul road leading to the Category 2/3 Waste Rock Stockpile, west of the OSP. MD-HRW is located along the haul road to the West Pit, between the haul road and the OSLA. MD-HRN is located south of the Category 4 Waste Rock Stockpile and Central Pit, east of the West Pit, at the intersection of two haul roads. MD-HRC is located on the south side of the haul road leading to the OSP. MD-HRC and MD-HRE will be needed in Mine Year 1, while MD-HRW and MD-HRN will be constructed as the haul roads are expanded to those areas.

Haul road ponds have been designed to contain runoff from the 100-year, 24-hour storm event with three feet of freeboard with design capacities as listed on Table 2-5. Drainage from the haul roads will be directed to these mine drainage ponds prior to being pumped to the WWTF. In some cases, haul road runoff may be directed to a mine pit and included in mine dewatering rather than routed to these ponds.

The haul roads will either be constructed to divide surface runoff to both sides of the road by crowning the middle of the road or by directing surface runoff to one side by super-elevating one side of the road. Depending on the height of these roads above the natural grade, ditches will either be built in the road section or adjacent to the road. These mine drainage ditches will only collect surface runoff from the road cross-section. Stormwater runoff from adjacent areas will be intercepted before entering the road section and routed to stormwater ponds. This may mean construction of parallel ditches in some areas, one for mine drainage and one intercepting adjacent stormwater. This will minimize the size of the mine drainage ditches and the amount of water requiring treatment from haul road drainage.

The haul roads will be constructed with safety berms as required by the Mine Safety and Health Administration (MSHA) and described in Section 2.1 of Reference (1). Several construction methods may be used to allow surface drainage from the haul roads to flow past the safety berms into the mine drainage ditches. Such methods include:

- Constructing safety berms of coarse, free-draining rock
- Installing culverts under safety berms
- Constructing safety berms of boulders with gaps between them allowing road drainage



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• Leaving a small gap in the safety berm, as allowed by MSHA rules

2.1.5.3 Rail Transfer Hopper (RTH) Area Drainage

As described in Section 2.2.1 of Reference (1), the RTH is used for loading ore into rail cars. Due to the nature of the work and potential for ore spillage, surface runoff from the RTH active areas will be considered mine drainage. The layout of the RTH consists of a raised platform on which haul trucks enter and exit the area and from which they dump ore into a hopper over a pan feeder, which conveys the ore into rail cars. There will be a sloped concrete floor within the RTH, directing runoff to the south. The runoff will cross the rail spur on sloped concrete panels to a small swale along the south side of the railroad track to pond MD-RTH. Water from MD-RTH will get pumped to the WWTF.

Pond MD-RTH will be designed to accommodate runoff volumes from the 100-year, 24-hour storm event from the RTH with one foot of freeboard with design capacities as listed on Table 2-5.

2.1.6 Waste Water Treatment Facility (WWTF)

Mine drainage, with the exception of mine drainage from the OSLA, will be treated at the WWTF and then pumped to the FTB for re-use or to the East Pit during pit filling after Mine Year 11. The purpose of this water treatment is to improve the quality of water going to the FTB. The design and treatment process of the WWTF can be modified as needed. Because the design of the WWTF can be adapted as modeling and monitoring require, the details of the design are included in Section 2.2 of Reference (4).

The WWTF will be located west of the RTH, as shown on Large Figure 4. Mine drainage streams at the Mine Site will be combined into three waste streams for treatment at the WWTF: construction water, mine drainage with relatively high levels of metals and sulfate, and mine drainage with relatively low levels of metals and sulfate. Mine drainage from construction dewatering of Saturated Overburden will be treated in a construction water stream and will only be produced through approximately Mine Year 11. Mine drainage that is anticipated to contain relatively high levels of metals and sulfate (drainage from the temporary Category 2/3 and Category 4 Waste Rock Stockpile liners and the temporary OSP liner) will be stored in the West Equalization Basin (West EQ Basin) and routed to the chemical precipitation treatment train. Mine drainage that is anticipated to contain relatively low concentrations of metals and sulfate (drainage from haul roads, the RTH, pit dewatering and Category 1 Waste Rock Stockpile drainage) will be stored in the East Equalization Basin (East EQ Basin) and routed to the membrane filtration treatment train.

The WWTF effluent will be conveyed to the CPS pond to be blended with the OSLA runoff and construction water prior to being pumped through the TWP for use at the FTB or, after approximately Mine Year 11, used to supplement flooding of the East Pit after.



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2.1.7 Central Pumping Station (CPS) and Treated Water Pipeline (TWP)

Mine drainage treated by the WWTF, mine drainage from the OSLA that does not need treatment, and construction water will be discharged into the CPS pond, which will be the collection point for all water that will be pumped to the FTB or to the East Pit during pit filling. Consequently, the CPS pond will be constructed with a geosynthetic clay liner. The CPS pond will have an active storage capacity of approximately 1.2 million gallons plus three feet of freeboard.

The CPS houses three pumps that have a combined design capacity of 4,000 gpm. Water collected in the CPS pond will be pumped through the TWP to the FTB (see Drawing ME-003 of Attachment A), with the exception of any water needed during East Pit flooding operations, starting in Mine Year 12. A pipeline will be constructed from the CPS to the pits to facilitate pit flooding. Flows through the CPS are expected to be continuous year-round, with lower flows during the winter months and during periods with low precipitation. The CPS pond is expected to receive flows that do not vary significantly as a result of storm and snowmelt events due to the upstream storage in mine drainage ponds and the WWTF equalization basins and treatment units.

The TWP will be used continuously throughout the year and will be designed and constructed to prevent freezing in the winter. The TWP will consist of the pipeline, air/vacuum relief valves, drain valves, and in-line flow meters on each end of the TWP.

The alignment selected for the TWP is parallel to the existing Dunka Road alignment and has a total length of approximately 40,000 feet. The TWP will be designed so that it safely discharges into the FTB to prevent any potential erosion of tailings or the FTB dams. The following criteria were used in selecting this route:

- The TWP will be next to Dunka Road, which will be utilized for daily mine traffic.
 This means that the corridor will be under regular observation by mine personnel. In
 the unlikely event that a leak should develop, it can be quickly identified and
 repaired. In addition, flow meters at both ends of the TWP will allow for quick
 detection of any loss of fluid.
- Wetland impacts along this established route are not as great as along the other alignments considered.
- The alignment never crosses a major road planned for regular mine traffic or a rail line, minimizing the risk of structural failure due to surface loads from heavy mine vehicles or trains.
- The majority of the route is in areas already disturbed by previous activities.
- This route provides easy access for operations, maintenance, and repairs of the TWP.



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• Preliminary review of the alignment did not identify any major constructability concerns.

The TWP will be designed to handle flow rates from one, two or three operating CPS pumps. The total design head (static plus dynamic) is estimated at approximately 410 feet at the design flow of 4,000 gpm with a nominal inner pipe diameter of 16 inches. Smaller pipe diameters would result in a significant increase in pumping head at higher flows, and larger pipe diameters would result in increased pipeline cost with no significant reduction in pump horsepower. Larger pipe diameters would also result in slower water velocities, which would not suitably scour settled particles. Pipeline velocities ranging from approximately 1 fps to 10 fps at the selected pump capacity will be used to help select the TWP diameter for construction.

The majority of the pipeline is proposed to be installed above grade to minimize bedrock excavation, minimize installation costs and facilitate ease of maintenance. To protect the above grade sections from freezing during winter operations, it will be covered with approximately eight feet of material. Side slopes of this cover will be approximately 1.5 (horizontal) to 1 (vertical), resulting in a footprint that will be approximately 26 feet in width. Sections of the TWP will be buried to allow for access over the TWP: the depth of cover of these sections will be determined based on bedrock elevation. In sections where available coverage is not adequate between the existing grade and bedrock, insulation will be added to protect the TWP from freezing.

The material used for bedding and cover will consist of overburden from the TWP construction or from the Mine Site, and/or LTV Steel Mining Company (LTVSMC) tailings from the Plant Site. The TWP cover material will be protected from erosion with topsoil where needed and seeded to establish vegetation.

The TWP drawings included in Attachment A include the general layout (Drawing TWP-001), plan and profile sheets (Drawing TWP-002 to TWP-009), typical sections (Drawing TWP-010), and details of the installation (Drawing TWP-011). Because varying topographic conditions along the TWP corridor will require different installation methods, five typical cross-sections have been developed to illustrate the methods of construction. Although it is anticipated that the five standard cross-sections shown on Drawing TWP-010 will address most conditions encountered, variations and modifications to these standard arrangements may occasionally be necessary, and will be addressed during the final design of the TWP prior to construction. As shown, the TWP generally follows the surface profile of the Dunka Road corridor with sufficient cut and backfill to avoid abrupt changes in elevation. The TWP layout also avoids abrupt changes in horizontal direction minimizing the need for pipeline fittings. Automatic air/vacuum relief valves will be placed along the alignment at the high points as shown on the plan and profile sheets. Likewise, manually operated drain valves will be provided at the low points to allow drainage of pipeline sections for maintenance. Flow metering will be provided at the CPS discharge and in a meter manhole prior to discharge into the FTB.



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2.2 Stormwater Management

Three types of stormwater will be managed at the Mine Site:

- Non-contact stormwater is precipitation and runoff that contact natural or reclaimed surfaces, including reclaimed portions of the permanent Category 1 Waste Rock Stockpile, and surface runoff that has not been exposed to mining activities.
- Stormwater associated with construction activities (i.e., construction stormwater) which consists of runoff from construction areas with no excavation and construction areas where the majority of the material being excavated is Unsaturated Overburden or Peat.
- Stormwater associated with industrial activities (i.e., industrial stormwater) which consists of precipitation and runoff that come in contact with on-site features constructed of rock (either Category 1 waste rock or rock from off-site sources), where the discharge is composed entirely of stormwater and not combined with mine drainage.

These three categories of stormwater will be comingled and will be kept separate from mine drainage through a system of ditches, dikes, and ponds. For the purpose of this chapter, these three categories of stormwater will be referred to collectively as stormwater.

Runoff from the Transportation and Utility Corridors, including the Dunka Road corridor and the adjacent railroad corridor (which will generally remain in its existing condition), will contribute to the stormwater runoff volumes from the Project.

The Mine Site stormwater management system will be developed as required throughout the mining operation to control site stormwater flows and volumes that would result from the 100-year, 24-hour storm event, at minimum. The overall system capacity will be based on the Mine Site configuration, and the individual segments will be installed when needed, as shown on Large Figure 7 to Large Figure 9. Permit design drawings of the stormwater system are included in Attachment B.

Stormwater management is modified during reclamation and during long-term closure, including filling of some ditches, construction of new ditches, and reclamation of the sedimentation ponds into wetlands or uplands, as described in Section 7.0.

Stormwater in and around the Mine Site will be managed in a manner that reduces potential impacts to mining activities, protects the environment, and maintains existing flow patterns to the extent practicable. The volume and rate of stormwater flows will be altered by construction of stockpiles, pits, and mine infrastructure (haul roads, RTH, OSLA, etc.), because runoff from these areas will be captured and treated as mine drainage.

Stormwater flowing on and off the Mine Site will be controlled by natural watershed divides and a series of dikes and ditches constructed around the perimeter of the Mine Site, along the pit



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rims, and around the interior of the Mine Site. Sedimentation ponds will be constructed along the perimeter of the Mine Site to reduce TSS from these stormwater ditches prior to discharging offsite.

2.2.1 Stormwater Modeling

The stormwater ditches and sedimentation ponds were modeled using XP-SWMM, Version 10.6, which is a software package used to model stormwater, sanitary water, and river systems. The design for the stormwater ditches and sedimentation ponds was based on a critical year, which represents the Mine Year producing the highest quantity of runoff for each ditch and pond network. Once the critical year was established, the sedimentation ponds and stormwater ditches were designed using the 10-year and 100-year, 24 hour Soil Conservation Service (SCS) Type II storm events with rainfall depths for the mine location obtained from Reference (20).

For the sedimentation ponds, the design goal was to control, at a minimum, the 10-year, 24-hour storm event through the outlet pipe(s). The ponds provide a secondary spillway to control the discharges greater than the 10-year, 24-hour storm event. See Section 2.2.4.2 for detailed information regarding design of the ponds and outlets.

The following sections describe the major components of the stormwater management system.

2.2.2 Exclusion Dikes

Dikes will be placed at strategic locations around the perimeter of the site and around the pit rim as described in the following sections.

2.2.2.1 Perimeter Dikes

The purpose of constructing dikes and ditches at or near the perimeter of the Mine Site is to:

- minimize the amount of surface water flowing onto the Mine Site
- minimize dewatering of wetlands outside the perimeter of the Mine Site
- prevent mine drainage (i.e., water that has contacted surfaces disturbed by mining activities) from flowing uncontrolled off the Mine Site
- manage the rate and location of stormwater flowing off the Mine Site

The criteria used to select dike alignments include:

- as close to the Project boundary as practicable to avoid obstructing mining operations
- where needed to facilitate construction of subsurface flow cutoff to prevent shallow groundwater flow from entering the Mine Site



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- where the ground surface at the Project boundary is lower than flood levels in surrounding water bodies, and flood levels are high enough to flow onto the Mine Site if not controlled
- where mine drainage from construction areas or other surfaces disturbed by mining activities will otherwise discharge off the site and where ditches will not adequately control the runoff
- where needed to ensure that stormwater runoff is detained and discharged in a manner that will meet stormwater quality requirements

Dikes will be constructed of the silty sands or glacial till material excavated during construction of ditches and removal of overburden. Side slopes will be vegetated to control erosion. Small dikes will also be constructed, as needed, along interior stormwater ditches and around stockpile construction areas to separate stormwater and mine drainage around the Mine Site.

In order to convey stormwater adjacent to the dikes, prevent surface runoff from entering the mine pits, intercept stormwater prior to reaching mine drainage areas, and prevent water from pooling in areas where the dikes cut across low areas, ditches will be constructed along the interior of most of the perimeter dike system. In addition, there will be some areas along the site perimeter where the existing ground is already relatively high so that a ditch will be able to capture the site surface runoff without a dike. Stormwater captured by the ditches will be directed to sedimentation ponds and then routed into a natural drainage system. Where glacial till is present in the dike foundation zone below the water table and where inspection trenching (conducted at the time of construction) indicates potential for high-permeability conditions or where peat is present, seepage control measures may be installed to restrict groundwater movement. As part of construction-level design, test trenches will be excavated along the perimeter dike alignments to determine the underlying soil conditions. The test trenches will be used to evaluate the need for construction of cutoff trenches.

In areas where glacial till is present, seepage control measures may include soil cut-off trenches constructed of compacted silty sand or compacted glacial till, or slurry trenches. The decision on which design to use will depend on depth to bedrock and soil type on which the dike will be built. In areas where peat is present, seepage will be prevented by compressing the peat by placing earthen dike materials over the surface to surcharge the peat to create a low-permeability layer. If a sand seam or other high-permeability material is found in the dike foundation zone below the peat deposit, a soil cutoff trench, slurry wall, or sheetpile wall will be installed (depending on depth to bedrock) to cut off seepage. Geotechnical testing has indicated that silty sand soils found at the Mine Site are a relatively low-permeability material in their natural state, as discussed in Section 4.1 of Reference (21). Therefore, seepage cutoffs are generally not planned to be used in areas where dike foundation soils are silty sand.



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The alignment of the perimeter dikes for the various years of mine operation are shown on Large Figure 7 to Large Figure 9 or on Drawings SW-003 and SW-004 of Attachment B.

2.2.2.2 Pit Rim Dikes

Pit rim dikes will be constructed in areas where surface water might otherwise drain into the mine pits. The pit rim dikes are temporary in nature, intended to be in place only as long as the rim of the mine pit is at a specific location. Dikes will be constructed by pushing up a ridge of soil where needed around the rims of the pits during overburden stripping operations. Pit rim dikes do not require as rigorous control of construction materials (compaction and moisture content control) as the perimeter dikes need. They can be constructed quickly and economically to cut off surface water flow into the pits. Dikes will be removed and reconstructed in a new location as the mine pit expands. These dikes are intended only to intercept and direct surface runoff, not to impede movement of groundwater flow. The dikes will also serve as safety berms for mining operations at some locations and will need to remain in place until mining operations are terminated at those locations.

2.2.3 Interior Ditches

The intent of stormwater ditch construction throughout the interior of the Mine Site is to:

- route stormwater away from the areas of mining activity to minimize the amount of mine drainage created on the Mine Site
- convey collected stormwater to perimeter ditches and sedimentation ponds prior to controlled discharge from the Mine Site
- minimize the impacts of mining operations on the Partridge River system

The layout of the proposed stormwater system was designed to maintain existing drainage boundaries and discharge locations at the Mine Site to the extent practical while still maintaining the objectives of the system. The primary strategy is to intercept stormwater prior to contacting areas that have been disturbed by mining activities, which will minimize the amount of mine drainage and the overall impacts to the Partridge River.

Mine Site Stormwater Permit Support Drawings SW-003 through SW-005 of Attachment B, show the layout of the stormwater ditches, dikes, and ponds for Mine Years 1, 11, and 20, respectively. Ditch plan and profile views are shown in Drawings SW-013 through SW-030 of Attachment B. Cross-sections and details for stormwater ditches are shown in Drawings SW-007 through SW-012 of Attachment B.

2.2.4 Sedimentation Ponds and Outlets

Sedimentation ponds will be constructed to reduce TSS from stormwater runoff and allow for controlled discharge of stormwater from the Mine Site. There are five stormwater sedimentation



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ponds planned for the Mine Site, as shown on Large Figure 7 to Large Figure 9. Pond A is located at the northeast corner of the Category 1 Waste Rock Stockpile and directs stormwater from the north and west sides of the stockpile off-site. Pond B is located between the East Pit and northern border of the Mine Site. Pond C (West) is located west of the West Pit and was designed to provide additional flood storage and reduction of peak discharge rates prior to Pond C (East), which is located west of the OSLA and downstream of Pond C (West). Pond D is located west of the OSP, on the north side of Dunka Road.

Stormwater will be routed from the Mine Site to these five locations around the perimeter of the site. These locations were selected to discharge stormwater into existing flow paths outside of the Mine Site boundary to mimic existing conditions to the extent possible and minimize the overall hydrologic impacts to the Partridge River. Some existing culvert locations along Dunka Road were consolidated through diking and ditching to limit the number of outlets from the site to simplify management, monitoring, and operations. Four of these sedimentation ponds (shown on Drawing SW-003 of Attachment B) will be constructed in Mine Year 1, with the remaining pond, Pond C (West), constructed in Mine Year 2 (shown on Drawing SW-004 of Attachment B).

2.2.4.1 General Design Criteria

The stormwater sedimentation ponds will be designed to limit TSS outflow concentrations into natural flow paths to meet the TSS discharge limits established in the NPDES/SDS Multi-Sector General Permit for Industrial Stormwater Activity (Permit Number [No.] MNR050000). It may take several years to establish a thick vegetative cover on the reclaimed stockpile surfaces and as a result, sediments in stormwater may temporarily be higher than under natural conditions until the vegetative cover is fully established.

The inflow TSS concentrations may fluctuate over time and can only be estimated for this design. The design assumes inflow TSS concentrations of 50 milligram per liter (mg/L) during baseflow conditions and 100 mg/L during storm events. These TSS estimates are believed to overestimate the actual concentrations that will occur, although the inflow concentrations used in the design will need to be confirmed once water quality sampling can be conducted.

Sediment removal in the sedimentation ponds is extremely sensitive to the grain size distribution of the sediments in the stormwater entering the pond. The grain size distribution of the inflow sediments used in the design will also need to be confirmed once water quality sampling can be conducted at the site and additional geotechnical data can be obtained. The ponds and outlet configuration will be modified as necessary according to any new data as necessary to meet the permit requirements. The pond surface areas were designed to remove 70% of sediment during the 10-year and 100-year storm events. The baseflow and the 10-year storm event assumes a larger percentage of fines using a design gradation of 0.0363 millimeters (mm), 70% of which will be larger than this according to the reference gradations. The 100-year storm event uses a design gradation of 0.05 mm, of which 70% of the expected sediment will be larger than this



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according to the reference gradations. However, a small change in the grain size distribution could result in a large change in the required surface area of the pond for sediment removal.

Additionally, stormwater permitting has been taken into account in the design of these ponds. The MPCA issued its new NPDES/SDS Multi-Sector General Permit for Industrial Stormwater Activity (Permit No. MNR050000) on April 5, 2010. This permit includes TSS storm event benchmark limit of 100 mg/L. The Project is expected to have an individual NPDES/SDS permit but these same limits will likely apply; therefore this requirement was used in the design criteria. The MPCA has issued its next version of the NPDES/SDS Multi-Sector General Permit for Industrial Stormwater Activity (Permit No. MNR050000), which goes into effect April 5, 2015. This new permit maintains the TSS storm event benchmark limit of 100 mg/L.

The annual average flow was used to size the ponds for the baseflow condition, for which a lower TSS concentration but higher percent of finer sediments is expected. The peak flows from the SCS Type II, 10-year and 100-year, 24-hour storm events determined from the XP-SWMM model were used to size the ponds for storm event flows, for which a higher TSS concentration but lower percent of finer sediments is expected. The 100-year, 24-hour storm event flow was the driving factor in the required pond surface area. The pond, culvert, and weir sizes were designed to slow the flow through the stormwater ponds long enough to allow the required settling within each pond. TSS removal was estimated using the assumption of a steady-state plug flow reactor and computing sediment settling velocity (in still water) according to Dietrich (Reference (22)).

In general, the storage volumes available in the sedimentation ponds will result partially from excavation and partially due to construction of dikes above natural ground. The planned 3 (horizontal) to 1 (vertical) side slopes of the pond dikes will ensure a stable cross-section that will provide sufficient flow path length to control leakage. Side slopes will be covered with soil and seeded to control erosion. With this design, the diversion ditches will flow by gravity from the channels into the sedimentation ponds, and additional storage can be provided above the ground. This also allows better control of the pond outflows and increases their sedimentation efficiency.

2.2.4.2 Sedimentation Pond Sizing and Outlet Design

The sizes of the sedimentation ponds have been designed and will be constructed to meet the objectives for the MPCA's NPDES/SDS Multi-Sector General Permit for Industrial Stormwater Activity (Permit No. MNR050000). The primary design objective is reduction of sediment in runoff from storms up to the 100-year, 24-hour storm event.

The primary outlet structures for the ponds will be designed and constructed to accommodate flows up to the 10-year, 24-hour storm without overtopping the secondary spillway. Detention storage will be provided to accommodate runoff volumes up to the 10-year storm event. An earthen weir and secondary spillway will also be constructed through the pond embankment to accommodate flows from storms larger than the 10-year storm event without overtopping the



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dikes or roads. The downstream side of these overflow structures will include erosion control measures such as riprap. Accumulated sediment will be removed from sedimentation ponds by pumping or dredging, as required. Due to its outlet through Dunka Road, Pond D has a single weir outlet (with no secondary spillway) designed to accommodate the 100-year storm and convey stormwater discharge from the pond to an existing wetland south of Dunka Road.

Due to the proximity to the Partridge River, and the predicted elevations of the Partridge River flood flows, each of the outlet culverts on Ponds A and B will be fitted with check valves to prevent water from flowing from off-site into the ponds. Both the primary outlet and an emergency outlet were designed for conditions specific to each pond's needs, with the primary outlet designed to provide flood attenuation and the secondary outlet designed to pass flows larger than the design values used to size the sedimentation pond and primary outlet. The pond outlet configuration showing the primary outlet culverts and secondary overflow earthen weirs are illustrated on Drawings SW-008 to SW-012 of Attachment B. Similar to all long-term infrastructure at the Mine Site, this outlet design was chosen to provide reliable service and to minimize maintenance.

The ponds will be excavated to provide 8 to 10 feet of dead storage below the primary outlet invert to provide capacity for the sedimentation and to prevent resuspension of sediment. As required under the MPCA's general stormwater program, these ponds will have depths no greater than 10 feet. The ponds will typically be sized to achieve a length-to-width ratio that will range from about 2 to 3 to reduce the probability of short-circuiting and allow adequate sedimentation for flows up to the 100-year storm event.

In many proposed pond locations, groundwater is near or at the existing ground surface. The normal water level of the ponds will be based on the expected groundwater elevations.

To achieve the required TSS removal efficiencies, the surface area of the ponds range from 1.7 to 6.0 acres. For all ponds, with the exception of Pond D, the primary outlet of each pond will consist of between one and six reinforced concrete pipe culverts with diameters ranging from 24 to 48 inches. The culvert inverts will be set at the normal water level (NWL) in each pond. Culverts will have a positive slope discharging to the downstream side of the dike or road embankment. With the exception of Pond D, each pond will also have a secondary overflow structure to allow flows above the capacity of the primary outlet to discharge without overtopping the dikes or road embankments. The spillway will be an earthen spillway with an elevation set between 0.5 feet and one foot below the dike elevation and vary in length from 6 to 200 feet. The downstream side of these spillways will include erosion control measures, such as riprap, as needed. The riprap will be Minnesota Department of Transportation Standard Specification 3601 Random Riprap Class III, IV, or V, depending on the size of riprap required for the calculated discharge velocity for the spillway.

Existing culvert invert elevations under Dunka Road were maintained where they will be replaced to ensure that existing flows will not be impeded. Therefore, culverts to or from ponds located adjacent to Dunka Road, including culverts directing flow from Pond D and Pond C-



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East, are designed to maintain the surveyed grades of the existing corrugated metal pipe culverts under the road. The culverts under Dunka Road for Pond C-East have been designed to convey the 100-year, 24-hour storm event.



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3.0 Key Outcomes

Through modeling (described in detail in Reference (2)), water quantity and quality estimates have been determined and used in the design of these water management systems. The modeling also includes the expected water quantity and quality outcomes resulting from these water management systems which are summarized in this section.

3.1 Water Quantity

The Mine Site water balance determines the quantity of water that will be removed from the Upper Partridge River watershed and the disposition of that water. Mine drainage will be pumped to the FTB, which will reduce the amount of water withdrawn from Colby Lake, or used to flood the mine pits.

Groundwater appropriation will be needed for both construction and operations at the Mine Site. Estimated water appropriation flows will be provided as part of water appropriations permitting.

3.2 Water Quality

Reference (2) describes the water quality modeling with key outcomes summarized as follows:

- estimated West Pit lake water quality in Large Table 1
- estimated groundwater quality in Large Table 2 and Large Table 3 along two surficial groundwater flow paths downstream of the Mine Site
- estimated surface water quality in Large Table 4 and Large Table 5 at two surface water locations downstream of the Mine Site
- estimated stockpile drainage water quality in Large Table 6
- estimated Treated Water Pipeline water quality in Large Table 7



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4.0 Operating Plan

Once construction begins and until the West Pit lake concentrations meet the required water resource objectives or until non-mechanical treatment has been proven effective at achieving water quality objectives, as described in Section 2.1.1 of Reference (4), water at the Mine Site must be continually channeled, monitored, treated at the WWTF, and pumped as necessary to allow the mine to function efficiently and to protect the environment. This section describes the steps and processes planned at the Mine Site during the operating phase. Section 7.0 describes the management of water during reclamation and in long-term closure.

4.1 Mine Drainage

Mine drainage may not meet water quality limits for metals or other constituents. With the exception of water from the OSLA, all mine drainage will be routed to the WWTF prior to being routed through the TWP to the FTB or for pit flooding in later years. Mine drainage will be intercepted by ditches, dikes, the Category 1 Waste Rock Groundwater Containment System, and stockpile foundation liners to keep it separate from the stormwater conveyance systems as detailed below.

4.1.1 Waste Water Treatment Facility (WWTF)

In the early months of Mine Site development (Mine Year 0), the first phase of the WWTF will be built, specifically the East EQ Basin and the Construction Water Treatment Building. These facilities will treat construction water generated during Mine Site development activities described in Section 2.2 of Reference (4). During Mine Year 1, these facilities will treat both construction water and mine drainage, while construction of the West EQ Basin, Construction Water Basin, and the first half of the mechanical treatment are taking place. Mechanical treatment includes chemical precipitation and membrane filtration treatment. The WWTF will be fully operational at the end of Mine Year 1 and able to treat mine drainage. After Mine Year 1, construction water will be routed to the Construction Water Basin, treated by chemical addition from the Construction Water Treatment Building, and subsequently discharged to the CPS pond. It is anticipated that the second half of the mechanical treatment will be constructed starting in Mine Year 3.

Operation of the WWTF is described in Section 2.2 of Reference (4).

4.1.2 Central Pumping Station (CPS)

Under normal conditions, the CPS pumps will be operated automatically by liquid level sensing equipment. The three pumps will be started in sequence, one at a time, as required to maintain the water level in the CPS pond at safe levels. Start pump and stop pump levels will be based on the depth of water in the basin. As the water level in the pond rises, the first pump will be started at reduced speed. If the water level continues to rise, a second pump will be started at reduced speed. Likewise, the third pump will be started should the inflow to the CPS pond exceed the capacity of two pumps operating at maximum speed to maintain the desired water level in the



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basin. During normal operations, the third pump is considered redundant, with the design flow of 4,000 gpm provided by two pumps operating in parallel.

Preventive maintenance will be an integral part of the operation of the CPS. Preventive maintenance will be focused on keeping equipment operable under the expected range of operating conditions. Preventive maintenance tasks include, but are not limited to:

- daily observation of pump operation and review of alarm conditions, if any, that have occurred
- daily verification that the flow meters at the CPS and the end of the TWP are properly sending data and that data appears to be valid over the previous 24-hour period
- weekly inspection of the intake screens; clearing debris as required
- inspection of any ice control measures at the intake, prior to winter, to ensure that they are operational; during winter, daily inspection of the pump station intake to ensure that ice is not forming to the extent that could damage the intake and/or restrict flow to the pumps
- annual inspection of instrumentation, controls, and electrical components and replacement of worn or damaged parts
- annual cleaning of intake well, as required, to remove any solids that may have collected
- inspection of pumps and valves, with rebuilding, as required, at intervals of approximately one billion gallons of water pumped for each pump; valve lubrication as required
- regular inspection of building services, such as heating and ventilation; service as required

4.1.3 Treated Water Pipeline (TWP)

A flow meter will be installed at each end of the TWP. The difference in flow between these flow meters will be continuously monitored. If the difference in flows indicates a leak is occurring, an alarm will sound and the CPS pumps will automatically be stopped.

4.1.4 Mine Site Pipelines

The Mine Site pipelines will carry the water from the mine drainage sumps and ponds, located around the Mine Site, to the WWTF and CPS pond. The only water that is expected to be pumped directly to the CPS pond is from the OSLA mine drainage pond (MD-OSLA). All other mine drainage at the Mine Site is expected to require treatment and will be sent to the WWTF.



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The mine drainage piping system will expand as the mine expands, connecting the pits, stockpiles, and ore handling areas with the WWTF and CPS pond. The condition of the pipes will be monitored, and maintenance will be performed as necessary.

4.1.5 Mine Site Sumps and Pumps

Sumps are located throughout the Mine Site in the pits, around the edge of the temporary waste rock stockpiles, and along the edge of the OSP, as described in Section 2.1.4.1. The water that collects in these sumps will be pumped to the WWTF via the mine drainage pipelines. The condition of the pumps and sumps will be monitored, and maintenance or replacement of the pumps will be performed, as necessary.

4.1.6 Mine Site Mine Drainage Ponds

If a storm event, snowmelt, or power outage creates more water than a stockpile sump can contain, the excess water from the sumps will overflow to adjacent mine drainage ponds.

Mine drainage ponds without sumps are located in six locations to collect drainage from the haul roads, RTH, and OSLA. In these cases, mine drainage runoff will flow by gravity from these mine drainage areas to their appropriate ponds. Water in the mine drainage ponds will be pumped to the WWTF for treatment via the mine drainage pipelines.

The condition of the ponds and pumps will be monitored, and maintenance or replacement of the pumps will be performed as necessary. Mine drainage ponds have been designed with access for maintenance clean out, as needed. The need for and frequency of sediment clean out will be assessed during annual pond inspections.

4.2 Stormwater

Stormwater ponds will be inspected annually to determine the depth of sedimentation within the ponds. These ponds will be dredged if the depth of sedimentation reduces the required storage capacity beyond what is needed based on the pond design.

Stormwater dikes and ditches will be monitored after construction as part of standard operation and maintenance activities to detect excessive seepage. Should a zone of excessive seepage be identified, sheet pile, grouting, or other seepage control technologies can be installed with the dike in place. Alternatively, a low-permeability material could be compacted in a trench excavated near the toe of the dike without disturbing the dike.

The stormwater management infrastructure will be operated in accordance with the Construction Stormwater Pollution Prevention Plan (SWPPP), which will be developed prior to construction, and the Industrial SWPPP, which will be developed prior to the start of operations. These SWPPPs will be developed to meet the requirements of the Minnesota NPDES/SDS Construction Stormwater General Permit (Permit No. MN R100001) and the Minnesota NPDES/SDS Industrial Stormwater General Permit (Permit No. MNR050000), respectively.



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A SWPPP is a "living" document that evolves with changes at a site. PolyMet will amend these SWPPPs whenever there is:

- a change in Mine Site facilities
- a change in the operating procedures of the facility
- a change that may impact the potential for pollutants to be discharged via stormwater

The intent of these SWPPs is to protect water quality by preventing pollution from stormwater associated with construction activities and industrial activities, respectively. These SWPPs will identify and describe controls and Best Management Practices (BMP) proposed for the site; these controls and BMPs are designed to minimize the discharge of potential pollutants in stormwater runoff.

Inspections and recording activities are important parts of the continued success of these SWPPPs. The frequency and extent of the inspections will be defined in each SWPPP. Draft recordkeeping forms will be included in each SWPPP.

4.3 Spills

This section is a summary of the Mine Site Spill Prevention Control and Countermeasures (SPCC) Plan which will be developed prior to start of operations). The SPCC provides the procedures for response to spills. These procedures apply to all PolyMet employees, contractors, and vendors delivering, dispensing, or using petroleum products at the Mine Site. It is the policy of PolyMet to promote a long-term, continuous effort towards spill prevention first, and control and countermeasures where necessary. An SPCC Plan Administrator will be designated and is responsible for developing, implementing, and maintaining the SPCC Plan. In the case of a spill, the procedures for emergency contacts and a spill contingency plan will be further described in the SPCC Plan. Training sessions and spill prevention briefings for operating personnel will review the requirements of the SPCC Plan and highlight and describe recently developed precautionary measures.

4.4 Overflows

This section includes discussion of what will occur in the event of an overflow of process and stormwater containment features. An overflow may occur when a storm event exceeds the design storm or an extended power outage occurs at the Mine Site. In order to prevent and mitigate the effects of possible overflows, the following operational plan will be used.

4.4.1 Mine Drainage

The storage capacities of the mine drainage sumps and ponds are provided on Table 2-4 and Table 2-5 and on Drawing ME-004 of Attachment A. Where the sumps and ponds are next to each other, the sump volume only includes the volume before it will overflow into the pond,



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whereas the pond volume is the remaining volume apart from the sump. This is the storage capacity of the design and does not include the freeboard (typically three feet) or contingency response plans for removing or rerouting the water to prevent overtopping.

Mine drainage collection from the temporary stockpiles, haul roads, and ore handling areas (OSP and RTH) will likely require treatment to meet water quality standards. The design storm for these facilities, the 100-year, 24-hour event, only has a 1% chance of being exceeded in any given year, or an 18% chance of being exceeded during the 20-year life of the Mine Site. Although these facilities have been designed according to a significant design storm, there may be occasions during the life of the mine that the design storm is exceeded, resulting in runoff exceeding the capacity of the facilities. The design includes a factor of safety in the form of freeboard volume, and additional contingencies have been developed to minimize environmental impacts in the event the total volume available is exceeded.

For storm events in excess of the design storm, mine drainage from temporary stockpiles, haul roads, and the ore handling areas will continue to fill the ponds within the excess capacity (freeboard) included in the design of each pond. With the exception of the RTH pond, the sump and pond designs include three feet of freeboard based on the MPCA's *Recommended Pond Design Criteria* (Reference (23)). Due to the lack of room, the RTH pond has been designed with one foot of freeboard, but will include a larger pump with that can meet the higher capacities. Use of freeboard in the design provides a significant factor of safety for these ponds, with a total excess capacity (design volume plus freeboard) ranging from approximately 30% to 170% over the required capacity, as shown in Table 4-1.

Table 4-1 Sump and Pond Excess Capacity

Infrastructure	Sump/Pond Name	Required Capacity (acre-feet)	Design Volume (acre-feet)	Freeboard Volume (acre-feet)	Excess Capacity ⁽¹⁾
	S23-1	14.9	14.9		33%
Category 2/3	S23-2	11.9	12.0	15.7	
Waste Rock	MD-S23-1	21.4	21.5		
Stockpile	S23-3	6.6	6.6	6.4	54%
	MD-S23-3	5.3	5.3		
Category 4	S4	10.1	12.5	10.6	81%
Waste Rock Stockpile	MD-S4	8.1	9.9		
Ore Surge Pile	SOSP	8.5	8.7	5.6	47%
Ore Surge File	MD-SOSP	4.8	5.3	5.0	41 70



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Infrastructure	Sump/Pond Name	Required Capacity (acre-feet)	Design Volume (acre-feet)	Freeboard Volume (acre-feet)	Excess Capacity ⁽¹⁾
Overburden Storage and Laydown Area	MD-OSLA	10.7	14.5 ⁽²⁾	14.1 ⁽²⁾	167% ⁽²⁾
	MD-HRE	10.7	10.7	4.2	39%
Haul Roads	MD-HRN	4.4	4.6	2.8	69%
Haui Roads	MD-HRC	6.1	6.9	3.0	61%
	MD-HRW	3.7	4.0	2.6	77%
Rail Transfer Hopper	MD-RTH ⁽³⁾	0.7	0.7	0.3	43%

- (1) Excess capacity compares the total capacity (design volume plus freeboard volume) to required capacity.
- (2) MD-OSLA was oversized to allow for storage of Peat within the pond, as described in Section 2.1.5.1.
- (3) MD-RTH was sized based on available area with a larger pump capacity.

Although the chance that the total design volume will be exceeded is small, an operational contingency plan has been developed in the event that this occurs. The pumping networks draining these sumps and ponds are sized for the snowmelt event; therefore any additional pumping capacity required must be increased through a second pump system. Although it will not be cost-effective to have a second permanent pump and pipeline network in place in the event of an extended power outage or storm event that causes the design capacities to be exceeded, an emergency operating procedure has been developed to manage mine drainage under these circumstances. This operational contingency plan includes use of temporary diesel pumps to operate during events greater than the design volume or under circumstances of extended power outages associated with heavy rainfall. This plan will maintain water levels below the total capacity of the sumps and ponds, pumping to the pits until mine drainage volumes are down to manageable levels.

Under circumstances of design events exceeding sump and pond capacity or extended power outages during heavy rainfall, it is likely that the WWTF may also reach capacity and shut down the pumping network leading to it. In these circumstances, pumped mine drainage may be temporarily pumped into the pits, with mining operations in the lower levels temporarily shut down until water in the pit sumps are back to manageable levels. If the emergency operating procedure, as described above, is put into effect, mine drainage will be pumped to the pits, based on the level of reactivity of material stockpiled, following these priorities in descending order of reactivity and priority: OSP sump SOSP and overflow pond MD-SOSP; Category 4 Waste Rock stockpile sump S4 and overflow pond MD-S4; Category 2/3 Waste Rock Stockpile sumps and overflow ponds; RTH runoff pond MD-RTH; haul road runoff ponds MD-HRC, MD-HRE, MD-HRW, and MD-HRN; and OSLA runoff pond MD-OSLA. Pit dewatering may be temporarily



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stopped during these conditions to allow lowering of the water in these sumps and mine drainage ponds to manageable levels.

In the unlikely event of runoff exceeding the total design capacity of the sumps and ponds and containment under the emergency contingency plan is insufficient, overflows from the mine drainage areas will ultimately overflow into the Mine Site stormwater system, which ultimately flows off-site to the Partridge River.

Sump overflow ponds will contain sump overflow during events exceeding the 10-year, 24-hour storm up to the 100-year, 24-hour storm, but they will also receive direct precipitation during all rainfall events. Therefore these ponds will require periodic pumping because there is not a separate pumping system for these ponds. The pump system installed for the sumps will be used for periodic pumping of these overflow ponds to maintain capacity for the design storm.

After major storm events, stockpile sumps will be pumped down to their normal water levels to maintain storage volume for future storms, and then the overflow pond water will be conveyed to the WWTF, taking a lower priority over the stockpile sumps.

Overflow from the Category 1 Waste Rock Stockpile Groundwater Containment System is prevented by gravity overflow pipes from system sumps directly to the East and West Pits (Section 2.1.2 of Reference (3)).

4.4.2 Stormwater

Stormwater Sedimentation Ponds A and B will have their outlets fitted with check valves to prevent water from the Partridge River from flowing onto the Mine Site under flood conditions on the river. For each of the four exterior stormwater sedimentation ponds, both the primary outlet and an emergency outlet were design for each pond's specific conditions, with the primary outlet designed to provide flood attenuation and the secondary outlet designed to pass flows larger than the design values used to size the primary outlet. During large flood events in the headwaters of the Partridge River along the north side of the Mine Site, excess stormwater from Ponds A and B will be pumped to the stormwater ditch that flows south to Pond C or off-site using temporary portable pumps. If necessary, flood control pumping priorities will be given to the stormwater pond with potential to overflow into the mine drainage systems.



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5.0 Water Quantity and Quality Monitoring

Proper long-term management of water quality and quantity at the Mine Site will depend in part on a systematic monitoring plan that will be finalized in permitting. As operations proceed, the monitoring plan will be updated as required. Monitoring will be used to determine project compliance with permits, improve model accuracy, identify potential causes of changes to water quality or quantity, and identify options, if necessary, to adapt the Project to ensure short-term and long-term compliance. The proposed water quality and quantity monitoring plans that are associated with the various permits and regulations applicable to mining operations are being developed as part of each permit application process. The specifics of monitoring for the Project, including the specific locations, nomenclature, frequency, and parameters, have been outlined in the permit applications, and will be finalized during each applicable permitting process.



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6.0 Reporting and Adaptive Management

Adaptive management is a system of management practices based on clearly defined outcomes and monitoring requirements to determine if management actions are meeting the desired outcomes; and, if not, implementing changes that will best ensure that outcomes are met or reevaluated. Adaptive management recognizes the uncertainty associated with estimates based on natural systems as a result of the baseline monitoring data, waste characterization, scale of plan, decisions on modeling inputs and other limiting factors. Adaptive management measures will be developed through the Environmental Review process, permitting, and during operations, reclamation, and long-term closure to define when changes are needed to the proposed water management system.

A key component of adaptive management for water is the Adaptive Water Management Plan (Reference (4)) that describes adaptive engineering controls that manage water quality and quantity. Fixed engineering controls (liners, dikes, ditches, etc.) are described in this plan and other management plans. Contingency mitigations that could be applied if engineering controls do not manage water quality and quantity properly are also described in this document.

6.1 Monthly Reporting

The NPDES/SDS permit and the Water Appropriations permit will require and define routine water quality and quantity reporting and annual reports. The content required for those reports will be defined in those permits.

Routine water quality reports will be submitted to the MPCA, and monthly water quantity reports will be submitted to the MDNR. In addition to water quantity and quality monitoring described in Section 5.0, it is anticipated that routine reports will include:

- sulfur content of ore and waste rock placed in stockpiles
- monthly precipitation
- identification and explanation of variations from permit requirements, if any

6.2 Annual Reporting

An Annual NPDES/SDS Report will be submitted to the MPCA. It is anticipated that it will include:

 a comparison of actual mine pit and stockpile drainage water chemistry to the water chemistry estimated by the Project water quality model from start of operations through the past year



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- identification of any changes to the stockpile liners or groundwater containment system made during the last year
- a summary of any previously reported variations from permit requirements during the past year, if any
- identification of any changes to the stockpile liners or groundwater containment system planned for the coming year

An Annual PTM Report will be submitted to the MDNR. A draft version of the Annual PTM Report will be included with the PTM application and will include:

- the total tons of overburden and waste rock by type placed in stockpiles or mine pits from the start of operations through the past year and the remaining planned capacity
- the average sulfur content (based on the most recent block model) of waste rock placed in stockpiles, used for construction (by construction location), placed in the East Pit from the West and Central Pits and, to the extent practical, from temporary stockpiles (recognizing that the rock placed in the East Pit from temporary stockpiles will not be re-evaluated for sulfur content prior to pit disposal)
- the total tons of overburden and waste rock by type used for constructions from the start of operations through the past year and remaining planned applications
- a map showing where waste rock and overburden were placed and where vegetation was established for reclamation during the past year
- a map showing where overburden and waste rock are planned to be placed and where vegetation is planned to be established for reclamation during the coming year
- identification of any planned changes in operations that could impact final reclamation
- an update of the waste rock waste characterization program
- an update of any Special Performance Monitoring defined in Reference (4)
- an update on the results of any Test Projects defined in Reference (4)

An Annual Appropriations Report will be submitted to the MDNR. This appropriation report will include the monitoring data collected in accordance with the permit including:

• monthly records of the amount of water appropriated or used for each appropriation



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• total amount of water appropriated for the year

6.3 Annual Comparison to Model

Annual reports will include comparison of actual water quantity and quality to the quantity and quality estimated by the Project water quality model updated with the most recent monitoring data for the conditions existing at the time of the report.

6.4 Model Refinements

The Project water model developed in Reference (2) is an integrated model that includes all aspects of the Project. If the annual comparison of the model shows differences that can be logically explained as being caused by modeling assumptions that have been demonstrated to be incorrect, the model will be refined.

The adjusted model will be used to update the Project water quantity and quality estimates. If the update indicates that outcomes will not be acceptable, adaptive management will be initiated as described in Section 6.5.

6.5 Adaptive Management

There are adaptive management actions that could be implemented if there is an exceedance of a surface or groundwater standard detected as part of water quality monitoring or the water model projects a future exceedance of surface or groundwater standards given observed conditions. In general the steps will be:

- 1. Initiate any field studies that may be necessary to determine the root cause of the exceedance.
- 2. Once the root cause is identified, implement any adjustments that can be made to the adaptive engineering controls described in Reference (4) that will remedy the root cause. Adjustments to the adaptive engineering controls include changing the scale or type of control and its design.
- 3. If the modeled exceedances persist, implement contingency mitigation (Section 6.66.6) that will remedy the root cause and include that contingency mitigation as an adaptive engineering control in Reference (4).
- 4. Monitor and model effects to the environment with new or adjusted engineering control. If issue persists, begin step 1 again.

6.6 Contingency Mitigations

If monitoring or refined model estimates with adaptive engineering controls show that water quantity or quality at compliance points are projected to not meet compliance parameters, mitigations are available that would address the following situations. The contingency mitigations described in the following paragraphs are feasible but depend on site-specific



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conditions and do not include modifications to adaptive engineering controls which are described in Reference (4). These mitigations would be developed and designed if needed and coordinated with the MDNR and MPCA as appropriate.

- A. A pattern of overflows of the mine drainage sumps or ponds develop.
 - i. As described in Section 4.4, there is excess capacity designed as a safety factor in all the mine drainage sumps and ponds ranging from approximately 30 to 170% of required capacity. Additional capacity could be developed by expanding the pond areas.
- B. Streams along the railroad corridor between the Mine Site and Plant Site show degradation in water quality as a result of material spilled from the rail cars.
 - i. The PTM application will include details of the loading, unloading, and transport of ore and the engineering controls and procedures that will be used to minimize the potential of ore spillage along the railroad corridor. If degradation of water quality is found as a result of ore spillage, catchment areas could be developed adjacent to the tracks at stream crossings to further minimize the amount of spilled material that reaches the streams. Solids in the catchment areas would be removed and placed in the Process Plant, Category 4 Waste Rock Stockpile, or the East Pit.
- C. Groundwater downgradient of lined infrastructure or the mine pits has compliance issues.
 - i. Interception wells could collect groundwater flows impacted by a leak from one of the liner systems. Water collected by interception wells would be pumped to the WWTF or an approved non-mechanical treatment system for treatment. Because all liner systems at the Mine Site are for temporary infrastructure (temporary stockpiles, temporary ponds, etc.), the interception wells would only be needed while the liner is in use or until a liner repair could be performed.
 - ii. Interception wells could collect groundwater flows impacted by pit overflow into the surficial aquifer. Water collected by interception wells would be pumped to the WWTF or an approved non-mechanical treatment system for treatment.
- iii. Groundwater outflows from the pits could be contained by the use of injection grouting or grout curtains, as described in PolyMet's conceptual plan for bedrock groundwater flow mitigation (Reference (24)).
- D. West Pit water quality is worse than expected.
 - i. The contaminant load from the West Pit walls could be reduced by several methods:



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- a. Once the West Pit reaches its full water level, dams could be constructed in the low areas of the pit rim to raise the water level, decreasing the amount of exposed wall rock.
- b. A low permeability soil barrier could be constructed along the Ore Grade Material portions of the exposed pit wall such that the groundwater level rises in that area to an elevation above the top of the exposed Ore Grade Material. This would effectively create a wetland over this material, holding water over the exposed material, and limiting groundwater flow through the material.
- ii. The contaminant load to the West Pit from the East Pit could be reduced by several methods:
 - a. A low permeability soil barrier could be constructed along the Virginia Formation portion of the exposed pit wall such that the groundwater level rises in that area to an elevation above the top of the exposed Virginia Formation. This would effectively create a wetland over this material, holding water over the exposed material, and limiting groundwater flow through the material.
 - b. A Permeable Reactive Barrier (PRB) could be installed in the East Pit outlet channel to remove contaminates. Use of PRBs to remove sulfate, trace metals, and other dissolved or suspended constituents from water is described in detail in Section 6 of Reference (4).
 - c. The water leaving the East Pit could be pumped to the WWTF for treatment before flowing to the West Pit.
- iii. Injection grouting or grout curtains could be used to minimize groundwater inflows to the pits, as described in PolyMet's conceptual plan for bedrock groundwater flow mitigation (Reference (24)).
- iv. The West Pit water could be diluted by routing additional stormwater to the West Pit.
- v. The West Pit could be treated by several methods:
 - a. The West Pit water could be pumped to the WWTF, treated, and returned to the West Pit.
 - b. The West Pit lake could be treated in-situ with iron salts, fertilizer, or other methods tailored to the contaminant of concern. For example, Alexco is the industry leader for pit lake remediation and has technologies that have successfully treated billion gallon pit lakes for contaminants including selenium, zinc, uranium, and nitrate. Alexco's technologies have been successfully applied at numerous sites and locations and have demonstrated successful remediation.



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7.0 Reclamation and Long-Term Closure

Reclamation information included in this document is for the Mine Site water management systems only. This includes incremental reclamation, final reclamation, and long-term closure activities. Reclamation information for the mine pits is in Reference (1). Reclamation information for the stockpiles is in Reference (3). Reclamation information for other Mine Site infrastructure is included in Reference (6).

7.1 Incremental Reclamation

The Category 2/3 and 4 Waste Rock Stockpiles are temporary and, starting in Mine Year 11, the waste rock and Saturated Overburden materials in the stockpiles will be relocated to the East and Central Pits for ultimate disposal. The Category 4 Waste Rock Stockpile will be completely removed and dismantled in Mine Year 11, with stripping of the Central Pit occurring in that same year. The Category 2/3 Waste Rock Stockpile will be relocated to the East Pit starting in Mine Year 12 and continuing through Mine Year 19. Reclamation of the former temporary stockpile footprints will occur incrementally as large areas of the stockpile are removed to make it efficient to complete reclamation. This will include portions of the temporary Category 4 Waste Rock Stockpile that are outside the extent of the Central Pit and the entire footprint of the Category 2/3 Waste Rock Stockpile. Reclamation will also include removal of all piping, pump systems, and liner systems associated with the stockpile foundations and the stockpile sumps and ponds. Once these systems have been removed, the stockpile, sump, and pond footprints will be reclaimed into a mixture of upland and wetland areas, depending on the ultimate elevation of the remaining materials. Once reclamation in these areas is complete, the haul roads to these areas will also be scarified and seeded to allow continued access by small vehicles only for long-term monitoring.

7.2 Final Reclamation

Once mining operations in the West Pit are complete, final reclamation at the Mine Site will begin. During this time the West Pit will be flooded, as described in Section 6.2.6 of Reference (1) and Section 2.1.1 of Reference (4). Large Figure 10 shows infrastructure that will be removed or reclaimed during reclamation, and Large Figure 11 shows the Mine Site infrastructure that will remain for long-term closure.

7.2.1 Perimeter and Interior Dikes

The perimeter dike located north of the Central and East Pits will be maintained in order to minimize mixing of Partridge River flows with the East Pit water. Perimeter dikes located on the north side of the Category 1 Waste Rock Stockpile and along the west boundary of the Mine Site will be maintained to provide access to groundwater monitoring locations.

Most pit rim dikes will be removed. During reclamation, stormwater runoff within the Mine Site will be routed to the mine pits using a combination of existing and new ditches (Section 7.2.2). Some portions of the pit rim dikes will remain in place during reclamation if they are needed to



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prevent an uncontrolled discharge inflow to the pits and potential erosion (headcutting) of the pits walls. A more detailed evaluation of this requirement will be conducted prior to Mine Year 20.

Material will be removed from the main body of the dikes, except in locations noted above where dikes will be maintained, and will be used at the site for restoration of disturbed surfaces prior to reclamation. To minimize disturbance of subsurface soils, the subsurface seepage control component of the dikes will remain in place. Typical construction erosion control measures will be taken as part of the dike removal work, such as installing silt fence on the down slope side of disturbed areas and control of surface water runoff. The reclaimed surface will be scarified, topsoil placed, and the area will be revegetated, as described in PolyMet's Reclamation Seeding and Mulching Procedure (Attachment A of Reference (6)).

7.2.2 Ditch Filling and Rerouting

Large Figure 11 shows the proposed alignment of ditches that will be maintained to direct stormwater into the West Pit for flooding. Use of ditches that already exist in Mine Year 20 will be maximized, but a few new ditches may need to be constructed to direct stormwater runoff into the East or West Pits during reclamation. New ditches will be designed using the same criteria as other stormwater ditches at the Mine Site (Section 2.2.3). Reclamation of ditches will include either installing ditch blocks or filling, covering with topsoil, and vegetating the restored surface.

7.2.3 Stormwater and Mine Drainage Pond Restoration

At closure, the stormwater sedimentation ponds, the mine drainage ponds, and the remaining stockpile sumps and overflow ponds will be reclaimed by developing wetlands or by filling, covering with topsoil, and revegetating the area (Large Figure 10). Outlet control structures from Ponds A and B will remain in-place to prevent Partridge River floodwater from entering the Mine Site. Outlet control structures from Ponds C (East) and D will remain in-place to direct water under Dunka Road and the railroad to the Partridge River along natural drainage paths. The overflow weir in Pond C (West) will be modified to create a more natural transition to the remaining stormwater ditch.

The mine drainage sumps and ponds may require cleanout and removal of the geomembrane liner in closure. Material removed from the ponds will be disposed of in the pits or an approved landfill.

7.2.4 Pipe and Pump Removal

During reclamation, all mine drainage pipes and pumps will be removed and recycled or abandoned in place except those used for the flooding of the West Pit or recycling of the East or West Pit water.



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7.2.5 Central Pumping Station and Treated Water Pipeline Removal

Once no longer necessary, the CPS building will be reclaimed and vegetated according to Minnesota Rules, part 6132.2700 by a qualified reclamation contractor. The CPS pond will be reclaimed as a wetland or filled, covered with topsoil, and revegetated.

The TWP will be removed, recycled or disposed, or abandoned in place. The area disturbed by these activities will be revegetated.

7.3 Long-Term Closure

Monitoring, reporting, and water treatment will continue after the reclamation process is complete, until release from these activities is granted via the PTM. If any of the monitoring data shows that additional work is needed, a plan will be created and implemented to further improve water quality. During long-term closure, the water level in the West Pit will be maintained below the natural overflow elevation by discharging treated water to a small watercourse south of the West Pit that flows off-site to the Partridge River, as shown on Large Figure 11. The discharged water will have been pumped from the West Pit to the WWTF for treatment to meet the appropriate water discharge limits as described in Section 2.2 of Reference (4) prior to discharge. The ultimate objective is to transition from the mechanical treatment provided by the WWTF to a non-mechanical treatment system once the non-mechanical treatment system has been demonstrated to provide the required water treatment. Potential non-mechanical treatment systems, including construction of an outlet structure from the West Pit, are described in Section 6 of Reference (4).

7.3.1 Monitoring and Reporting

The monitoring and reporting described in Section 5.05.0 and Section 6.06.0 will continue until the MDNR releases the company from doing so under the PTM and the MPCA releases the company from doing so under the NPDES/SDS permit.

7.3.2 Water Treatment

As described in Section 2.0 of Reference (3), the Saturated Overburden and waste rock in the Category 2/3 and Category 4 waste rock stockpiles will be relocated to the East Pit. This will result in a flushing of oxidation products into the East Pit water. As the East and West Pits flood with water, oxidation products that have accumulated on the pit wall rock will be flushed into the pits as the water level rises.

The flushed oxidation products will be removed from the West and East Pits by pumping the pit water to the WWTF for treatment and returning the treated water to the pits. The potential for pit stratification in the West and East Pits is discussed in Section 6.1.3 of Reference (2).

For long-term closure, water treatment is expected to continue until the West Pit water quality reaches an acceptable level, as described in Section 2 of Reference (4). The WWTF will be maintained operable until MDNR releases the company from doing so under the PTM.



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7.4 Contingency Reclamation Estimates

The following section provides an overview of the contingency reclamation plan for Mine Year 0 and Mine Year 1. For more specific details on reclamation and the associated cost estimates, see the permit-level version of the Reclamation Plan with the contingency reclamation estimates that will be part of the PTM application.

7.4.1 Contingency Reclamation Plan (Mine Years 0 and 1)

7.4.1.1 Mine Year 0 (end of construction/development)

If closure were to occur at the end of Mine Year 0, there will be no waste rock in the stockpiles, no ore in the OSP, and no mine pits. Stripping of the East Pit will have begun. The stockpiles and OSP foundations will be the size shown in Large Figure 4. The WWTF will be of limited operability for treatment, consisting only of the East EQ Basin and Construction Water Treatment Building for treating construction water. The activities described in Section 7.2 and Section 7.3 will be implemented. Key parameters driving reclamation costs for water management systems are shown in Table 7-1, which will be developed based on the permit-level design for the contingency reclamation cost estimate (CRE) for the PTM application.

Table 7-1 Key Reclamation Cost Parameters

Key Parameter	Removal / Reclamation ⁽¹⁾	Construction ⁽¹⁾	Monitoring ⁽¹⁾	Treatment ⁽¹⁾
Stormwater Dikes	X	N/A	X	N/A
Stormwater Ditches	X	X	Х	N/A
Stormwater Ponds	Х	N/A	Х	N/A
Mine Drainage Ponds	Х	N/A	Х	N/A
Pipes	Х	N/A	N/A	N/A
WWTF	N/A	N/A	Х	Х

^{(1) &#}x27;X' denotes action required.

This plan is used to develop the Mine Year 0 Contingency Reclamation Estimate that will be the basis for financial assurance required by Minnesota Rules, part 6132.1200, which is required before a PTM can be granted.



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7.4.1.2 Mine Year 1 (end of first year of operations)

If closure were to occur at the end of Mine Year 1, the activities described in Section 7.2 and Section 7.3 will be implemented. Development of the Mine Site will be as shown in Large Figure 4 and Large Figure 7. Only the East Pit will be developed, with stripping of the West Pit not yet started. Key parameters driving reclamation costs for water management systems are shown in Table 7-2, which will be developed based on the permit-level design for the contingency reclamation cost estimate for the PTM application.

Table 7-2 Key Reclamation Cost Parameters

Key Parameter	Removal / Reclamation ⁽¹⁾	Construction ⁽¹⁾	Monitoring ⁽¹⁾	Treatment ⁽¹⁾		
Stormwater Dikes	Х	X N/A		N/A		
Stormwater Ditches	X	Х	х	N/A		
Stormwater Ponds	Х	N/A	Х	N/A		
Mine Drainage Ponds	X	N/A	х	N/A		
Pipes	Х	N/A	N/A	N/A		
WWTF	N/A	N/A	Х	Х		

^{(1) &#}x27;X' denotes activity required.

This plan will be used to develop the contingency reclamation estimate that will be the basis for financial assurance required by Minnesota Rules, part 6132.1200 the first or second calendar year (depending on construction progress) after the issuance of the PTM. The Reclamation Plan and contingency reclamation estimate will be updated annually to include contingency reclamation for the site conditions representative of the end of the upcoming year of operation.



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Revision History

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11/30/2011	1	Initial release
01/09/2013	2	Significant changes to incorporate project changes related to the decisions made in the AWMP Version 4 and 5. These project changes include the extension of the groundwater containment system along the south side of the stockpile, the use of a geomembrane cover on the Category 1 Waste Rock Stockpile, the use of long-term mechanical treatment and the potential for non-mechanical treatment in long-term closure.
12/31/2014	3	Changes were made for clarification, to address agency comments (Sections 1.0, 1.1, 1.2, 1.3.2, 1.4, 2.1.1, 2.1.4.1.1, 2.1.4.1.2, 2.1.5.1, 2.1.7, 2.2.4.1, 4.0, 4.1, 4.2, 4.4.1, 4.4.2, 5.0, 5.1.6, 5.3, 6.0, 6.2, 6.3, 6.5, 6.6, 7.2.1), to incorporate minor design changes and project refinements (Sections 2 and 4), and to incorporate the results of water modeling (Section 3).
03/09/2015	4	Minor changes were made to address agency comments (Sections 1.0, 1.2, 1.3.1, 1.3.2, 2.1, 2.1.5.1, 2.1.6, 3.1, 5.0, 5.1.1, 5.1.2, 5.1.3, 5.1.3, 5.1.4, 5.1.5, 5.5.2, 6.1, 6.2, 6.6, and 7.2.3, Large Table 8, and Large Figure 12).
07/11/2016	5	Certification page added, minor changes made to Section 7 tables for clarity, minor changes made to Large Figures to account for changes to the WWTF footprint, permit application support drawings added to Attachments A and B, reference to Attachments C, D, and E were deleted from this document.



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Large Table 1 **Estimated West Pit Lake Water Quality**

Constituent	Mine Year		Mine Year 25 ⁽¹⁾		Mine Year 55 ^{(1), (3)}			Mine Year 75 ^{(1), (4)}			
	Percentile	Surface Water Quality	Average	Average	Average	Average	Average	Average	Average	Average	Average
	Units	Standard ⁽¹⁾	P10 ⁽²⁾	P50 ⁽²⁾	P90 ⁽²⁾	P10 ⁽²⁾	P50 ⁽²⁾	P90 ⁽²⁾	P10 ⁽²⁾	P50 ⁽²⁾	P90 ⁽²⁾
Ag (Silver)	μg/L	1	0.20	0.20	0.20	0.20	0.20	0.20	0.20	0.20	0.20
Al (Aluminum)	μg/L	125	0.93	1.41	2.15	0.93	1.41	2.15	0.93	1.41	2.15
Alkalinity	mg/L		34.82	39.69	51.12	34.82	39.69	51.12	34.82	39.69	51.12
As (Arsenic)	μg/L	53	10.88	13.59	18.77	8.88	10.81	14.88	7.98	9.72	13.32
B (Boron)	μg/L	500	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Ba (Barium)	μg/L		28.13	30.30	31.29	28.12	36.05	38.22	26.95	36.88	39.50
Be (Beryllium)	μg/L		0.40	0.40	0.40	0.35	0.40	0.40	0.32	0.40	0.40
Ca (Calcium)	mg/L		49.34	54.98	70.72	34.31	38.74	49.46	30.80	34.72	44.77
Cd (Cadmium) ⁽⁵⁾	μg/L	2.6	1.16	1.89	4.72	0.77	1.50	3.88	0.68	1.41	3.63
CI (Chloride)	mg/L	230	12.53	13.42	15.41	8.16	10.64	14.99	7.29	9.54	13.35
Co (Cobalt)	μg/L	5	21.90	52.83	113.41	17.21	38.25	80.09	15.47	33.99	73.49
Cr (Chromium)	μg/L	11	3.86	4.00	4.14	2.37	2.57	2.91	2.10	2.29	2.59
Cu (Copper) ⁽⁵⁾	μg/L	9.8	118.98	236.74	656.69	118.98	236.74	654.58	118.98	236.74	654.58
F (Fluoride)	mg/L		0.28	0.32	0.35	0.20	0.25	0.31	0.18	0.22	0.27
Fe (Iron)	μg/L		45.83	59.87	199.13	45.83	59.87	199.13	45.83	59.87	199.13
K (Potassium)	mg/L		12.83	14.19	17.31	7.96	10.00	13.71	7.06	8.85	12.05
Mg (Magnesium)	mg/L		32.72	35.27	39.74	14.42	15.84	18.88	12.98	14.27	16.94
Mn (Manganese)	μg/L		191.36	232.02	311.95	126.32	146.00	198.30	121.08	139.69	188.04
Na (Sodium)	mg/L		43.69	50.32	63.67	25.82	34.11	52.18	23.12	30.32	46.09
Ni (Nickel) ⁽⁵⁾	μg/L	54.6	330.95	683.53	1466.24	269.24	488.73	902.46	241.09	433.22	801.56
Pb (Lead) ⁽⁵⁾	μg/L	3.4	7.62	11.46	20.93	5.61	7.87	11.89	5.10	7.06	10.55
Sb (Antimony)	μg/L	31	9.90	11.21	13.67	6.37	7.68	9.56	5.64	6.83	8.50
Se (Selenium)	μg/L	5	0.25	2.71	3.40	0.25	1.78	2.41	0.25	1.61	2.17
SO4 (Sulfate)	mg/L		95.11	104.71	124.55	48.35	56.77	68.54	43.15	50.86	61.90
TI (Thallium)	μg/L	0.56	0.11	0.12	0.13	0.08	0.08	0.09	0.07	0.08	0.09
V (Vanadium)	μg/L		10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00
Zn (Zinc) ⁽⁵⁾	μg/L	125.4	86.02	115.60	274.24	63.58	98.58	229.45	57.38	95.73	221.91

⁽¹⁾ Surface water quality standard only applies at overflow, and therefore is only compared to Mine Years 55 and 75; values for those years above the applicable water quality standard are shown in bold with light red shading.
(2) Values shown are the average of the monthly P10, P50, and P90 values, as indicated, for the referenced Mine Year; see Section 6.2 of Reference (2).
(3) Flooding of the West Pit has a 90% probability of being complete by the end of Mine Year 55.
(4) Concentrations in the West Pit are typically stable or trending downward in long-term closure.
(5) Standard is hardness-based and evaluated at a hardness of 105.5 mg/L; see Section 6.5.3 of Reference (2) for surface water monitoring station SW004a, which is downstream of the West Pit discharge.

Estimated Surficial Groundwater Quality along the East Pit-Category 2/3 Stockpile Flow Path at the Property Boundary Large Table 2

Ag (Silver) Al (Aluminum) ⁽³⁾ Alkalinity As (Arsenic)	Percentile Units μg/L μg/L mg/L μg/L	Water Quality Standard 30 	Average P10 ⁽¹⁾ 0.11 49.64	Average P50 ⁽¹⁾	Average P90 ⁽¹⁾	Average P10 ⁽¹⁾	Average	Average		1						Ι	
Ag (Silver) Al (Aluminum) ⁽³⁾ Alkalinity As (Arsenic)	μg/L μg/L mg/L	30	0.11			Pittin	DEA(1)		Average								
Al (Aluminum) ⁽³⁾ Alkalinity As (Arsenic)	μg/L mg/L			0.11			P50 ⁽¹⁾	P90 ⁽¹⁾	P10 ⁽¹⁾	P50 ⁽¹⁾	P90 ⁽¹⁾	P10 ⁽¹⁾	P50 ⁽¹⁾	P90 ⁽¹⁾	P10 ⁽¹⁾	P50 ⁽¹⁾	P90 ⁽¹⁾
Alkalinity As (Arsenic)	mg/L		49.64		0.11	0.11	0.12	0.14	0.11	0.11	0.11	0.11	0.12	0.13	0.11	0.12	0.13
As (Arsenic)	_		10.01	54.05	58.90	72.17	134.72	335.31	49.89	55.17	70.97	43.76	49.95	56.02	42.94	47.52	53.35
` ,	μg/L		61.71	64.55	67.52	61.69	64.53	67.51	61.57	64.45	67.49	57.04	61.87	65.82	55.70	60.17	64.50
!		10	0.67	0.72	0.76	0.67	0.72	0.76	0.67	0.72	0.76	0.67	0.72	0.76	0.67	0.72	0.76
B (Boron)	μg/L	1,000	26.60	27.03	27.47	26.67	27.12	27.59	26.63	27.10	27.71	26.92	27.95	29.74	26.96	27.69	28.89
Ba (Barium)	μg/L	2,000	29.41	31.36	33.44	29.38	31.35	33.43	29.41	31.35	33.43	28.71	31.60	34.69	26.57	30.32	34.59
Be (Beryllium) ⁽⁴⁾	μg/L	0.45	0.12	0.12	0.12	0.12	0.12	0.13	0.12	0.12	0.12	0.12	0.13	0.15	0.12	0.13	0.15
Ca (Calcium)	mg/L		14.84	15.45	16.09	15.09	15.80	16.65	14.89	15.56	16.37	14.35	15.63	17.21	13.96	14.91	16.29
Cd (Cadmium)	μg/L	4	0.10	0.10	0.10	0.11	0.12	0.19	0.10	0.10	0.13	0.10	0.14	0.28	0.11	0.13	0.20
CI (Chloride)	mg/L	250	0.62	0.65	0.69	0.62	0.66	0.70	0.62	0.66	0.98	0.69	1.34	2.71	0.82	0.99	2.13
Co (Cobalt)	μg/L		0.79	0.87	0.98	1.19	2.76	10.42	0.80	0.92	2.20	0.94	1.78	3.48	1.06	1.52	2.73
Cr (Chromium)	μg/L	100	0.89	0.93	0.98	0.90	0.94	0.99	0.89	0.94	0.99	0.84	0.94	1.03	0.81	0.89	0.99
Cu (Copper)	μg/L		2.30	2.49	2.69	2.30	2.49	2.69	2.30	2.49	2.69	2.31	2.49	2.69	2.32	2.51	2.70
F (Fluoride)	mg/L	2	0.07	0.07	0.08	0.07	0.07	0.08	0.07	0.07	0.08	0.07	0.08	0.11	0.07	0.07	0.10
Fe (Iron) ⁽³⁾	μg/L		1,217.80	1,427.00	1,673.30	1,235.30	1,451.00	1,714.00	1,215.80	1,423.70	1,674.80	1,109.20	1,318.10	1,562.20	1,077.80	1,273.30	1,496.40
K (Potassium)	mg/L		1.63	1.69	1.74	1.66	1.72	1.78	1.64	1.70	1.98	1.64	2.33	3.75	1.55	1.81	3.03
Mg (Magnesium)	mg/L		6.67	6.96	7.26	6.82	7.14	7.70	6.71	7.00	7.36	6.49	7.01	7.63	6.33	6.72	7.27
Mn (Manganese) ^{(3),(4)}	μg/L	1,002	477.87	551.07	635.44	485.10	558.64	644.69	477.65	550.89	635.37	436.35	516.97	599.18	423.87	497.62	576.79
Na (Sodium)	mg/L		5.12	5.35	5.57	5.20	5.44	5.70	5.16	5.40	6.42	5.19	7.50	12.75	4.88	5.71	10.06
Ni (Nickel)	μg/L	100	1.86	2.01	2.18	1.86	2.01	2.18	1.86	2.02	2.18	1.88	2.04	2.23	1.91	2.08	2.35
Pb (Lead)	μg/L		0.57	0.61	0.65	0.62	0.68	0.85	0.57	0.62	0.66	0.60	0.72	0.85	0.64	0.78	0.86
Sb (Antimony)	μg/L	6	0.25	0.25	0.25	0.25	0.25	0.26	0.25	0.26	0.32	0.26	0.28	0.35	0.26	0.27	0.33
Se (Selenium)	μg/L	30	0.52	0.52	0.53	0.53	0.56	0.64	0.52	0.53	0.56	0.50	0.55	0.65	0.49	0.51	0.61
SO4 (Sulfate)	mg/L	250	9.15	9.62	10.15	10.01	11.30	15.84	9.19	9.83	11.37	9.39	11.35	15.76	9.17	10.09	13.71
TI (Thallium)	μg/L	0.6	0.12	0.12	0.13	0.12	0.12	0.13	0.12	0.12	0.13	0.11	0.12	0.13	0.11	0.12	0.12
V (Vanadium)	μg/L	50	3.50	3.62	3.75	3.52	3.64	3.77	3.50	3.64	3.78	3.59	4.08	4.61	3.49	4.20	4.66
Zn (Zinc)	μg/L	2,000	4.21	4.36	4.54	4.72	6.22	12.06	4.24	4.46	6.62	4.55	9.19	18.24	5.81	8.49	14.68

NOTE: Values above the applicable water quality standard are shown in bold with light red shading.
(1) Values shown are the average of the monthly P10, P50, and P90 values, as indicated, for the referenced Mine Year; see Section 6.3 of Reference (2).
(2) Model runs evaluated through Mine Year 200.

⁽³⁾ Not evaluated against the secondary groundwater standard.(4) Evaluated against the site-specific evaluation criteria shown.

Estimated Surficial Groundwater Quality along the West Pit Flow Path at the Property Boundary Large Table 3

	Mine Year		Mine Year 50			1	Mine Year 7	5	M	ine Year 15	0	Mine Year 200 ⁽²⁾			
	Percentile	Water Quality	Average	Average	Average										
Constituent	Units	Standard	P10 ⁽¹⁾	P50 ⁽¹⁾	P90 ⁽¹⁾	P10 ⁽¹⁾	P50 ⁽¹⁾	P90 ⁽¹⁾	P10 ⁽¹⁾	P50 ⁽¹⁾	P90 ⁽¹⁾	P10 ⁽¹⁾	P50 ⁽¹⁾	P90 ⁽¹⁾	
Ag (Silver)	μg/L	30	0.11	0.11	0.11	0.11	0.11	0.13	0.14	0.15	0.16	0.15	0.16	0.16	
Al (Aluminum) ⁽³⁾	μg/L		49.54	53.99	58.87	43.07	52.63	58.21	25.16	28.14	36.09	24.85	27.62	30.69	
Alkalinity	mg/L		61.71	64.55	67.52	59.57	63.79	67.09	49.38	53.01	59.06	49.10	52.33	57.77	
As (Arsenic)	μg/L	10	0.67	0.72	0.76	0.67	0.72	0.76	0.67	0.72	0.76	0.67	0.72	0.76	
B (Boron)	μg/L	1,000	26.59	27.03	27.47	26.70	27.41	44.44	52.40	63.08	65.67	55.42	61.52	65.13	
Ba (Barium)	μg/L	2,000	29.41	31.36	33.44	29.55	31.62	33.62	27.25	33.67	36.60	24.74	33.32	38.14	
Be (Beryllium) ⁽⁴⁾	μg/L	0.45	0.12	0.12	0.12	0.12	0.12	0.18	0.19	0.24	0.27	0.17	0.22	0.26	
Ca (Calcium)	mg/L		14.82	15.43	16.07	14.98	15.81	21.26	20.35	22.60	27.00	17.80	20.03	24.11	
Cd (Cadmium)	μg/L	4	0.10	0.10	0.10	0.10	0.11	0.47	0.32	0.64	1.73	0.28	0.56	1.73	
CI (Chloride)	mg/L	250	0.62	0.65	0.69	0.63	0.68	3.26	3.08	4.27	5.80	2.70	3.55	5.06	
Co (Cobalt)	μg/L		0.77	0.85	0.94	0.80	0.96	10.40	6.34	14.11	30.82	5.80	11.40	26.67	
Cr (Chromium)	μg/L	100	0.89	0.93	0.98	0.90	0.96	1.35	1.29	1.44	1.60	1.12	1.23	1.47	
Cu (Copper)	μg/L		2.30	2.49	2.69	2.30	2.49	2.69	2.30	2.49	2.69	2.30	2.49	2.69	
F (Fluoride)	mg/L	2	0.07	0.07	0.08	0.07	0.07	0.12	0.11	0.13	0.16	0.10	0.11	0.14	
Fe (Iron) ⁽³⁾	μg/L		1,217.80	1,427.10	1,673.30	1,089.10	1,376.70	1,652.40	642.89	778.48	1,029.80	634.53	753.62	913.80	
K (Potassium)	mg/L		1.63	1.69	1.74	1.65	1.73	3.70	3.62	4.53	5.82	3.06	3.82	5.11	
Mg (Magnesium)	mg/L		6.67	6.95	7.25	6.72	7.11	9.39	8.77	9.68	10.96	7.76	8.52	9.94	
Mn (Manganese) ^{(3),(4)}	μg/L	1,002	477.78	551.04	635.42	446.63	532.35	621.33	307.76	350.13	429.52	297.51	337.82	387.57	
Na (Sodium)	mg/L		5.12	5.34	5.57	5.18	5.49	12.72	11.68	15.12	22.23	10.00	12.80	18.37	
Ni (Nickel)	μg/L	100	1.86	2.01	2.18	1.86	2.01	2.18	1.86	2.01	2.18	1.86	2.01	2.18	
Pb (Lead)	μg/L		0.57	0.61	0.65	0.58	0.65	2.35	2.34	3.23	4.88	2.10	2.76	4.07	
Sb (Antimony)	μg/L	6	0.25	0.25	0.25	0.25	0.25	0.25	0.25	0.25	0.25	0.25	0.25	0.27	
Se (Selenium)	μg/L	30	0.51	0.52	0.53	0.51	0.53	0.77	0.41	0.94	1.18	0.39	0.82	1.07	
SO4 (Sulfate)	mg/L	250	9.11	9.58	10.08	9.25	10.00	21.62	21.60	26.19	31.65	18.74	22.15	28.46	
TI (Thallium)	μg/L	0.6	0.12	0.12	0.13	0.11	0.12	0.13	0.09	0.10	0.11	0.09	0.09	0.10	
V (Vanadium)	μg/L	50	3.50	3.62	3.75	3.53	3.69	5.17	5.79	6.77	7.02	6.57	6.82	7.03	
Zn (Zinc)	μg/L	2,000	4.18	4.34	4.50	4.26	4.57	29.09	25.42	45.04	105.51	22.13	42.11	106.05	

NOTE: Values above the applicable water quality standard are shown in bold with light red shading.

(1) Values shown are the average of the monthly P10, P50, and P90 values, as indicated, for the referenced Mine Year; see Section 6.3 of Reference (2).

(2) Model runs evaluated through Mine Year 200.

⁽³⁾ Not evaluated against the secondary groundwater standard.(4) Evaluated against the site-specific evaluation criteria shown.

Estimated Surface Water Quality for the Partridge River at SW004 Large Table 4

Constituent Ag (Silver) Al (Aluminum) Alkalinity As (Arsenic) B (Boron) Ba (Barium) Be (Beryllium) Ca (Calcium) Cd (Cadmium)(3) Cl (Chloride) Cr (Chromium) Cu (Copper)(3) F (Fluoride) Mg (Magnesium) Mg/L Mg (Magnesium) Percentil Units pg/L pg/L	Standar 1 125 53 500 2.5 230		Average P50 ⁽¹⁾ 0.11 70.35 73.28 0.88 81.35 9.54 0.10	Average P90 ⁽¹⁾ 0.11 244.71 129.93 2.36 162.27 18.12	Average P10 ⁽¹⁾ 0.10 30.78 24.53 0.20 17.75	Average P50 ⁽¹⁾ 0.11 70.81 72.90 0.88	Average P90 ⁽¹⁾ 0.11 246.21 128.45	Average P10 ⁽¹⁾ 0.10 44.88 19.11	Average P50 ⁽¹⁾ 0.10 96.38	Average P90 ⁽¹⁾ 0.11 254.55	Average P10 ⁽¹⁾ 0.10 41.59	Average P50 ⁽¹⁾ 0.10 95.87	Average P90 ⁽¹⁾ 0.11 257.23	Average P10 ⁽¹⁾ 0.10 43.47	Average P50 ⁽¹⁾ 0.10 94.89	Average P90 ⁽¹⁾ 0.11 270.05
Ag (Silver) Al (Aluminum) Alkalinity As (Arsenic) B (Boron) Ba (Barium) Be (Beryllium) Ca (Calcium) Cd (Cadmium)(3) Cl (Chloride) Co (Cobalt) Cr (Chromium) Cu (Copper)(3) F (Fluoride) Mg/L K (Potassium) Mg/L Mg (Magnesium) Mg/L Mg/L	1 125 53 500 2.5 230	0.10 30.09 25.47 0.20 18.86 2.67 0.10 6.99	0.11 70.35 73.28 0.88 81.35 9.54 0.10	0.11 244.71 129.93 2.36 162.27	0.10 30.78 24.53 0.20	0.11 70.81 72.90 0.88	0.11 246.21 128.45	0.10 44.88	0.10 96.38	0.11	0.10	0.10	0.11	0.10	0.10	0.11
Al (Aluminum) µg/L Alkalinity mg/L As (Arsenic) µg/L B (Boron) µg/L Ba (Barium) µg/L Be (Beryllium) µg/L Ca (Calcium) mg/L Cd (Cadmium)(3) µg/L Cl (Chloride) mg/L Co (Cobalt) µg/L Cr (Chromium) µg/L Cu (Copper)(3) µg/L F (Fluoride) mg/L Fe (Iron) µg/L K (Potassium) mg/L Mg (Magnesium) mg/L	 53 500 2.5 230	30.09 25.47 0.20 18.86 2.67 0.10 6.99	70.35 73.28 0.88 81.35 9.54 0.10	244.71 129.93 2.36 162.27	30.78 24.53 0.20	70.81 72.90 0.88	246.21 128.45	44.88	96.38							
Alkalinity mg/L As (Arsenic) µg/L B (Boron) µg/L Ba (Barium) µg/L Be (Beryllium) µg/L Ca (Calcium) mg/L Cd (Cadmium)(3) µg/L Cl (Chloride) mg/L Co (Cobalt) µg/L Cr (Chromium) µg/L F (Fluoride) mg/L Fe (Iron) µg/L K (Potassium) mg/L Mg (Magnesium) mg/L	 53 500 2.5 230	25.47 0.20 18.86 2.67 0.10 6.99	73.28 0.88 81.35 9.54 0.10	129.93 2.36 162.27	24.53 0.20	72.90 0.88	128.45			254.55	41.59	95.87	257.23	43.47	94.89	270.05
As (Arsenic) µg/L B (Boron) µg/L Ba (Barium) µg/L Be (Beryllium) µg/L Ca (Calcium) mg/L Cd (Cadmium)(3) µg/L Cl (Chloride) mg/L Co (Cobalt) µg/L Cr (Chromium) µg/L Cu (Copper)(3) µg/L F (Fluoride) mg/L Fe (Iron) µg/L K (Potassium) mg/L Mg (Magnesium) mg/L	53 500 2.5 230	0.20 18.86 2.67 0.10 6.99	0.88 81.35 9.54 0.10	2.36 162.27	0.20	0.88		19.11	- 4 00							210.00
B (Boron) µg/L Ba (Barium) µg/L Be (Beryllium) µg/L Ca (Calcium) mg/L Cd (Cadmium)(3) µg/L Cl (Chloride) mg/L Co (Cobalt) µg/L Cr (Chromium) µg/L Cu (Copper)(3) µg/L F (Fluoride) mg/L Fe (Iron) µg/L K (Potassium) mg/L Mg (Magnesium) mg/L	500 2.5 230	18.86 2.67 0.10 6.99	81.35 9.54 0.10	162.27			0.40		54.26	145.87	18.43	53.13	129.76	19.06	52.69	130.10
Ba (Barium) µg/L Be (Beryllium) µg/L Ca (Calcium) mg/L Cd (Cadmium)(3) µg/L Cl (Chloride) mg/L Co (Cobalt) µg/L Cr (Chromium) µg/L Cu (Copper)(3) µg/L F (Fluoride) mg/L Fe (Iron) µg/L K (Potassium) mg/L Mg (Magnesium) mg/L	 2.5 230	2.67 0.10 6.99	9.54 0.10		17.75		2.13	0.13	0.56	2.17	0.12	0.56	2.50	0.12	0.56	2.19
Be (Beryllium) µg/L Ca (Calcium) mg/L Cd (Cadmium)(3) µg/L Cl (Chloride) mg/L Co (Cobalt) µg/L Cr (Chromium) µg/L Cu (Copper)(3) µg/L F (Fluoride) mg/L Fe (Iron) µg/L K (Potassium) mg/L Mg (Magnesium) mg/L	2.5 230	0.10 6.99	0.10	18.12		81.37	195.40	9.95	36.47	158.70	9.89	35.92	161.46	10.71	36.56	192.83
Ca (Calcium) mg/L Cd (Cadmium) ⁽³⁾ μg/L Cl (Chloride) mg/L Co (Cobalt) μg/L Cr (Chromium) μg/L Cu (Copper) ⁽³⁾ μg/L F (Fluoride) mg/L Fe (Iron) μg/L K (Potassium) mg/L Mg (Magnesium) mg/L	2.5 230	6.99			2.66	9.68	18.30	2.09	9.30	27.59	2.12	9.39	27.91	2.01	9.30	27.85
Cd (Cadmium) ⁽³⁾ µg/L Cl (Chloride) mg/L Co (Cobalt) µg/L Cr (Chromium) µg/L Cu (Copper) ⁽³⁾ µg/L F (Fluoride) mg/L Fe (Iron) µg/L K (Potassium) mg/L Mg (Magnesium) mg/L	2.5		40.5:	0.10	0.10	0.10	0.10	0.10	0.10	0.12	0.10	0.10	0.12	0.10	0.10	0.12
CI (Chloride) mg/L Co (Cobalt) µg/L Cr (Chromium) µg/L Cu (Copper)(3) µg/L F (Fluoride) mg/L Fe (Iron) µg/L K (Potassium) mg/L Mg (Magnesium) mg/L	230	0.03	19.94	33.58	6.76	20.03	33.39	5.10	13.71	33.37	5.01	13.50	33.05	5.18	13.77	31.95
Co (Cobalt) µg/L Cr (Chromium) µg/L Cu (Copper) ⁽³⁾ µg/L F (Fluoride) mg/L Fe (Iron) µg/L K (Potassium) mg/L Mg (Magnesium) mg/L		1	0.08	0.14	0.03	0.08	0.14	0.02	0.07	0.15	0.02	0.07	0.15	0.02	0.07	0.15
$ \begin{array}{cccc} Cr \ (Chromium) & \mu g/L \\ Cu \ (Copper)^{(3)} & \mu g/L \\ F \ (Fluoride) & mg/L \\ Fe \ (Iron) & \mu g/L \\ K \ (Potassium) & mg/L \\ Mg \ (Magnesium) & mg/L \\ \end{array} $	-	1.69	8.29	15.00	1.72	8.33	16.26	0.64	2.45	13.68	0.61	2.51	15.06	0.59	2.37	13.58
Cu (Copper) ⁽³⁾ µg/L F (Fluoride) mg/L Fe (Iron) µg/L K (Potassium) mg/L Mg (Magnesium) mg/L	5	0.06	0.32	0.89	0.06	0.33	0.83	0.04	0.38	1.27	0.05	0.38	1.09	0.05	0.38	0.98
F (Fluoride) mg/L Fe (Iron) µg/L K (Potassium) mg/L Mg (Magnesium) mg/L	11	0.23	0.57	1.15	0.24	0.57	1.20	0.20	0.63	1.25	0.20	0.63	1.29	0.23	0.63	1.34
Fe (Iron) μg/L K (Potassium) mg/L Mg (Magnesium) mg/L	9.5	0.44	1.15	2.68	0.41	1.17	2.79	0.41	1.43	3.11	0.43	1.43	3.15	0.39	1.42	3.20
K (Potassium) mg/L Mg (Magnesium) mg/L		0.03	0.09	0.18	0.03	0.09	0.18	0.02	0.07	0.20	0.02	0.07	0.19	0.02	0.06	0.18
Mg (Magnesium) mg/L		549.50	1,838.05	4,718.10	551.61	1,844.31	4,406.60	441.44	1,467.87	5,267.90	396.27	1,453.83	4,813.50	461.71	1,433.34	4,737.80
		0.59	2.05	3.59	0.58	2.03	3.63	0.37	1.32	3.83	0.40	1.38	5.03	0.43	1.35	3.96
		4.19	10.27	14.56	4.31	10.17	14.50	3.23	6.64	14.15	3.28	6.72	12.77	3.32	6.73	13.82
Mn (Manganese) μg/L		13.94	131.67	277.54	15.31	132.79	281.62	9.24	141.46	498.47	8.45	141.73	504.29	8.53	141.72	502.25
Na (Sodium) mg/L		1.31	6.69	12.17	1.24	6.73	11.45	0.56	3.23	10.53	0.62	3.30	13.07	0.62	3.25	11.29
Ni (Nickel) ⁽³⁾ µg/L	53.3	0.24	0.93	2.56	0.25	0.95	2.97	0.20	1.12	3.28	0.19	1.13	3.37	0.22	1.14	3.28
Pb (Lead) ⁽³⁾ µg/L	3.3	0.03	0.23	0.58	0.03	0.23	0.61	0.02	0.23	0.79	0.02	0.23	0.74	0.02	0.24	0.77
Sb (Antimony) μg/L	31	0.25	0.25	0.25	0.25	0.25	0.25	0.25	0.25	0.26	0.25	0.25	0.26	0.25	0.25	0.26
Se (Selenium) µg/L	5	0.23	0.65	1.20	0.23	0.65	1.19	0.17	0.48	1.44	0.18	0.49	1.30	0.18	0.48	1.25
SO4 (Sulfate) mg/L		3.86	15.11	26.25	4.03	15.03	26.24	1.95	7.02	15.90	2.35	6.94	16.49	2.21	6.92	14.80
TI (Thallium) µg/L	0.56	0.00	0.03	0.06	0.00	0.03	0.06	0.00	0.03	0.11	0.00	0.03	0.11	0.00	0.03	0.11
V (Vanadium) μg/L		1.01	1.41	2.01	1.01	1.41	2.03	0.99	1.59	3.26	1.00	1.60	3.32	1.00	1.60	3.33
Zn (Zinc) ⁽³⁾ µg/L	122.3	1.31	5.03	19.41	1.37	5.08	17.34	1.06	4.30	20.97	0.91	4.40	21.88	1.04	4.28	19.31
Hardness mg/L	500	43.57	93.40	128.18	41.55	93.89	128.38	35.42	65.27	121.94	34.64	64.59	121.91	36.63	65.25	119.64

NOTE: Values above the applicable water quality standard are shown in bold with light red shading.
(1) Values shown are the average of the monthly P10, P50, and P90 values, as indicated, for the referenced Mine Year; see Section 6.5 of Reference (2).
(2) Model runs evaluated through Mine Year 200.
(3) Standard is hardness-based and evaluated at a hardness of 102.5 mg/L; see Section 6.5.3 of Reference (2).

Estimated Surface Water Quality for the Partridge River at SW004a Large Table 5

Constituent Water Quality Standard Average P10(1) Ag (Silver) μg/L 1 0.10 Al (Aluminum) μg/L 125 32.96 Alkalinity mg/L 21.32 As (Arsenic) μg/L 53 0.15 B (Boron) μg/L 500 12.92 Ba (Barium) μg/L 2.27 Be (Beryllium) μg/L 0.10 Ca (Calcium) mg/L 5.72 Cd (Cadmium)(3) μg/L 2.6 0.03 Cl (Chloride) mg/L 230 1.12 Co (Cobalt) μg/L 5 0.05 Cr (Chromium) μg/L 5 0.05 Cr (Chromium) μg/L 9.8 0.40 F (Fluoride) mg/L 0.02 Fe (Iron) μg/L 464.80 K (Potassium) mg/L 3.71 Mn (Manganese) μg/L <t< th=""><th>Mine Year 20</th><th>Mine Year</th><th>N</th><th>line Year 5</th><th>5</th><th>N</th><th>line Year 75</th><th></th><th colspan="3">Mine Year 125</th><th colspan="4">Mine Year 200⁽²⁾</th></t<>	Mine Year 20	Mine Year	N	line Year 5	5	N	line Year 75		Mine Year 125			Mine Year 200 ⁽²⁾			
Ag (Silver) μg/L 1 0.10 Al (Aluminum) μg/L 125 32.96 Alkalinity mg/L 21.32 As (Arsenic) μg/L 53 0.15 B (Boron) μg/L 500 12.92 Ba (Barium) μg/L 0.10 Ca (Calcium) μg/L 0.10 Ca (Calcium) mg/L 5.72 Cd (Cadmium)(3) μg/L 2.6 0.03 Cl (Chloride) mg/L 230 1.12 Co (Cobalt) μg/L 5 0.05 Cr (Chromium) μg/L 5 0.05 Cr (Chromium) μg/L 9.8 0.40 F (Fluoride) mg/L 0.02 Fe (Iron) μg/L 0.02 Fe (Iron) μg/L 0.46 Mg (Magnesium) mg/L 0.46 Mg (Magnesium) mg/L 0.87 <tr< th=""><th>Average P50⁽¹⁾</th><th>Standard</th><th>Average P90⁽¹⁾</th><th>Average P10⁽¹⁾</th><th>Average P50⁽¹⁾</th><th>Average P90⁽¹⁾</th><th>Average P10⁽¹⁾</th><th>Average P50⁽¹⁾</th><th>Average P90⁽¹⁾</th><th>Average P10⁽¹⁾</th><th>Average P50⁽¹⁾</th><th>Average P90⁽¹⁾</th><th>Average P10⁽¹⁾</th><th>Average P50⁽¹⁾</th><th>Average P90⁽¹⁾</th></tr<>	Average P50 ⁽¹⁾	Standard	Average P90 ⁽¹⁾	Average P10 ⁽¹⁾	Average P50 ⁽¹⁾	Average P90 ⁽¹⁾	Average P10 ⁽¹⁾	Average P50 ⁽¹⁾	Average P90 ⁽¹⁾	Average P10 ⁽¹⁾	Average P50 ⁽¹⁾	Average P90 ⁽¹⁾	Average P10 ⁽¹⁾	Average P50 ⁽¹⁾	Average P90 ⁽¹⁾
Al (Aluminum) μg/L 125 32.96 Alkalinity mg/L 21.32 As (Arsenic) μg/L 53 0.15 B (Boron) μg/L 500 12.92 Ba (Barium) μg/L 2.27 Be (Beryllium) μg/L 0.10 Ca (Calcium) mg/L 5.72 Cd (Cadmium)(3) μg/L 2.6 0.03 Cl (Chloride) mg/L 230 1.12 Co (Cobalt) μg/L 5 0.05 Cr (Chromium) μg/L 5 0.05 Cr (Chromium) μg/L 11 0.21 Cu (Copper)(3) μg/L 9.8 0.40 F (Fluoride) mg/L 0.02 Fe (Iron) μg/L 464.80 K (Potassium) mg/L 3.71 Mn (Manganese) μg/L 10.01 Na (Sodium) mg/L 0.87 <th></th> <th>Units</th> <th></th>		Units													
Alkalinity mg/L 21.32 As (Arsenic) μg/L 53 0.15 B (Boron) μg/L 500 12.92 Ba (Barium) μg/L 2.27 Be (Beryllium) μg/L 0.10 Ca (Calcium) mg/L 5.72 Cd (Cadmium)(3) μg/L 2.6 0.03 Cl (Chloride) mg/L 230 1.12 Co (Cobalt) μg/L 5 0.05 Cr (Chromium) μg/L 11 0.21 Cu (Copper)(3) μg/L 9.8 0.40 F (Fluoride) mg/L 0.02 Fe (Iron) μg/L 464.80 K (Potassium) mg/L 0.46 Mg (Magnesium) mg/L 10.01 Na (Sodium) mg/L 0.87 Ni (Nickel)(3) μg/L 54.6 0.20 Pb (Lead)(3) μg/L 3.4 0.02 <	0.10		0.11	0.10	0.11	0.12	0.10	0.11	0.15	0.10	0.11	0.15	0.10	0.11	0.15
As (Arsenic) μg/L 53 0.15 B (Boron) μg/L 500 12.92 Ba (Barium) μg/L 2.27 Be (Beryllium) μg/L 0.10 Ca (Calcium) mg/L 5.72 Cd (Cadmium)(3) μg/L 2.6 0.03 Cl (Chloride) mg/L 230 1.12 Co (Cobalt) μg/L 5 0.05 Cr (Chromium) μg/L 11 0.21 Cu (Copper)(3) μg/L 9.8 0.40 F (Fluoride) mg/L 0.02 Fe (Iron) μg/L 464.80 K (Potassium) mg/L 0.46 Mg (Magnesium) mg/L 0.46 Mg (Magnesium) mg/L 0.87 Ni (Nickel)(3) μg/L 54.6 0.20 Pb (Lead)(3) μg/L 3.4 0.02 Sb (Antimony) μg/L 3 0.19 SO4 (Sulfate) mg/L 2.92 T	79.13		267.93	29.20	75.87	254.52	33.05	82.98	250.45	31.88	83.07	256.89	32.03	82.77	263.34
B (Boron) μg/L 500 12.92 Ba (Barium) μg/L 2.27 Be (Beryllium) μg/L 0.10 Ca (Calcium) mg/L 5.72 Cd (Cadmium)(3) μg/L 2.6 0.03 Cl (Chloride) mg/L 230 1.12 Co (Cobalt) μg/L 5 0.05 Cr (Chromium) μg/L 11 0.21 Cu (Copper)(3) μg/L 9.8 0.40 F (Fluoride) mg/L 0.02 Fe (Iron) μg/L 464.80 K (Potassium) mg/L 0.46 Mg (Magnesium) mg/L 0.46 Mg (Magnesium) mg/L 0.87 Ni (Nickel)(3) μg/L 0.87 Ni (Nickel)(3) μg/L 54.6 0.20 Pb (Lead)(3) μg/L 3.4 0.02 Sb (Antimony) μg/L 5 0.19	65.79	<u> </u>	137.96	21.49	64.01	132.84	18.68	52.34	145.05	17.39	51.32	128.21	18.52	50.96	127.26
Ba (Barium) μg/L 2.27 Be (Beryllium) μg/L 0.10 Ca (Calcium) mg/L 5.72 Cd (Cadmium)(3) μg/L 2.6 0.03 Cl (Chloride) mg/L 230 1.12 Co (Cobalt) μg/L 5 0.05 Cr (Chromium) μg/L 11 0.21 Cu (Copper)(3) μg/L 9.8 0.40 F (Fluoride) mg/L 0.02 Fe (Iron) μg/L 464.80 K (Potassium) mg/L 0.46 Mg (Magnesium) mg/L 10.01 Na (Sodium) mg/L 0.87 Ni (Nickel)(3) μg/L 54.6 0.20 Pb (Lead)(3) μg/L 3.4 0.02 Sb (Antimony) μg/L 3.4 0.02 Se (Selenium) μg/L 5 0.19 SO4 (Sulfate) mg/L 2.92	0.76	μg/L 53	2.61	0.16	0.90	2.23	0.16	0.92	2.43	0.15	0.93	2.47	0.14	0.92	2.45
Be (Beryllium) μg/L 0.10 Ca (Calcium) mg/L 5.72 Cd (Cadmium)(3) μg/L 2.6 0.03 CI (Chloride) mg/L 230 1.12 Co (Cobalt) μg/L 5 0.05 Cr (Chromium) μg/L 11 0.21 Cu (Copper)(3) μg/L 9.8 0.40 F (Fluoride) mg/L 0.02 Fe (Iron) μg/L 464.80 K (Potassium) mg/L 0.46 Mg (Magnesium) mg/L 0.46 Mg (Magnesium) mg/L 10.01 Na (Sodium) mg/L 0.87 Ni (Nickel)(3) μg/L 54.6 0.20 Pb (Lead)(3) μg/L 3.4 0.02 Sb (Antimony) μg/L 3 0.19 SO4 (Sulfate) mg/L 2.92 TI (Thallium) μg/L 0.56 0.00 </td <td>64.04</td> <td>μg/L 500</td> <td>168.74</td> <td>13.75</td> <td>64.83</td> <td>199.71</td> <td>10.19</td> <td>43.53</td> <td>159.36</td> <td>10.39</td> <td>43.24</td> <td>163.91</td> <td>10.90</td> <td>40.59</td> <td>195.10</td>	64.04	μg/L 500	168.74	13.75	64.83	199.71	10.19	43.53	159.36	10.39	43.24	163.91	10.90	40.59	195.10
Ca (Calcium) mg/L 5.72 Cd (Cadmium)(3) μg/L 2.6 0.03 Cl (Chloride) mg/L 230 1.12 Co (Cobalt) μg/L 5 0.05 Cr (Chromium) μg/L 11 0.21 Cu (Copper)(3) μg/L 9.8 0.40 F (Fluoride) mg/L 0.02 Fe (Iron) μg/L 464.80 K (Potassium) mg/L 0.46 Mg (Magnesium) mg/L 3.71 Mn (Manganese) μg/L 0.87 Ni (Nickel)(3) μg/L 54.6 0.20 Pb (Lead)(3) μg/L 3.4 0.02 Sb (Antimony) μg/L 31 0.25 Se (Selenium) μg/L 5 0.19 SO4 (Sulfate) mg/L 2.92 Tl (Thallium) μg/L 0.56 0.00	9.31	μg/L	21.80	2.33	10.37	23.01	2.23	11.39	32.48	2.25	11.14	33.31	2.10	10.54	34.76
Cd (Cadmium)(3) μg/L 2.6 0.03 Cl (Chloride) mg/L 230 1.12 Co (Cobalt) μg/L 5 0.05 Cr (Chromium) μg/L 11 0.21 Cu (Copper)(3) μg/L 9.8 0.40 F (Fluoride) mg/L 0.02 Fe (Iron) μg/L 464.80 K (Potassium) mg/L 0.46 Mg (Magnesium) mg/L 3.71 Mn (Manganese) μg/L 10.01 Na (Sodium) mg/L 0.87 Ni (Nickel)(3) μg/L 54.6 0.20 Pb (Lead)(3) μg/L 3.4 0.02 Sb (Antimony) μg/L 3 0.25 Se (Selenium) μg/L 5 0.19 SO4 (Sulfate) mg/L 2.92 TI (Thallium) μg/L 0.56 0.00	0.10	μg/L	0.11	0.10	0.11	0.15	0.10	0.12	0.24	0.10	0.12	0.24	0.10	0.11	0.23
CI (Chloride) mg/L 230 1.12 Co (Cobalt) μg/L 5 0.05 Cr (Chromium) μg/L 11 0.21 Cu (Copper)(3) μg/L 9.8 0.40 F (Fluoride) mg/L 0.02 Fe (Iron) μg/L 464.80 K (Potassium) mg/L 0.46 Mg (Magnesium) mg/L 3.71 Mn (Manganese) μg/L 10.01 Na (Sodium) mg/L 0.87 Ni (Nickel)(3) μg/L 54.6 0.20 Pb (Lead)(3) μg/L 3.4 0.02 Sb (Antimony) μg/L 31 0.25 Se (Selenium) μg/L 5 0.19 SO4 (Sulfate) mg/L 2.92 TI (Thallium) μg/L 0.56 0.00	17.49	mg/L	34.03	5.65	17.88	34.65	5.09	15.02	33.78	5.09	14.76	33.55	5.34	14.70	31.81
Co (Cobalt) μg/L 5 0.05 Cr (Chromium) μg/L 11 0.21 Cu (Copper)(3) μg/L 9.8 0.40 F (Fluoride) mg/L 0.02 Fe (Iron) μg/L 464.80 K (Potassium) mg/L 0.46 Mg (Magnesium) mg/L 3.71 Mn (Manganese) μg/L 10.01 Na (Sodium) mg/L 0.87 Ni (Nickel)(3) μg/L 54.6 0.20 Pb (Lead)(3) μg/L 3.4 0.02 Sb (Antimony) μg/L 31 0.25 Se (Selenium) μg/L 5 0.19 SO4 (Sulfate) mg/L 2.92 TI (Thallium) μg/L 0.56 0.00	0.08	μg/L 2.6	0.14	0.03	0.13	0.39	0.03	0.17	0.89	0.03	0.15	0.91	0.03	0.13	0.77
Cr (Chromium) μg/L 11 0.21 Cu (Copper)(3) μg/L 9.8 0.40 F (Fluoride) mg/L 0.02 Fe (Iron) μg/L 464.80 K (Potassium) mg/L 0.46 Mg (Magnesium) mg/L 3.71 Mn (Manganese) μg/L 10.01 Na (Sodium) mg/L 0.87 Ni (Nickel)(3) μg/L 54.6 0.20 Pb (Lead)(3) μg/L 3.4 0.02 Sb (Antimony) μg/L 31 0.25 Se (Selenium) μg/L 5 0.19 SO4 (Sulfate) mg/L 2.92 TI (Thallium) μg/L 0.56 0.00	6.05	mg/L 230	13.62	1.21	6.23	16.37	0.75	3.23	13.37	0.66	2.95	15.30	0.62	2.58	13.64
Cu (Copper) ⁽³⁾ μg/L 9.8 0.40 F (Fluoride) mg/L 0.02 Fe (Iron) μg/L 464.80 K (Potassium) mg/L 0.46 Mg (Magnesium) mg/L 3.71 Mn (Manganese) μg/L 10.01 Na (Sodium) mg/L 0.87 Ni (Nickel) ⁽³⁾ μg/L 54.6 0.20 Pb (Lead) ⁽³⁾ μg/L 3.4 0.02 Sb (Antimony) μg/L 31 0.25 Se (Selenium) μg/L 5 0.19 SO4 (Sulfate) mg/L 2.92 TI (Thallium) μg/L 0.56 0.00	0.33	μg/L 5	0.91	0.07	0.54	1.22	0.06	0.78	2.91	0.07	0.83	3.03	0.08	0.81	2.93
F (Fluoride) mg/L 0.02 Fe (Iron) μg/L 464.80 K (Potassium) mg/L 0.46 Mg (Magnesium) mg/L 3.71 Mn (Manganese) μg/L 10.01 Na (Sodium) mg/L 0.87 Ni (Nickel)(3) μg/L 54.6 0.20 Pb (Lead)(3) μg/L 3.4 0.02 Sb (Antimony) μg/L 31 0.25 Se (Selenium) μg/L 5 0.19 SO4 (Sulfate) mg/L 2.92 TI (Thallium) μg/L 0.56 0.00	0.59	μg/L 11	1.20	0.24	0.70	1.26	0.21	0.81	1.63	0.22	0.76	1.39	0.24	0.70	1.34
Fe (Iron) μg/L 464.80 K (Potassium) mg/L 0.46 Mg (Magnesium) mg/L 3.71 Mn (Manganese) μg/L 10.01 Na (Sodium) mg/L 0.87 Ni (Nickel)(3) μg/L 54.6 0.20 Pb (Lead)(3) μg/L 3.4 0.02 Sb (Antimony) μg/L 31 0.25 Se (Selenium) μg/L 5 0.19 SO4 (Sulfate) mg/L 2.92 TI (Thallium) μg/L 0.56 0.00	1.25	μg/L 9.8	2.91	0.43	1.65	3.08	0.44	2.15	5.57	0.46	2.15	5.57	0.44	2.12	5.59
K (Potassium) mg/L 0.46 Mg (Magnesium) mg/L 3.71 Mn (Manganese) μg/L 10.01 Na (Sodium) mg/L 0.87 Ni (Nickel)(3) μg/L 54.6 0.20 Pb (Lead)(3) μg/L 3.4 0.02 Sb (Antimony) μg/L 31 0.25 Se (Selenium) μg/L 5 0.19 SO4 (Sulfate) mg/L 2.92 TI (Thallium) μg/L 0.56 0.00	0.08	mg/L	0.18	0.03	0.09	0.20	0.02	0.08	0.20	0.02	0.08	0.19	0.02	0.07	0.19
Mg (Magnesium) mg/L 3.71 Mn (Manganese) μg/L 10.01 Na (Sodium) mg/L 0.87 Ni (Nickel) ⁽³⁾ μg/L 54.6 0.20 Pb (Lead) ⁽³⁾ μg/L 3.4 0.02 Sb (Antimony) μg/L 31 0.25 Se (Selenium) μg/L 5 0.19 SO4 (Sulfate) mg/L 2.92 TI (Thallium) μg/L 0.56 0.00	1,682.78	μg/L	5,123.50	477.17	1,578.39	4,539.00	415.39	1,259.69	5,259.70	373.14	1,257.83	4,524.60	434.73	1,236.18	4,679.40
Mn (Manganese) μg/L 10.01 Na (Sodium) mg/L 0.87 Ni (Nickel) ⁽³⁾ μg/L 54.6 0.20 Pb (Lead) ⁽³⁾ μg/L 3.4 0.02 Sb (Antimony) μg/L 31 0.25 Se (Selenium) μg/L 5 0.19 SO4 (Sulfate) mg/L 2.92 TI (Thallium) μg/L 0.56 0.00	1.79	mg/L	3.77	0.48	1.68	3.67	0.36	1.20	3.80	0.39	1.24	4.98	0.40	1.23	3.96
Na (Sodium) mg/L 0.87 Ni (Nickel)(3) μg/L 54.6 0.20 Pb (Lead)(3) μg/L 3.4 0.02 Sb (Antimony) μg/L 31 0.25 Se (Selenium) μg/L 5 0.19 SO4 (Sulfate) mg/L 2.92 TI (Thallium) μg/L 0.56 0.00	8.96	mg/L	13.72	3.80	8.82	13.83	3.27	6.99	14.33	3.29	7.03	12.84	3.29	6.93	13.91
Ni (Nickel) ⁽³⁾ μg/L 54.6 0.20 Pb (Lead) ⁽³⁾ μg/L 3.4 0.02 Sb (Antimony) μg/L 31 0.25 Se (Selenium) μg/L 5 0.19 SO4 (Sulfate) mg/L 2.92 TI (Thallium) μg/L 0.56 0.00	132.57	μg/L	357.08	11.43	126.31	334.87	8.68	125.66	417.39	8.03	126.21	424.46	8.02	125.81	421.94
Pb (Lead)(3) μg/L 3.4 0.02 Sb (Antimony) μg/L 31 0.25 Se (Selenium) μg/L 5 0.19 SO4 (Sulfate) mg/L 2.92 TI (Thallium) μg/L 0.56 0.00	5.34	mg/L	12.21	1.00	6.67	16.08	0.79	5.99	20.72	0.75	5.39	17.77	0.72	4.57	12.41
Sb (Antimony) μg/L 31 0.25 Se (Selenium) μg/L 5 0.19 SO4 (Sulfate) mg/L 2.92 TI (Thallium) μg/L 0.56 0.00	0.98	μg/L 54.6	2.80	0.39	3.07	9.03	0.40	5.11	24.61	0.46	5.38	25.14	0.47	5.11	24.90
Se (Selenium) μg/L 5 0.19 SO4 (Sulfate) mg/L 2.92 TI (Thallium) μg/L 0.56 0.00	0.23	μg/L 3.4	0.63	0.03	0.35	0.79	0.03	0.48	1.79	0.04	0.49	1.79	0.03	0.48	1.78
Se (Selenium) μg/L 5 0.19 SO4 (Sulfate) mg/L 2.92 TI (Thallium) μg/L 0.56 0.00	0.25	μg/L 31	0.25	0.25	0.53	1.63	0.27	0.76	3.60	0.26	0.64	2.87	0.26	0.49	1.87
SO4 (Sulfate) mg/L 2.92 TI (Thallium) μg/L 0.56 0.00	0.58		1.25	0.20	0.63	1.25	0.18	0.58	1.46	0.18	0.57	1.32	0.18	0.53	1.26
TI (Thallium) μg/L 0.56 0.00	11.96		24.17	3.12	11.57	22.04	2.00	7.22	15.94	2.35	7.22	16.53	2.21	7.18	14.54
, , ,	0.03	-	0.08	0.00	0.03	0.08	0.00	0.03	0.11	0.00	0.03	0.11	0.00	0.03	0.10
, , , , , , , , , , , , , , , , , , , ,	1.46		2.46	1.02	1.81	3.31	1.02	2.24	6.47	1.02	2.29	6.48	1.02	2.21	6.51
Zn (Zinc) ⁽³⁾ μg/L 125.4 1.10	4.64		21.80	1.52	8.64	20.83	1.54	11.74	46.39	1.48	10.92	48.09	1.39	9.49	45.35
Hardness mg/L 500 39.07	82.56		124.06	37.88	83.30	128.16	35.38	69.56	122.73	34.54	68.75	122.42	37.02	68.11	119.28

NOTE: Values above the applicable water quality standard are shown in bold with light red shading.
(1) Values shown are the average of the monthly P10, P50, and P90 values, as indicated, for the referenced Mine Year; see Section 6.5 of Reference (2).
(2) Model runs evaluated through Mine Year 200.
(3) Standard is hardness-based and evaluated at a hardness of 105.5 mg/L; see Section 6.5.3 of Reference (2).

Estimated Water Quality from Stockpile Drainage Large Table 6

	Stockpile	Category	y 2/3 Stockpil	e (Temporary)	Category 4 Stockpile (Temporary)		Ore Su	ırge Pile (Ten	nporary)	Category 1 Stockpile (Permanent)					
Constituent ⁽¹⁾	Mine Year Units	Mine Year 1	Mine Year 11	Mine Year 19 ⁽²⁾	Mine Year	Mine Year 11 ⁽²⁾	Mine Year 1	Mine Year 11	Mine Year 20 ⁽²⁾	Mine Year 1	Mine Year 11	Mine Year 20	Mine Year 30 ⁽³⁾	Mine Year 75 ⁽³⁾	
Ag (Silver)	μg/L	0.18	16.30	41.87	21.43	32.89	0.16	39.13	40.54	0.000	0.003	0.01	0.19	0.20	
Al (Aluminum)	μg/L	0.44	150,188.33	378,285.83	253,425.83	404,926.67	0.40	351,548.33	345,180.00	1.14	1.41	1.41	1.41	1.41	
Alkalinity	mg/L	25.03	16.65	0.00	0.00	0.00	21.19	1.17	0.14	32.13	39.69	39.68	39.69	39.69	
As (Arsenic)	μg/L	89.42	100.00	100.08	122.93	159.34	79.47	99.83	97.32	17.53	100.00	99.98	100.00	100.00	
B (Boron)	μg/L	89.46	138.35	200.40	617.68	1,247.21	81.00	193.41	194.70	80.77	100.00	99.98	100.00	100.00	
Ba (Barium)	μg/L	28.18	10.27	11.32	73.24	91.62	25.52	7.78	11.72	31.96	14.96	14.50	10.79	10.67	
Be (Beryllium)	μg/L	0.36	6.66	16.61	11.30	14.94	0.32	15.69	13.84	0.12	0.40	0.40	0.40	0.40	
Ca (Calcium)	mg/L	215.42	577.78	405.76	294.51	434.49	80.20	422.61	387.71	144.00	682.43	683.98	683.89	684.28	
Cd (Cadmium)	μg/L	4.23	45.66	64.31	90.65	125.67	2.33	82.88	24.41	1.06	3.48	3.59	3.67	3.67	
CI (Chloride)	mg/L	44.57	10.18	0.00	21.82	3.98	12.83	0.00	0.00	37.48	9.52	0.00	0.00	0.00	
Co (Cobalt)	μg/L	261.85	5457.81	2881.22	6,449.77	15,151.08	662.06	28,145.75	8,023.15	83.27	122.65	122.62	122.65	122.65	
Cr (Chromium)	μg/L	6.19	12.28	15.96	8.35	17.16	4.56	15.57	15.29	5.92	10.00	10.00	10.00	10.00	
Cu (Copper)	μg/L	4,751.60	61,553.42	149,495.83	1,437.46	11,040.08	4,300.46	140,653.33	145,123.33	191.18	236.74	236.69	236.73	236.74	
F (Fluoride)	mg/L	1.81	1.53	1.78	1.83	1.90	1.07	1.75	1.75	1.84	1.37	1.37	1.37	1.37	
Fe (Iron)	μg/L	86.86	32,754.25	86,679.25	194,950.00	317,081.67	78.62	80,801.83	84,178.58	48.52	59.87	59.86	59.87	59.87	
K (Potassium)	mg/L	31.92	42.03	27.98	9.41	13.29	13.84	29.22	27.03	23.74	46.90	46.89	46.90	46.90	
Mg (Magnesium)	mg/L	35.77	389.22	303.89	276.57	1,059.48	25.80	732.48	226.73	32.95	146.90	150.97	150.76	151.06	
Mn (Manganese)	μg/L	583.62	11212.28	7176.27	8,960.25	61,068.75	1,386.01	45,722.58	17,019.33	150.96	232.11	232.06	232.10	232.11	
Na (Sodium)	mg/L	40.55	154.53	62.91	11.32	20.20	34.07	77.90	59.26	35.56	219.15	225.86	234.82	235.37	
Ni (Nickel)	μg/L	3,669.22	83,192.58	42,749.81	9,183.46	31,350.58	12,158.82	588,671.67	146,079.33	1,062.53	2,226.35	2,227.61	2,230.10	2,230.10	
Pb (Lead)	μg/L	1.79	137.15	360.09	288.10	385.96	1.62	337.01	341.77	0.62	7.35	10.77	99.93	100.00	
Sb (Antimony)	μg/L	40.93	691.01	702.75	414.47	1,714.23	28.02	1,557.98	359.45	22.68	52.61	53.26	54.15	54.15	
Se (Selenium)	μg/L	12.85	81.09	99.86	39.08	109.64	15.17	99.25	96.78	0.19	1.68	2.02	4.28	4.46	
SO4 (Sulfate)	mg/L	394.16	4,251.70	2,978.15	4,225.02	11,551.67	369.83	7,846.88	2,728.82	119.90	1,265.19	1,393.40	2,727.05	2,793.10	
TI (Thallium)	μg/L	0.10	1.03	2.07	1.07	2.75	0.16	2.14	1.95	0.01	0.08	0.08	0.20	0.20	
V (Vanadium)	μg/L	8.94	27.53	55.00	9.78	15.42	8.09	52.29	53.17	8.08	10.00	10.00	10.00	10.00	
Zn (Zinc)	μg/L	305.46	3,640.50	6,322.72	7,779.83	10,721.28	346.31	7,991.74	3,635.80	55.38	181.17	187.68	192.16	192.16	

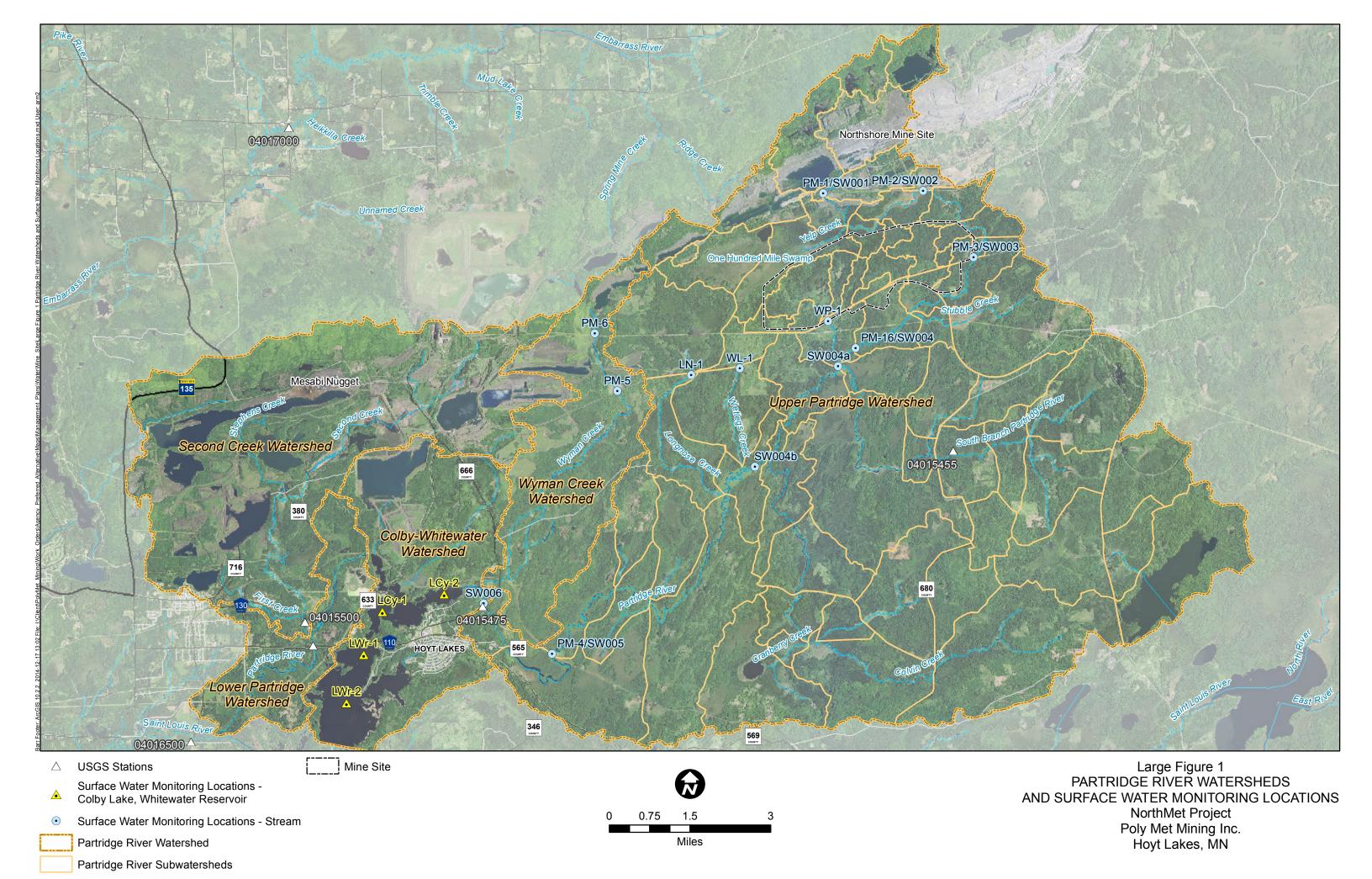
⁽¹⁾ Values shown are the average of the monthly P50 values for the referenced Mine Year; see Section 6.2 of Reference (2).
(2) Temporary stockpiles are shown through the years they are active: Category 2/3 Stockpile through Year 19, Category 4 Stockpile through Mine Year 11, and Ore Surge Pile th rough Mine Year 20.
(3) Concentrations in the Category 1 Waste Rock Stockpile seepage are typically stable from approximately Mine Year 30 through long-term closure.

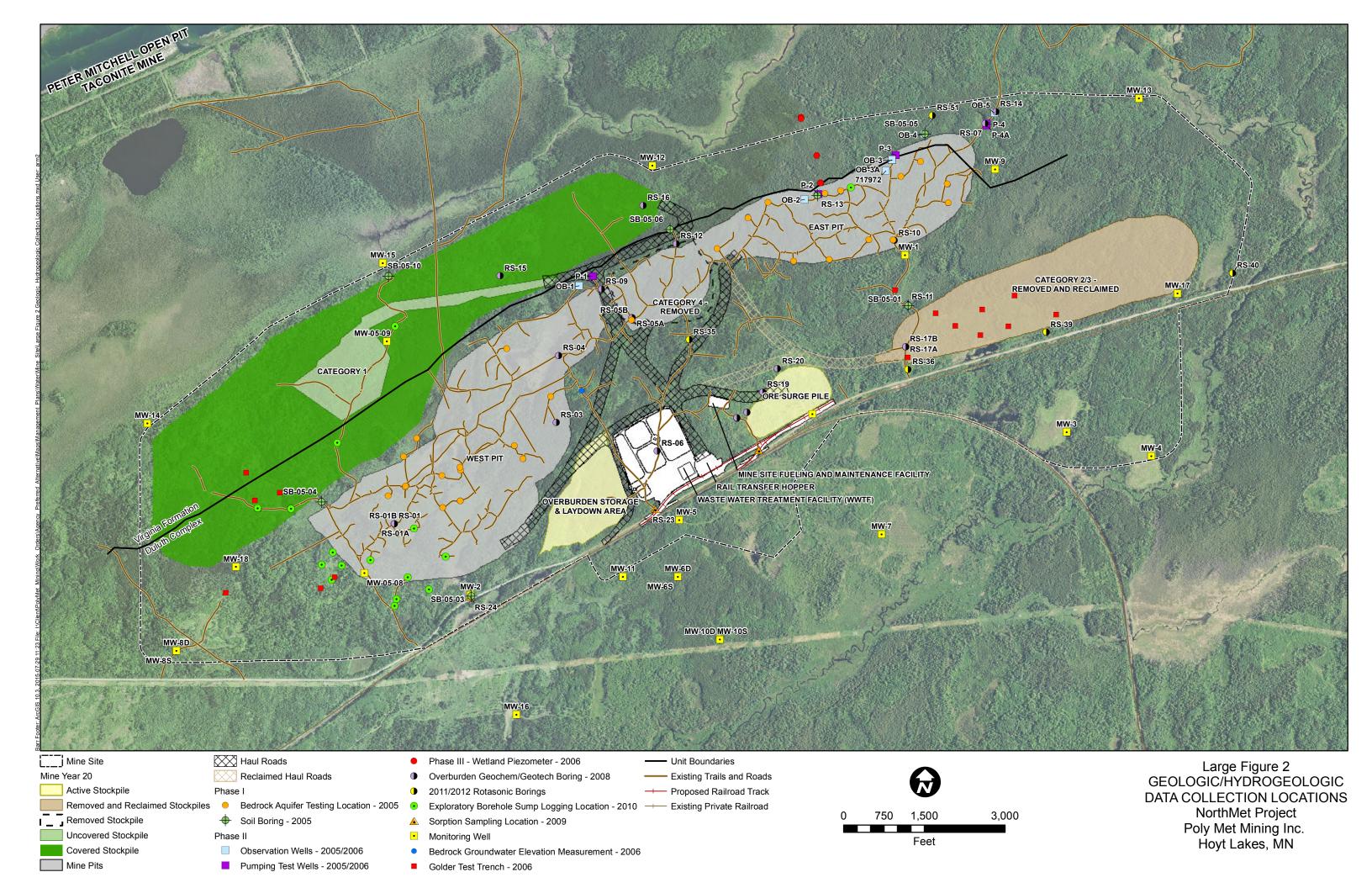
Large Table 7 Estimated Treated Water Pipeline Water Quality

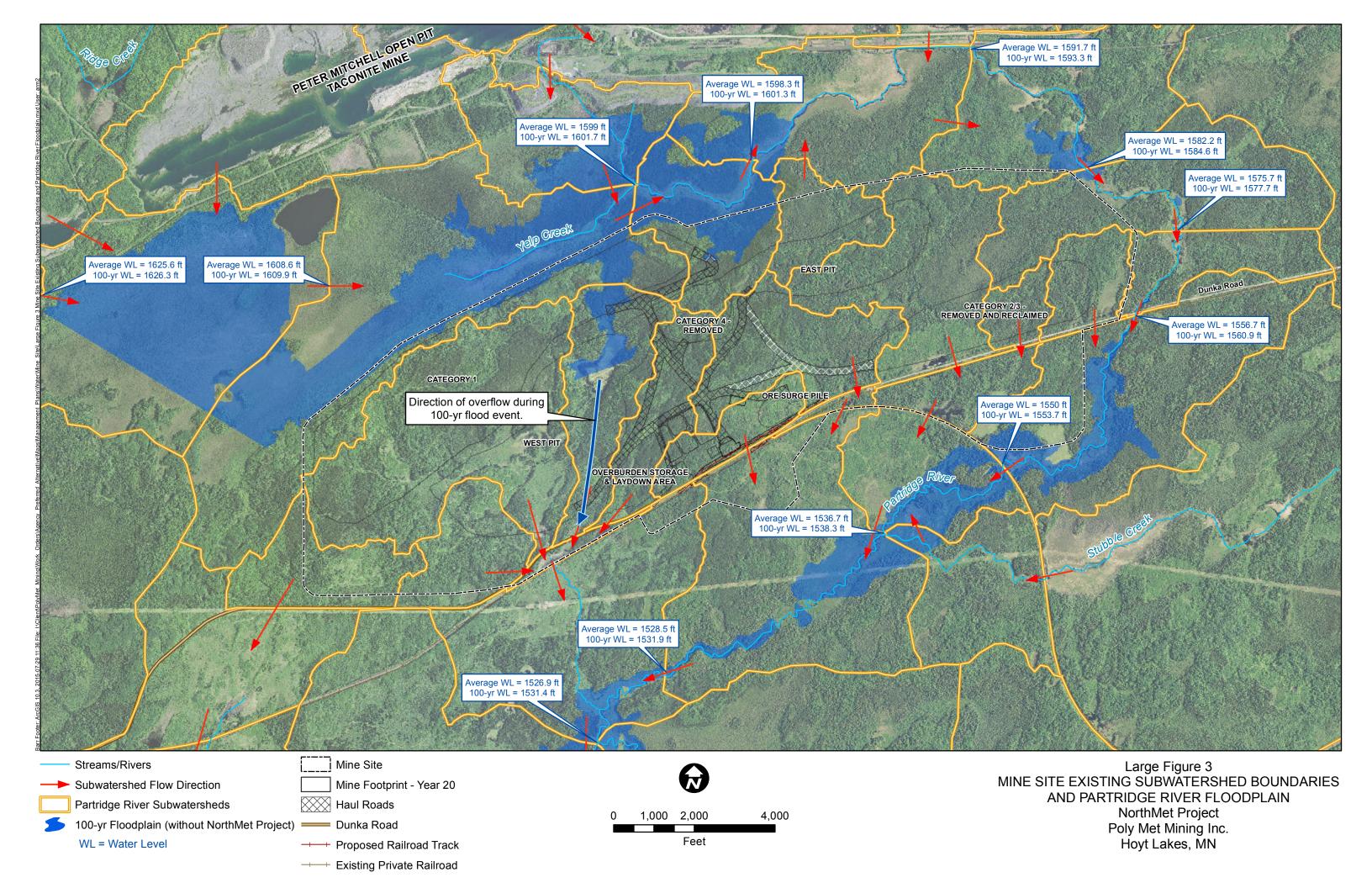
	Mine Year		Mine Year 1			Mine Year 1	1	ı	Mine Year 20	
Constituent	Percentile Units	Average P10 ⁽²⁾	Average P50 ⁽²⁾	Average P90 ⁽²⁾	Average P10 ⁽²⁾	Average P50 ⁽²⁾	Average P90 ⁽²⁾	Average P10 ⁽²⁾	Average P50 ⁽²⁾	Average P90 ⁽²⁾
Ag (Silver)	μg/L	0.34	0.76	0.84	0.98	1.00	1.00	0.97	0.98	1.00
Al (Aluminum)	μg/L	104.42	104.45	104.48	125.00	125.00	125.00	125.00	125.00	125.00
Alkalinity	mg/L	602.17	674.52	758.37	24.33	33.74	42.91	30.90	36.62	43.63
As (Arsenic)	μg/L	8.62	8.74	8.92	10.00	10.00	10.00	10.00	10.00	10.00
B (Boron)	μg/L	107.64	135.82	170.18	80.00	103.63	136.54	69.67	82.93	101.98
Ba (Barium)	μg/L	668.86	746.93	836.44	47.60	63.13	81.98	87.42	97.84	109.22
Be (Beryllium)	μg/L	0.42	0.69	3.07	0.90	1.34	3.08	0.42	0.84	1.13
Ca (Calcium)	mg/L	16.99	22.13	28.70	37.26	44.29	51.62	50.19	59.18	66.16
Cd (Cadmium)	μg/L	3.05	3.35	3.38	3.88	4.00	4.00	1.74	3.00	3.89
CI (Chloride)	mg/L	78.60	98.38	135.09	31.63	47.47	70.84	111.16	129.11	140.68
Co (Cobalt)	μg/L	4.58	4.58	4.58	5.00	5.00	5.00	5.00	5.00	5.00
Cr (Chromium)	μg/L	2.57	3.83	5.03	4.28	5.04	6.21	4.60	5.29	5.95
Cu (Copper)	μg/L	26.19	27.50	27.50	30.00	30.00	30.00	30.00	30.00	30.00
F (Fluoride)	mg/L	1.83	1.83	1.83	1.90	1.99	2.00	2.00	2.00	2.00
Fe (Iron)	μg/L	275.00	275.00	275.00	300.00	300.00	300.00	300.00	300.00	300.00
K (Potassium)	mg/L	42.30	50.13	58.64	42.14	56.04	72.34	183.79	210.93	240.25
Mg (Magnesium)	mg/L	38.39	42.40	45.53	29.50	33.97	38.25	20.63	24.89	30.37
Mn (Manganese)	μg/L	45.83	45.83	45.83	50.00	50.00	50.00	50.00	50.00	50.00
Na (Sodium)	mg/L	247.70	285.64	334.89	140.14	192.87	248.75	502.24	589.61	686.07
Ni (Nickel)	μg/L	91.67	91.67	91.67	100.00	100.00	100.00	100.00	100.00	100.00
Pb (Lead)	μg/L	6.79	11.72	14.30	19.00	19.00	19.00	19.00	19.00	19.00
Sb (Antimony)	μg/L	18.95	25.94	26.23	31.00	31.00	31.00	17.89	24.64	31.00
Se (Selenium)	μg/L	2.74	4.08	4.24	5.00	5.00	5.00	3.61	4.78	5.00
SO4 (Sulfate)	mg/L	229.17	229.17	229.17	250.00	250.00	250.00	250.00	250.00	250.00
TI (Thallium)	μg/L	0.10	0.13	0.18	0.23	0.32	0.56	0.12	0.17	0.23
V (Vanadium)	μg/L	6.31	8.00	13.76	9.95	11.06	14.37	9.79	10.68	11.56
Zn (Zinc)	μg/L	282.98	322.86	326.37	380.37	388.00	388.00	249.70	351.99	388.00

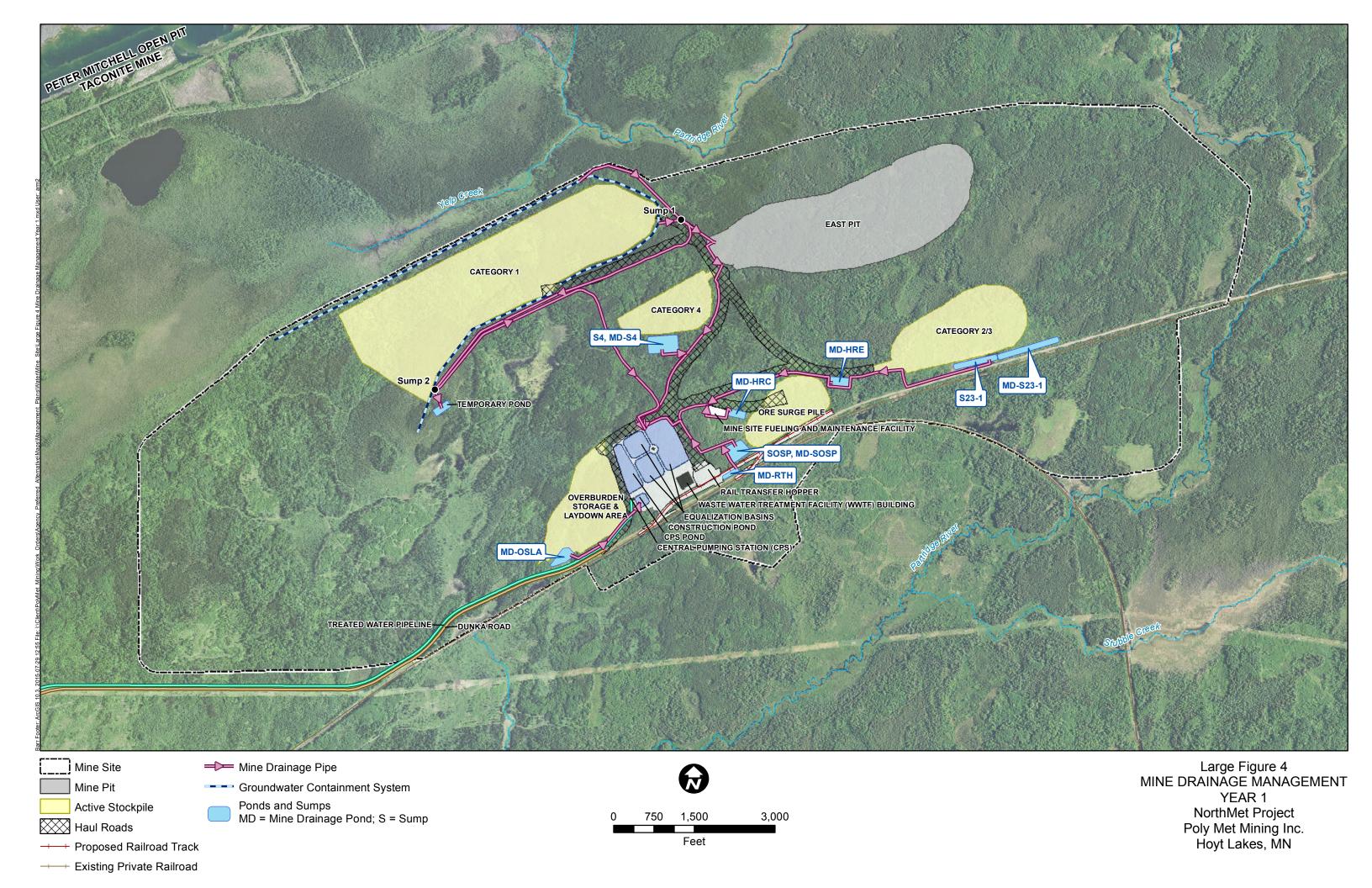
⁽¹⁾ Values shown are the average of the monthly P10, P50, and P90 values, as indicated, for the referenced Mine Year; see Section 6.2 of Reference (2).

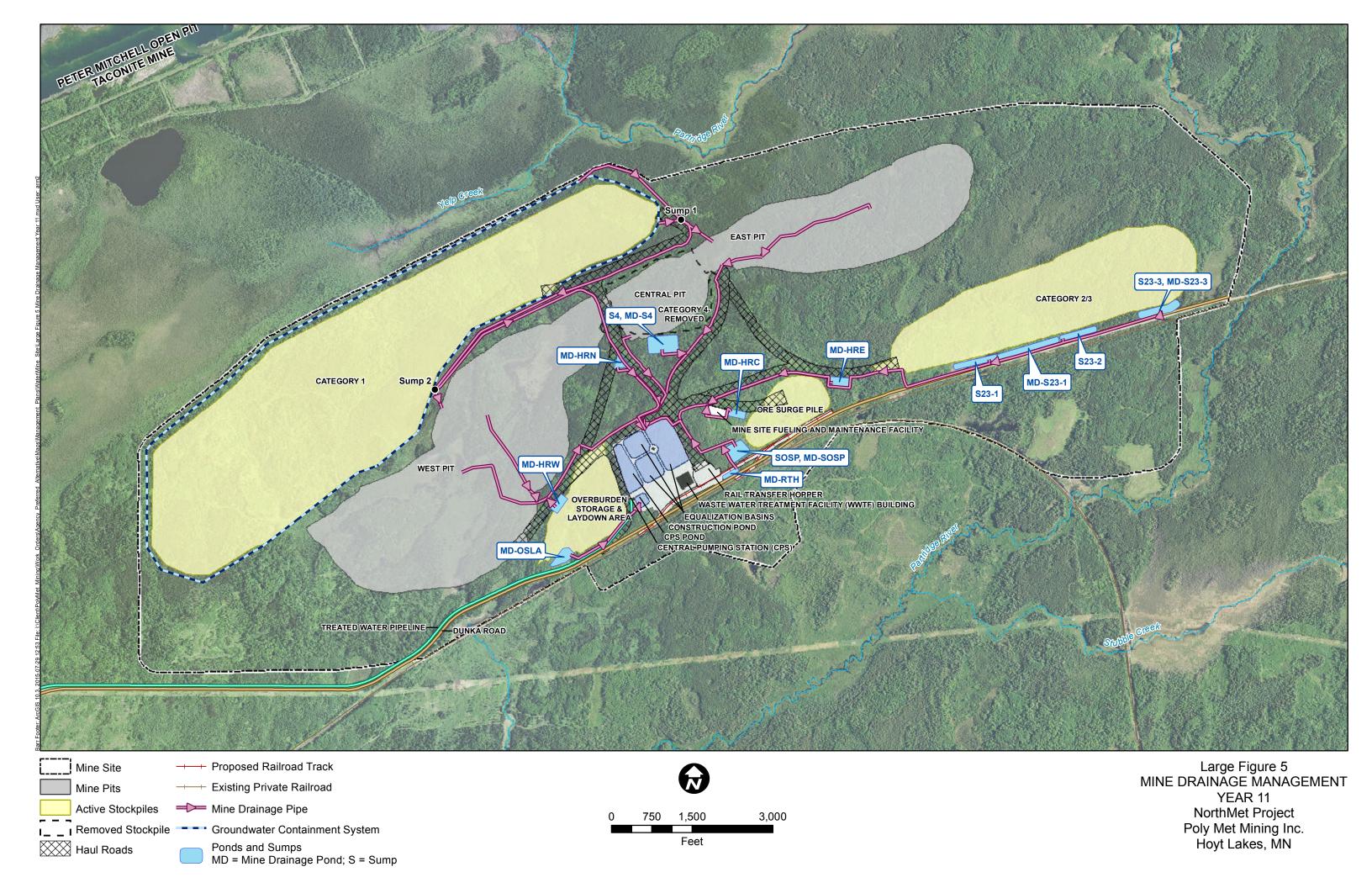
Large Figures

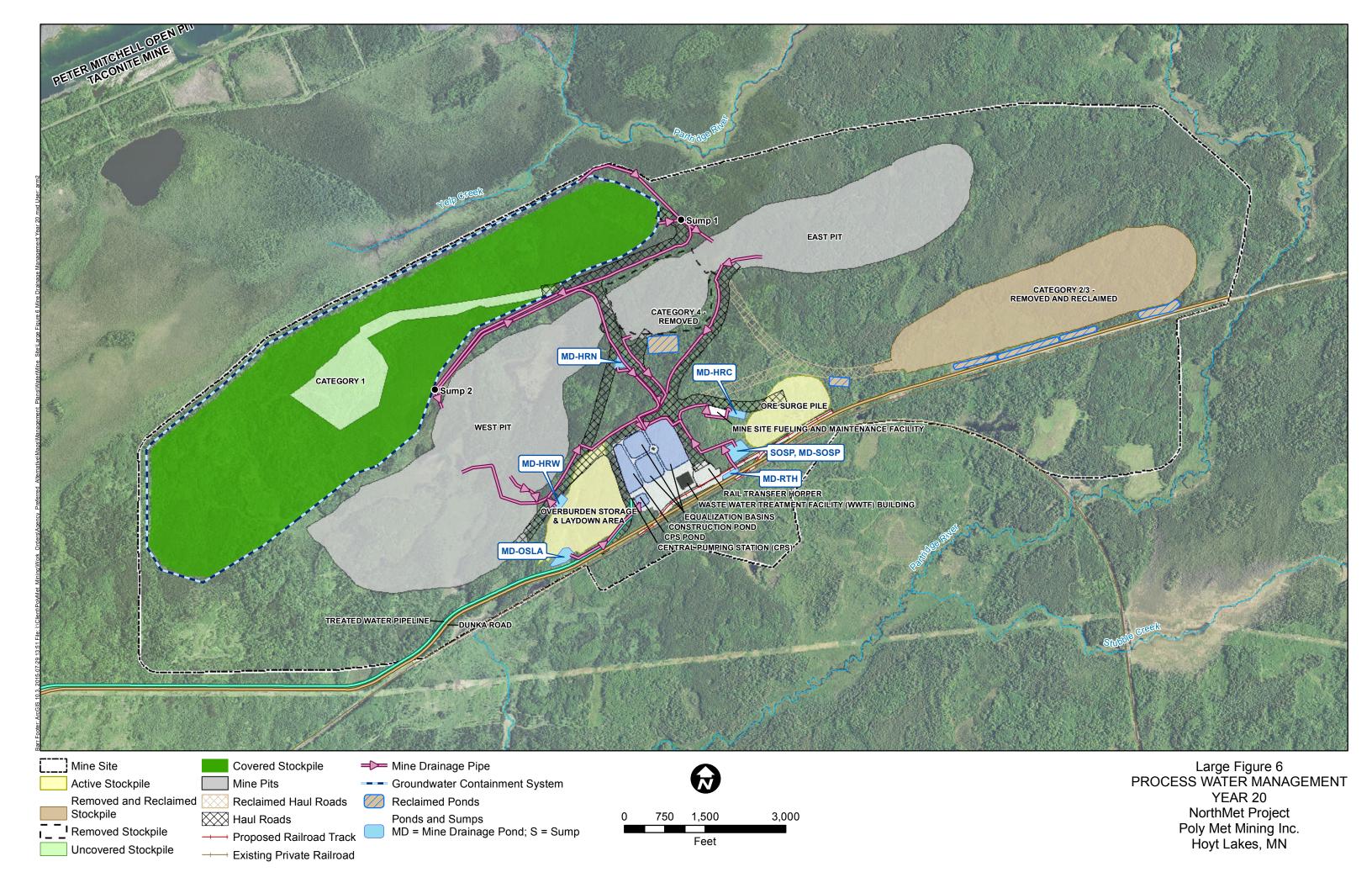


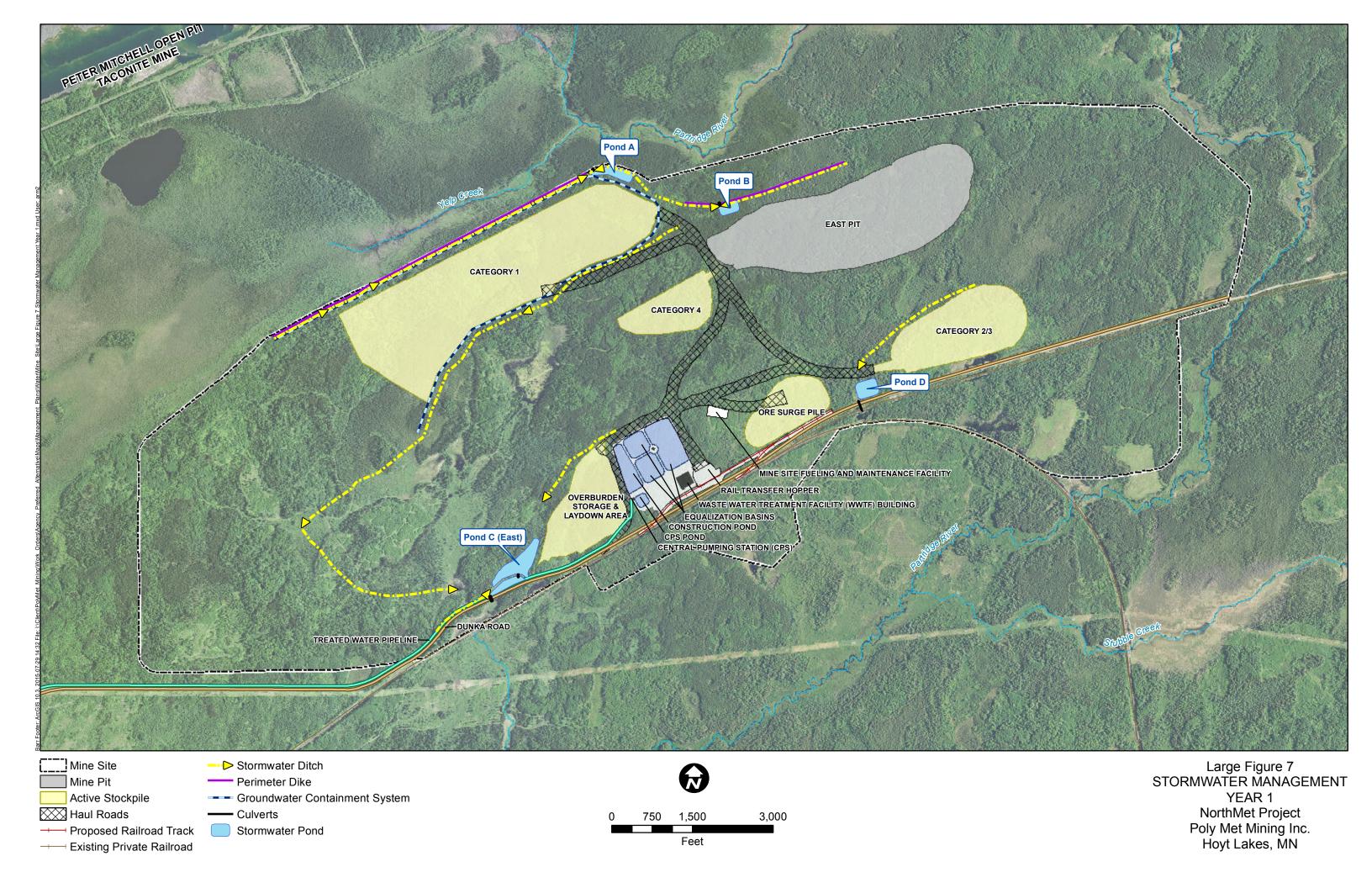


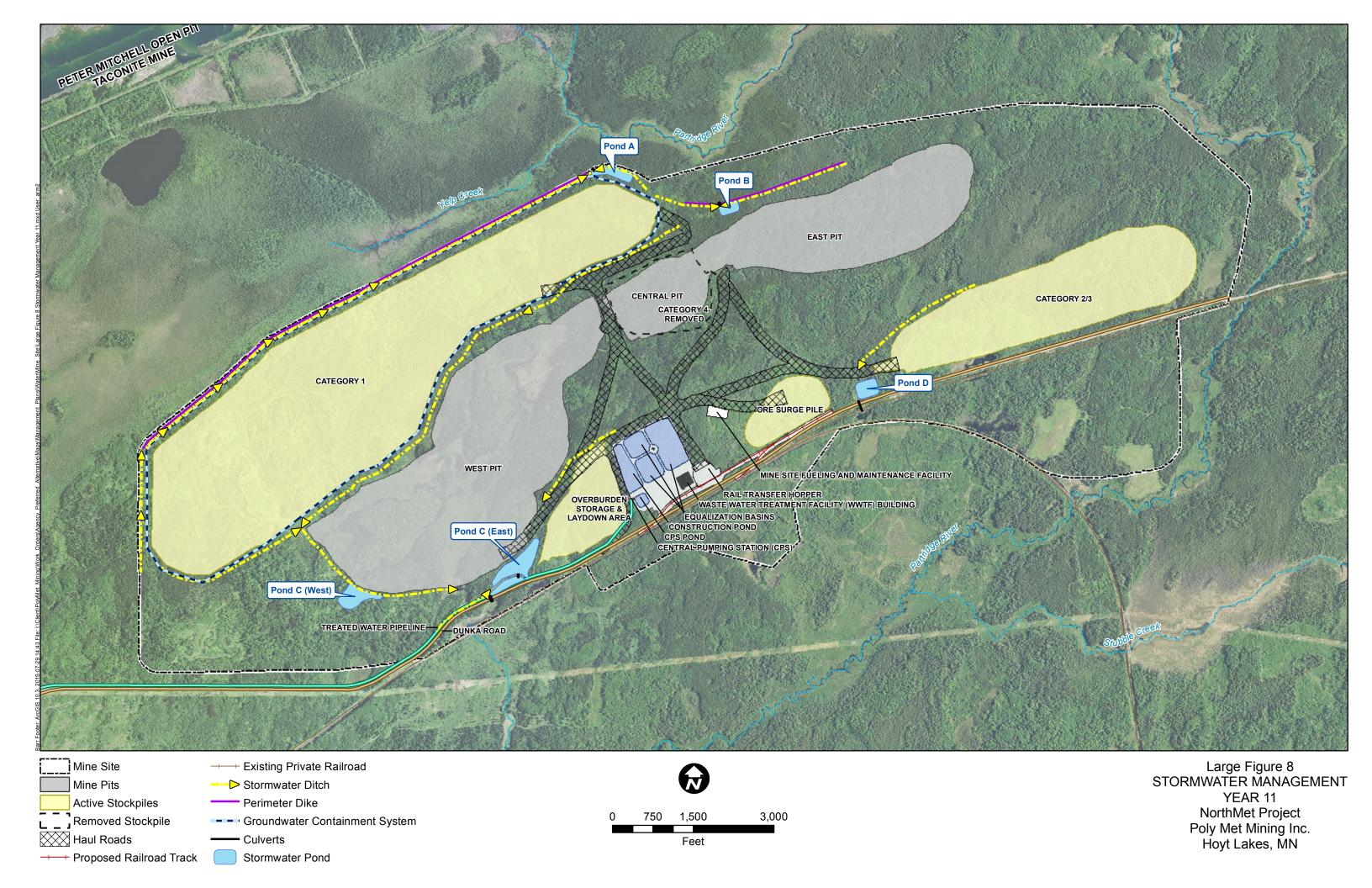


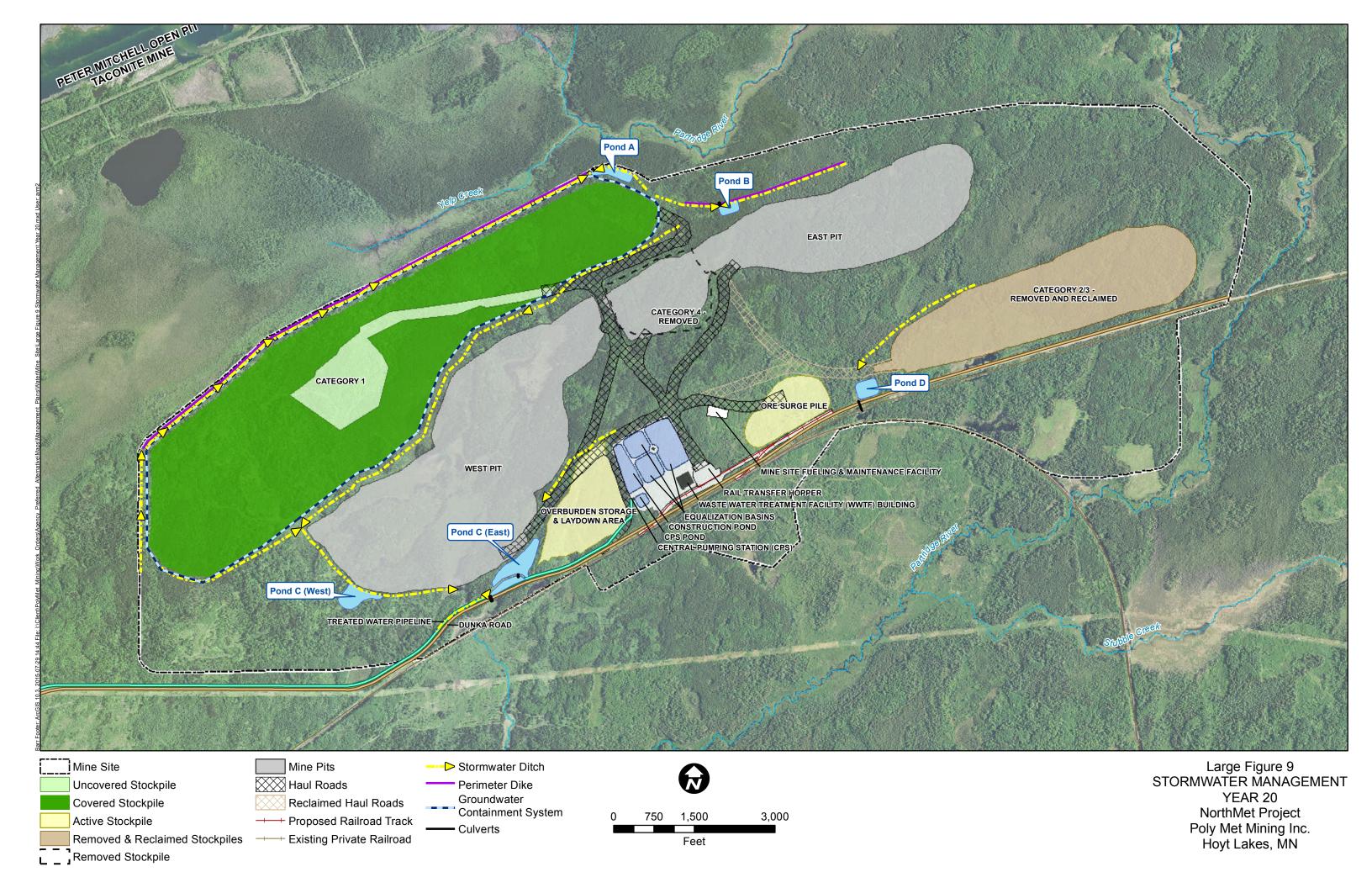


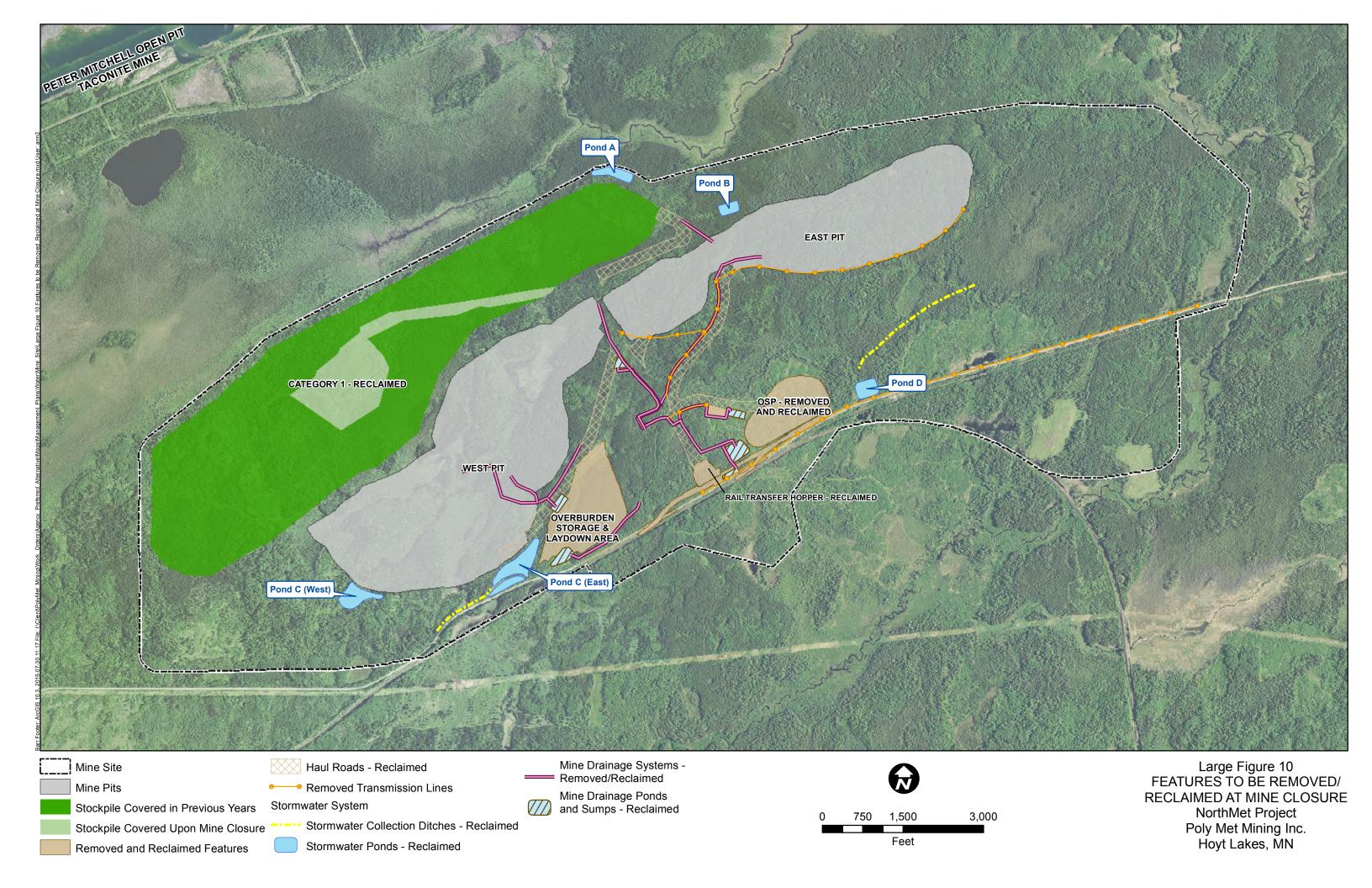


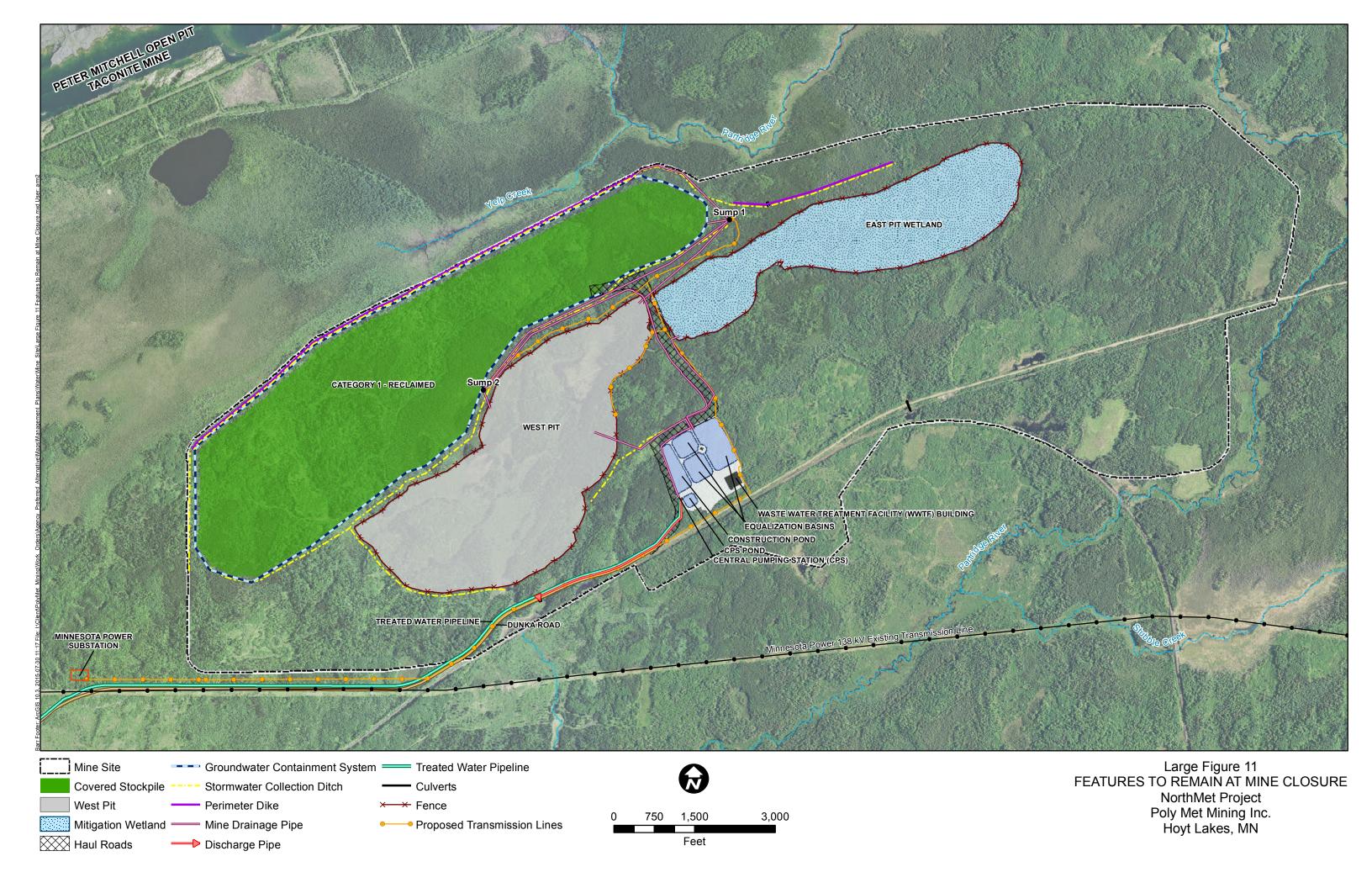












Attachments

Attachment A

Mechanical Design Drawings

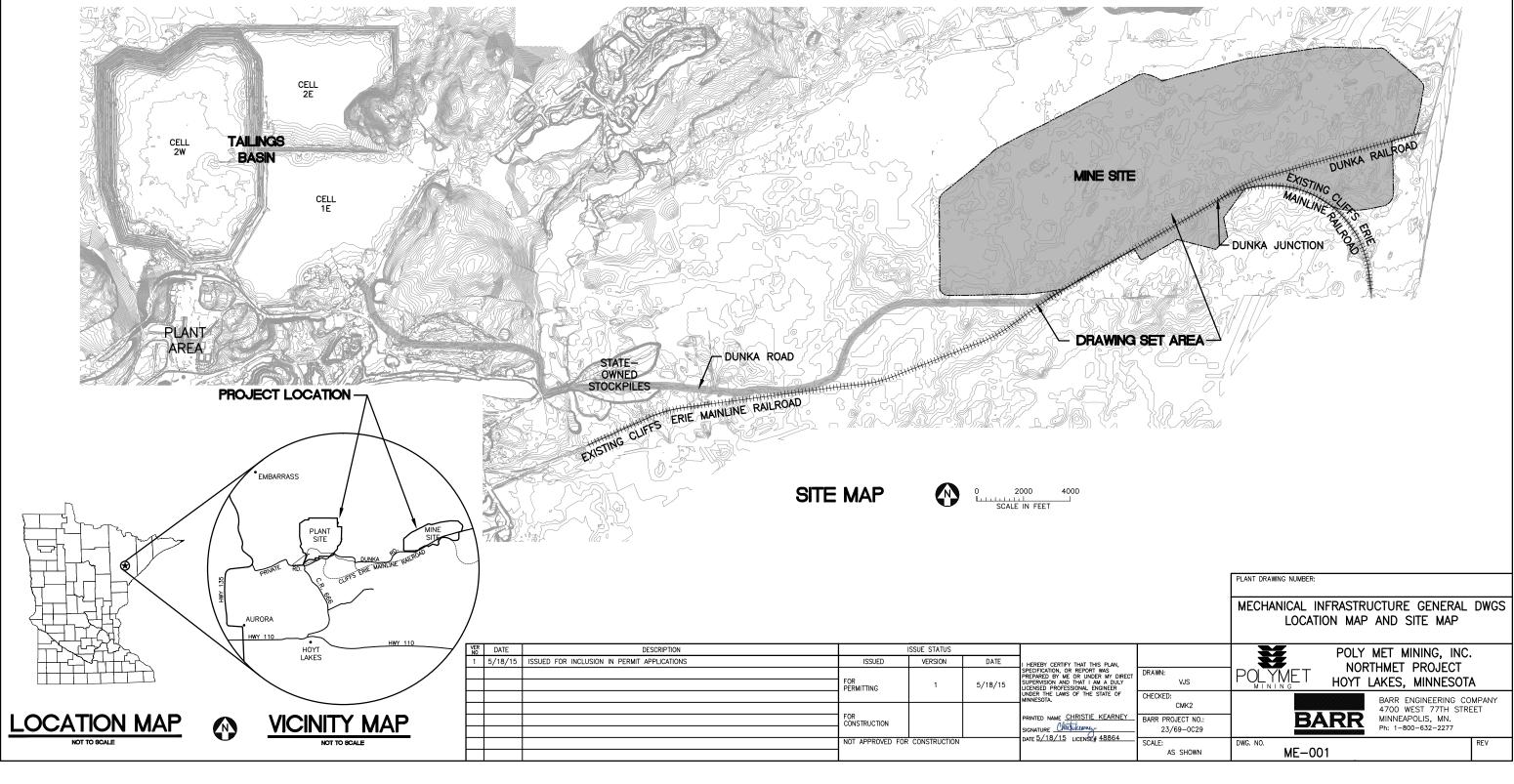
Errata Sheet

Poly Met Mining Inc. NorthMet Project

Permit Application Support Drawings: Mechanical Infrastructure May 2016

The table below lists changes that were identified during completion of the Construction Stormwater Pollution Prevention Plans (SWPPPs) and have not yet been incorporated in the attached permit application support drawings within this set. These changes and additional details developed during final design will be incorporated into the final design drawing set.

Drawing Sheet(s)	Change
Global change to all sheets, as needed	The terminology "mine drainage" as noted in these drawings has been changed to "mine water".
MD-003	The grading for the access road from the Fueling and Maintenance Facility to Pond MD-SOSP & Sump SOSP has been revised as a result of additional engineering.
MD-001	To meet requirements, Sump 2 overflow pond and associated piping was added to the SWPPP where the "construction water pond" callout is located on MD-001.
TWP-009	The berm over the TWP was revised to match the contours of the road where it crosses the proposed access roads near the CPS.
MD-001	An access road has been added adjacent to a Mine Water pipe for construction and maintenance purposes. This access road follows the Mine Water pipe that connects the Category 1 Stockpile and Haul Road F (in a general north-south orientation).
TWP-010	The option of rock as the top berm surface was eliminated to minimize additional impervious surfaces. Remove "or 1" minus rock" text on Sections 1, 2, 4 & 5.



MECHANICAL INFRASTRUCTURE LEGEND

CENTRAL PUMPING STATION

PROPOSED MINE DRAINAGE PIPE

PROPOSED MINE DRAINAGE PIPE

PROPOSED SUMP MANHOLE

PROPOSED MINE DRAINAGE CULVERT

PROPOSED TREATED WATER PIPELINE

PROPOSED HAUL ROAD MINE DRAINAGE DITCH

PROPOSED STOCKPILE LINER OUTLET PIPE

PROPOSED MINE DRAINAGE WATERSHED

MINE DRAINAGE

TOP OF DIKE BENCH

----<

GENERAL

PROPOSED TREATED WATER PIPELINE EXISTING CONTOUR - MAJOR EXISTING CONTOUR - MINOR PROPOSED CULVERT (NON-MINE DRAINAGE) ---1000----PROPOSED CONTOUR - MAJOR PROPOSED CONTOUR - MINOR ---1000----

OTHER FACILITY PROPOSED CONTOUR - MAJOR OTHER FACILITY PROPOSED CONTOUR - MINOR PROPOSED RAILROAD EXISTING RAILROAD

PROPOSED ACCESS ROADS EXISTING ROAD MINE SITE BOUNDARY (<u>*</u> WETLAND BOUNDARY

+/+/+HAUL ROAD

TREATED WATER PIPELINE

FXISTING POWER POLE EXISTING TRAIL EXISTING UNIMPROVED TRAIL ----R/W---RIGHT OF WAY EXISTING STRUCTURES TRFF LINE EXISTING OVERHEAD ELECTRIC EXISTING UNDERGROUND ELECTRIC PROPOSED TREATED WATER PIPELINE PROPOSED CULVERT (NON-MINE DRAINAGE) PROPOSED MINE DRAINAGE PIPE

NOTES

- 1. COORDINATE SYSTEM IS MINNESOTA STATE PLANE NORTH ZONE, NAD83.
- 2. ELEVATIONS ARE MEAN SEA LEVEL (MSL), NAVD88.
- 3. EXISTING TOPOGRAPHIC INFORMATION SHOWN ON THE DRAWINGS WAS PREPARED BY AEROMETRIC, INC. FROM LIDAR DATA COLLECTED ON MARCH

SHEET INDEX

SHEET NO. TITLE

MECHANICAL INFRASTRUCTURE GENERAL DRAWINGS

ME-001 LOCATION MAP AND SITE MAP
ME-002 LECEND & SHEET INDEX
ME-003 MINE SITE - MINE DRAINAGE FLOW DIAGRAM
ME-004 MINE SITE - SUMP, POND AND PIPE DETAIL TABLES

TREATED WATER PIPELINE DRAWINGS

TWP-001 GENERAL LAYOUT AND SHEET INDEX TWP-002 PLAN & PROFILE STATION 113+70 TO 130+00 TWP-003 PLAN & PROFILE STATION 130+00 TO 190+00 TWP-004 PLAN & PROFILE STATION 190+00 TO 250+00 TWP-005 PLAN & PROFILE STATION 250+00 TO 310+00 | TWP-005 | PLAN & PROFILE SIATION 250+00 | 10 310+00 | TWP-006 | PLAN & PROFILE STATION 370+00 | TO 430+00 | TWP-008 | PLAN & PROFILE STATION 430+00 | TO 490+00 | TWP-009 | PLAN & PROFILE STATION 430+00 | TO 490+00 | TWP-010 | PLAN & PROFILE STATION 490+00 | TO 512+50 | TWP-011 | DETAILS | TWP-012 | DETAILS | TWP-013 | DETAILS | TWP-014 | TWP-015 | DETAILS | TWP-015 | DETAILS | TWP-016 | TWP-017 | DETAILS | TWP-017 | TWP-017 | DETAILS | TWP-018 | TWP-019 | DETAILS | TWP-019 | TWP-TWP-011 DETAILS IN CLOSURE

CENTRAL PUMPING STATION (CPS) DRAWINGS

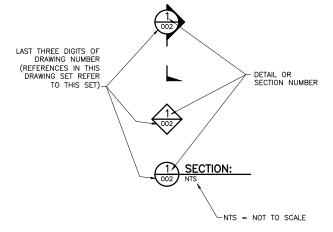
CPS-001 SITE PLAN
CPS-002 PUMP STATION PLAN
CPS-003 PROCESS FLOW DIAGRAM

SHEET NO. TITLE

MINE DRAINAGE DRAWINGS

MD-001 YEAR 1 GENERAL LAYOUT
MD-002 YEAR 11 GENERAL LAYOUT
MD-003 SUMP SOSP & MD-SOSP GRADING PLAN
MD-004 SUMP S4 & MD-S4 GRADING PLAN
MD-005 SUMP S23-1 & MD-S23-1 GRADING PLAN
MD-006 SUMP S23-2 GRADING PLAN
MD-007 SUMP S23-3 & MD-S23-3 GRADING PLAN
MD-008 POND MD-05LA GRADING PLAN
MD-009 POND MD-0TH GRADING PLAN
MD-009 POND MD-NTH GRADING PLAN
MD-010 POND MD-HTP GRADING PLAN MD-014 SUMP/POND TYPICAL DETAILS
MD-015 PIPE TYPICAL DETAILS MD-016 CLOSURE PLAN

DRAWING NUMBERING



ABBREVIATIONS

ACRE-FEET AVERAGE CATEGORY CENTERLINE AVE CAT € CMP CORRUGATED METAL PIPE CPS DIP DV DWG CENTRAL PUMPING STATION DUCTILE IRON PIPE DRAIN VALVE DRAWING ELEVATION
GALLONS
GEOSYNTHETIC CLAY LINER

EL.
GAL
GCL
GPM
HDPE
HRE
HRE
HRN
HRW
INV
LF
MD
MG
MH
MIL
MIN
MnDOT GALLONS PER MINUTE
HIGH-DENSITY POLYETHYLENE
HAUL ROAD CENTRAL
HAUL ROAD EAST HAUL ROAD NORTH HAUL ROAD WEST INVERT LINEAR FEET
MINE DRAINAGE
MILLION GALLONS

MEASUREMENT OF LINER THICKNESS; A MIL IS A THOUSANDTH OF AN INCH

MEASUREMENT OF LINER THICKNESS; A MIL IS MINIMUM MINNESOTA DEPARTMENT OF TRANSPORTATION OVERBURDEN STORAGE AND LAYDOWN AREA ORE SURGE PILE PIPELINE POUNDS PER SQUARE INCH

OSLA OSP PL PSI RTH SDR STA RAIL TRANSFER HOPPER STANDARD DIMENSION RATIO STATION
WASTE ROCK STOCKPILE STOCKPILE-TDH TWP TYP TOTAL DESIGN HEAD TREATED WATER PIPELINE VACUUM/AIR RELIEF V/A

WASTE WATER TREATMENT FACILITY

PLANT DRAWING NUMBER:

MECHANICAL INFRASTRUCTURE GENERAL DWGS LEGEND AND SHEET INDEX

ER IO	DATE	DESCRIPTION		SSUE STATUS			
	5/18/15	ISSUED FOR INCLUSION IN PERMIT APPLICATIONS	ISSUED	VERSION	DATE	I HEREBY CERTIFY THAT THIS PLAN.	
			FOR PERMITTING	1	5/18/15	SPECIFICATION, OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER	DRAWN: VJS
							CHECKED:
							CMK2
			FOR CONSTRUCTION			PRINTED NAME CHRISTIE KEARNEY SIGNATURE	BARR PROJECT NO.:
						DATE 5/18/15 LICENSE# 48864	23/69-0C29
			NOT APPROVED FOR CONSTRUCTION			DATE 9/ 10/ 13 LICENSE# 40804	SCALE:
							AS SHOWN

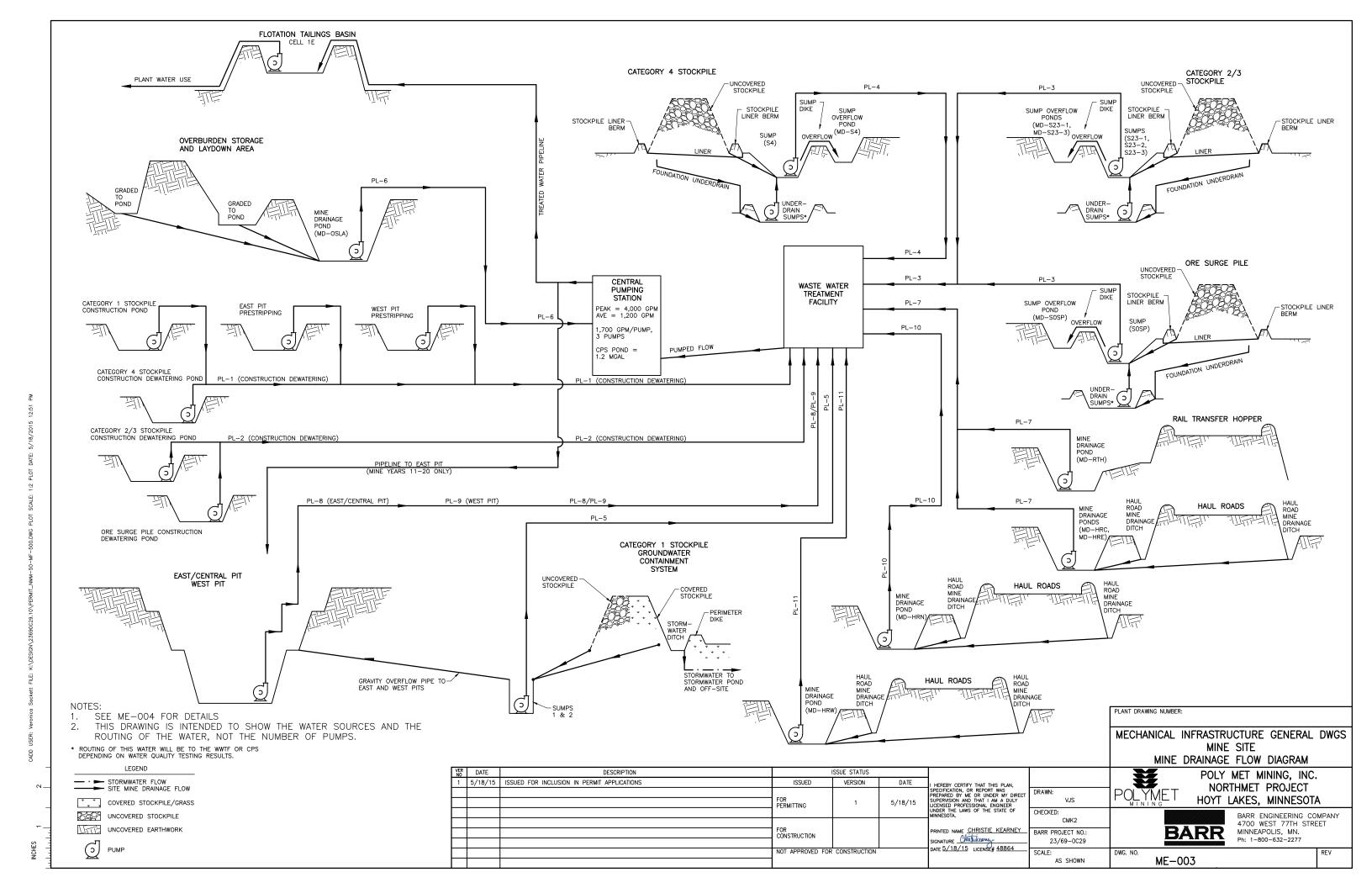
POLYME

POLY MET MINING, INC. NORTHMET PROJECT HOYT LAKES, MINNESOTA

BARR

ME-002

BARR ENGINEERING COMPANY 4700 WEST 77TH STREET MINNEAPOLIS, MN. Ph: 1-800-632-2277



SUMPS - TEMPORARY

				APPROXIMA DESIGN CAPAC				ACTUAL		
ID	DESCRIPTION	OBJECTIVES	VOLU	VOLUME* (GAL)	GPM	TDH (FT)	LINER TYPE	VOLUME* (GAL)	OVERFLOWS TO	SHOWN ON SHEET #
S23-1	CATEGORY 2/3 STOCKPILE SUMP	PROVIDE RUNOFF STORAGE FOR THE 10 YEAR 24 HOUR EVENT	1–19	4,855,000	190	210	MINE DRAINAGE SUMP LINER	4,855,000	MD-S23-1	MD-005
S23-2	CATEGORY 2/3 STOCKPILE SUMP	PROVIDE RUNOFF STORAGE FOR THE 10 YEAR 24 HOUR EVENT	3–17	3,878,000	150	230	MINE DRAINAGE SUMP LINER	3,910,000	MD-S23-1	MD-006
S23-3	CATEGORY 2/3 STOCKPILE SUMP	PROVIDE RUNOFF STORAGE FOR THE 10 YEAR 24 HOUR EVENT	6-16	2,151,000	90	270	MINE DRAINAGE SUMP LINER	2,151,000	MD-S23-3	MD-007
S4	CATEGORY 4 STOCKPILE SUMP	PROVIDE RUNOFF STORAGE FOR THE 10 YEAR 24 HOUR EVENT	1-11	3,291,000	130	50	MINE DRAINAGE SUMP LINER	4,073,000	MD-S4	MD-004
SOSP	ORE SURGE PILE SUMP	PROVIDE RUNOFF STORAGE FOR THE 10 YEAR 24 HOUR EVENT	1-20	2,770,000	80	90	MINE DRAINAGE SUMP LINER	2,835,000	MD-SOSP	MD-003

^{*} DESIGN VOLUME REFLECTS THE VOLUME REQUIRED BASED ON THE DESIGN NEEDS; WHEREAS ACTUAL VOLUME REFLECTS THE VOLUME SHOWN IN THE ATTACHED DRAWING SET. ACTUAL VOLUME DOES NOT INCLUDE ADDITIONAL VOLUME FROM 3 FEET OF FREEBOARD

MINE PIT SUMPS

				INITIAL SUMP		APPROXIMATE P - INITIAL YEAR YEA		
ID	DESCRIPTION	OBJECTIVES	MINE YEARS	CAPACITY (AC-FT)	MAXIMUM SUMP CAPACITY (AC-FT)	GPM	TDH (FT)	OVERFLOWS TO
WP-W	WEST PIT — WEST SUMP	*COLLECTION IN PIT	2-20	6.6	14.0	YEAR 2: 820 YEAR 20: 1,590	YEAR 2: 120 YEAR 20: 740	NONE
WP-E	WEST PIT — EAST SUMP	*COLLECTION IN PIT	10-20	4.7	9.4	YEAR 10: 530 YEAR 20: 1,050	YEAR 10: 110 YEAR 20: 350	NONE
EP	EAST PIT	*COLLECTION IN PIT	1-20	11.6	19.5	YEAR 1: 1,520 YEAR 11: 2,340	YEAR 1: 120 YEAR 11: 750	NONE
CP	CENTRAL PIT	*COLLECTION IN PIT	11-20	3.8	3.8	YEAR 11: 440 YEAR 16: 440	YEAR 1: 60 YEAR 16: 390	NONE

^{*} PIT COLLECTION IS BASED ON 40% OF THE AVERAGE ANNUAL SNOW MELT OCCURRING WITHIN ONE DAY AND THE PUMP CAPACITY DESIGNED TO REMOVE THAT SNOW MELT EVENT WITHIN 3 DAYS

SUMPS - PERMANENT

				MINIMUM	APPROXIMATE PUMP CAPACITY				
ID	DESCRIPTION	OBJECTIVES	MINE YEARS	CAPACITY (GAL)	GРM	TDH (FTI)	LINER TYPE	OVERFLOWS TO	SHOWN ON SHEET #
SUMP 1	CATEGORY 1 STOCKPILE SUMP – EAST	COLLECTION FOR GROUNDWATER CONTAINMENT SYSTEM	1-20+	NA – MANHOLE	7,200	50	NA - MANHOLE	EAST PIT	SEE CATEGORY 1 STOCKPILE
SUMP 2	CATEGORY 1 STOCKPILE SUMP – WEST	COLLECTION FOR GROUNDWATER CONTAINMENT SYSTEM	1-20+	NA – MANHOLE	7,200	50	NA - MANHOLE	WEST PIT	CONTAINMENT SYSTEM DRAWING SET

NOTES:

- 1. ACTUAL PUMP, PIPE, AND POND SIZES WILL BE OPTIMIZED IN FINAL DESIGN

- 1. ACTOAL POWP, PIPE, AND POND SIZES WILL BE OPTIMIZED IN FINAL DESIGN
 2. STANDARDIZED PUMP SIZE TO BE DETERMINED DURING FINAL DESIGN
 3. SOIL LINER 2 IS SHOWN IN DETAIL 2 ON SHEET CPS-002
 4. MINE DRAINAGE SUMP LINER IS SHOWN IN DETAIL 1 ON SHEET MD-014
 5. MINE DRAINAGE POND LINER IS SHOWN IN DETAIL 2 ON SHEET MD-014
 6. ALL PUMP CAPACITY FLOWS AND TDH VALUES HAVE BEEN ROUNDED

MINE DRAINAGE PONDS

				DESIGN	APPROXIMATE PUMP CAPACITY			ACTUAL		
ID	DESCRIPTION	OBJECTIVES	MINE YEARS	VOLUME* (GAL)	GРM	TDH (FT)	LINER TYPE	VOLUME* (GAL)	OVERFLOWS TO	SHOWN ON SHEET #
CPS	CENTRAL PUMPING STATION POND	STORE WATER FOR CPS CONVEYANCE	1-20+	1,200,000	4,000	450	SOIL LINER 2	1,200,000	NONE	CPS-001
MD-S23-1	CATEGORY 2/3 STOCKPILE SUMP OVERFLOW POND	PROVIDE SUMP OVERFLOW STORAGE UP TO THE 100 YEAR 24 HOUR EVENT	1-19	6,973,000	NA	NA	MINE DRAINAGE POND LINER	7,006,000	NONE	MD-005/ MD-006
MD-S23-3	CATEGORY 2/3 STOCKPILE SUMP OVERFLOW POND	PROVIDE SUMP OVERFLOW STORAGE UP TO THE 100 YEAR 24 HOUR EVENT	6-16	1,727,000	NA	NA	MINE DRAINAGE POND LINER	1,727,000	NONE	MD-007
MD-S4	CATEGORY 4 STOCKPILE SUMP OVERFLOW POND	PROVIDE SUMP OVERFLOW STORAGE UP TO THE 100 YEAR 24 HOUR EVENT	1-11	2,639,000	NA	NA	MINE DRAINAGE POND LINER	3,226,000	NONE	MD-004
MD-SOSP	ORE SURGE PILE SUMP OVERFLOW POND	PROVIDE SUMP OVERFLOW STORAGE UP TO THE 100 YEAR 24 HOUR EVENT	1-20	1,564,000	NA	NA	MINE DRAINAGE POND LINER	1,727,000	NONE	MD-003
MD-HRC	HAUL ROAD RUNOFF POND	PROVIDE FLOOD STORAGE UP TO THE 100 YEAR 24 HOUR EVENT AND REDUCE TSS	1-20	1,988,000	40	80	MINE DRAINAGE POND LINER	2,248,000	NONE	MD-011
MD-HRE	HAUL ROAD RUNOFF POND	PROVIDE FLOOD STORAGE UP TO THE 100 YEAR 24 HOUR EVENT AND REDUCE TSS	1-20	3,487,000	70	110	MINE DRAINAGE POND LINER	3,487,000	NONE	MD-010
MD-HRW	HAUL ROAD RUNOFF POND	PROVIDE FLOOD STORAGE UP TO THE 100 YEAR 24 HOUR EVENT AND REDUCE TSS	2-20	1,206,000	30	70	MINE DRAINAGE POND LINER	1,303,000	NONE	MD-012
MD-HRN	HAUL ROAD RUNOFF POND	PROVIDE FLOOD STORAGE UP TO THE 100 YEAR 24 HOUR EVENT AND REDUCE TSS	2-20	1,434,000	30	110	MINE DRAINAGE POND LINER	1,499,000	NONE	MD-013
MD-RTH	RAIL TRANSFER HOPPER RUNOFF POND	PROVIDE FLOOD STORAGE UP TO THE 100 YEAR 24 HOUR EVENT AND REDUCE TSS	1-20	228,000	200	60	MINE DRAINAGE SUMP LINER	228,000	NONE	MD-009
MD-OSLA	OVERBURDEN STORAGE & LAYDOWN AREA RUNOFF POND	PROVIDE FLOOD STORAGE UP TO THE 25 YEAR 24 HOUR EVENT AND REDUCE TSS	1-20	3,487,000	100	90	NONE	4,725,000	NONE	MD-008
TEMP (VARIOUS)	STOCKPILE CONSTRUCTION RUNOFF PONDS AND PIT STRIPPING	TEMPORARY POND TO COLLECT RUNOFF DURING CONSTRUCTION	VARIES	VARIES	VARIES	VARIES	NONE	VARIES	NONE	NONE

^{*}DESIGN VOLUME REFLECTS THE VOLUME REQUIRED BASED ON THE DESIGN NEEDS; WHEREAS ACTUAL VOLUME REFLECTS THE VOLUME SHOWN IN THE ATTACHED DRAWING SET. ACTUAL VOLUME DOES NOT INCLUDE ADDITIONAL VOLUME FROM 3 FEET OF FREEBOARD (1 FOOT FOR MD-RTH)

<u>PIPING</u>

ID	DESCRIPTION	OBJECTIVES	WATER SOURCE	NOMINAL PIPI SIZES* (IN)
TWP	TREATED WATER PIPELINE	TRANSPORT WATER FROM THE CPS TO FLOTATION TAILINGS BASIN	CPS	20
PL-1	PIPELINE NUMBER 1	TRANSPORT CONSTRUCTION WATER TO THE WWTF	TEMP — CAT 1, CAT 4, EAST PIT & WEST PIT	2 TO 8
PL-2	PIPELINE NUMBER 2	TRANSPORT CONSTRUCTION WATER TO THE WWTF	TEMP - CAT 2/3 + OSP	2 TO 8
PL-3	PIPELINE NUMBER 3	TRANSPORT MINE DRAINAGE TO THE WWTF	CAT 2/3 & OSP	3 TO 8
PL-4	PIPELINE NUMBER 4	TRANSPORT MINE DRAINAGE TO THE WWTF	CAT 4	4
PL-5	PIPELINE NUMBER 5	TRANSPORT MINE DRAINAGE TO THE WWTF	CAT 1	28 TO 42
PL-6	PIPELINE NUMBER 6	TRANSPORT MINE DRAINAGE TO THE CPS	OSLA	3
PL-7	PIPELINE NUMBER 7	TRANSPORT MINE DRAINAGE TO THE WWTF	RTH, MD-HRE & MD-HRC	2 TO 6
PL-8	PIPELINE NUMBER 8	TRANSPORT MINE DRAINAGE TO THE WWTF	EAST PIT & CENTRAL PIT	12 TO 20
PL-9	PIPELINE NUMBER 9	TRANSPORT MINE DRAINAGE TO THE WWTF	WEST PIT	10 TO 22
PL-10	PIPELINE NUMBER 10	TRANSPORT MINE DRAINAGE TO THE WWTF	MD-HRN	2
PL-11	PIPELINE NUMBER 11	TRANSPORT MINE DRAINAGE TO THE WWTF	MD-HRW	2

PLANT DRAWING NUMBER:

MECHANICAL INFRASTRUCTURE GENERAL DWGS MINE SITE SUMP, POND AND PIPE DETAIL TABLES

R	DATE	DESCRIPTION	ı	SSUE STATUS			
	5/18/15	ISSUED FOR INCLUSION IN PERMIT APPLICATIONS	ISSUED	VERSION	DATE	I HEREBY CERTIFY THAT THIS PLAN,	
			FOR PERMITTING	1		SPECIFICATION, OR REPORT WAS PREPARED BY ME OR UNDER W DIRECT SUPERVISION AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER	DRAWN: VJS
						UNDER THE LAWS OF THE STATE OF MINNESOTA.	CHECKED: CMK2
			FOR CONSTRUCTION			PRINTED NAME CHRISTIE KEARNEY SIGNATURE	BARR PROJECT NO.: 23/69-0C29
			NOT APPROVED FOR CONSTRUCTION			DATE 5/18/15 LICENSE# 48864	SCALE:



AS SHOWN

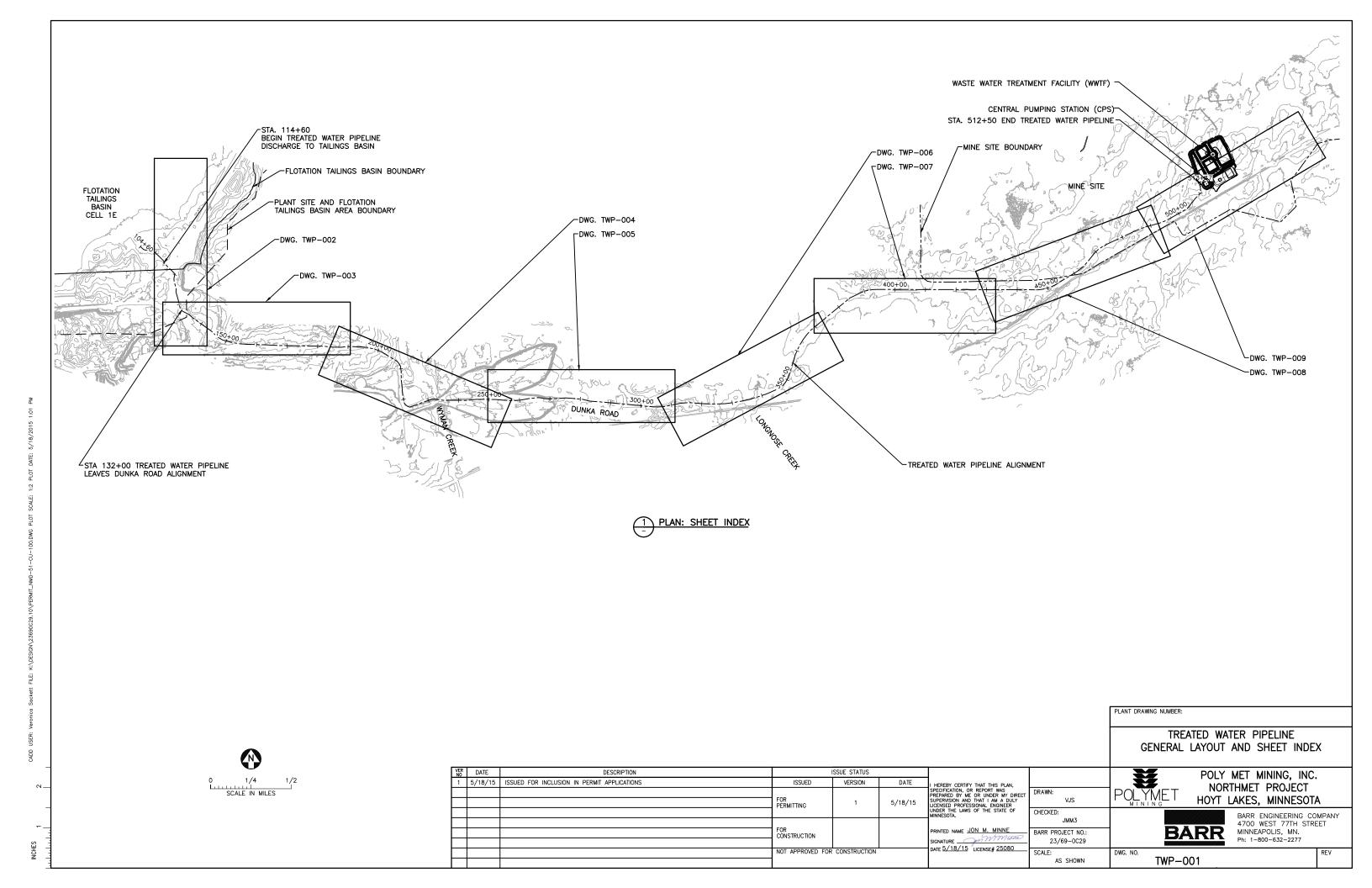
POLY MET MINING, INC. NORTHMET PROJECT HOYT LAKES, MINNESOTA

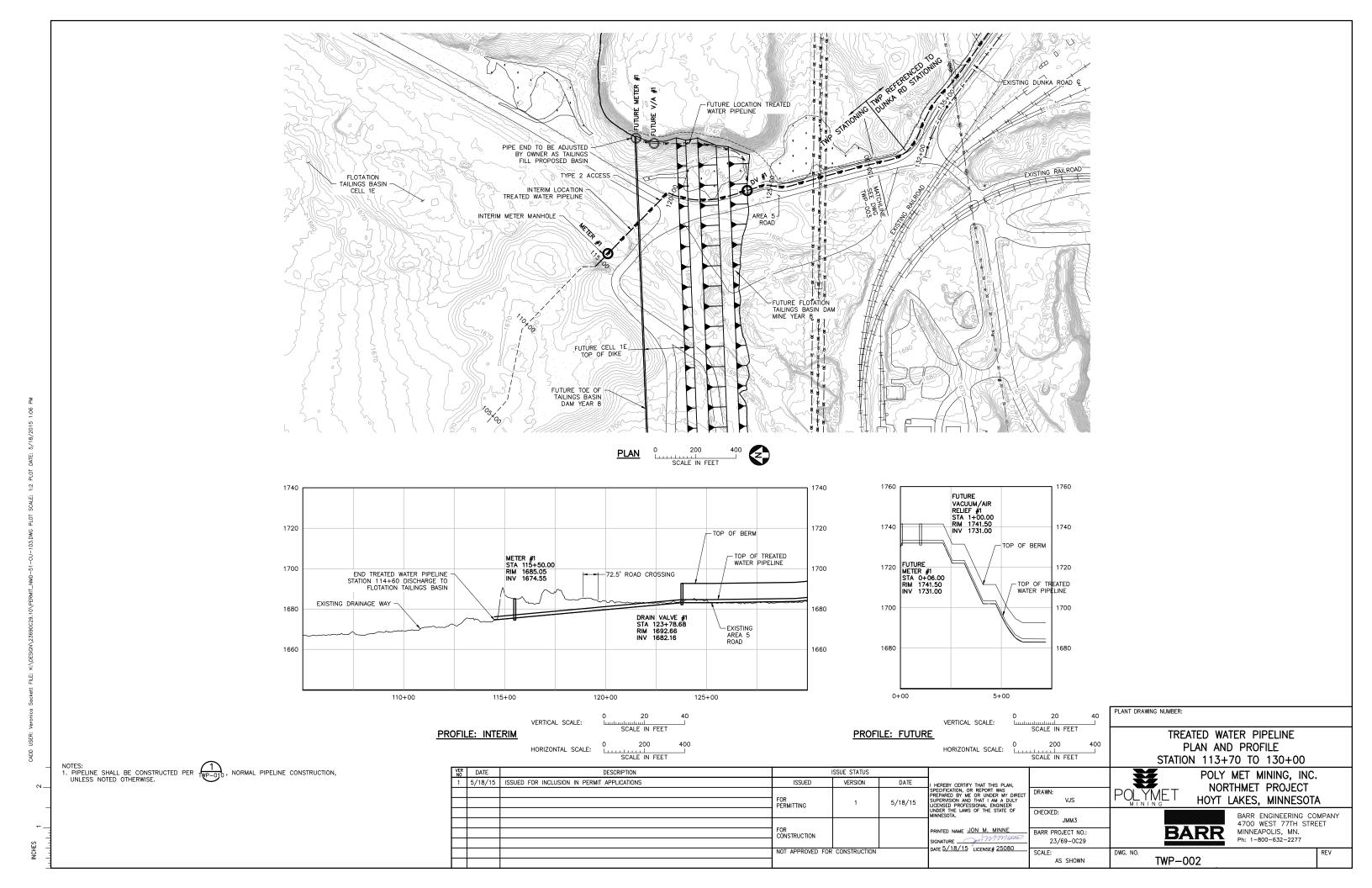
BARR

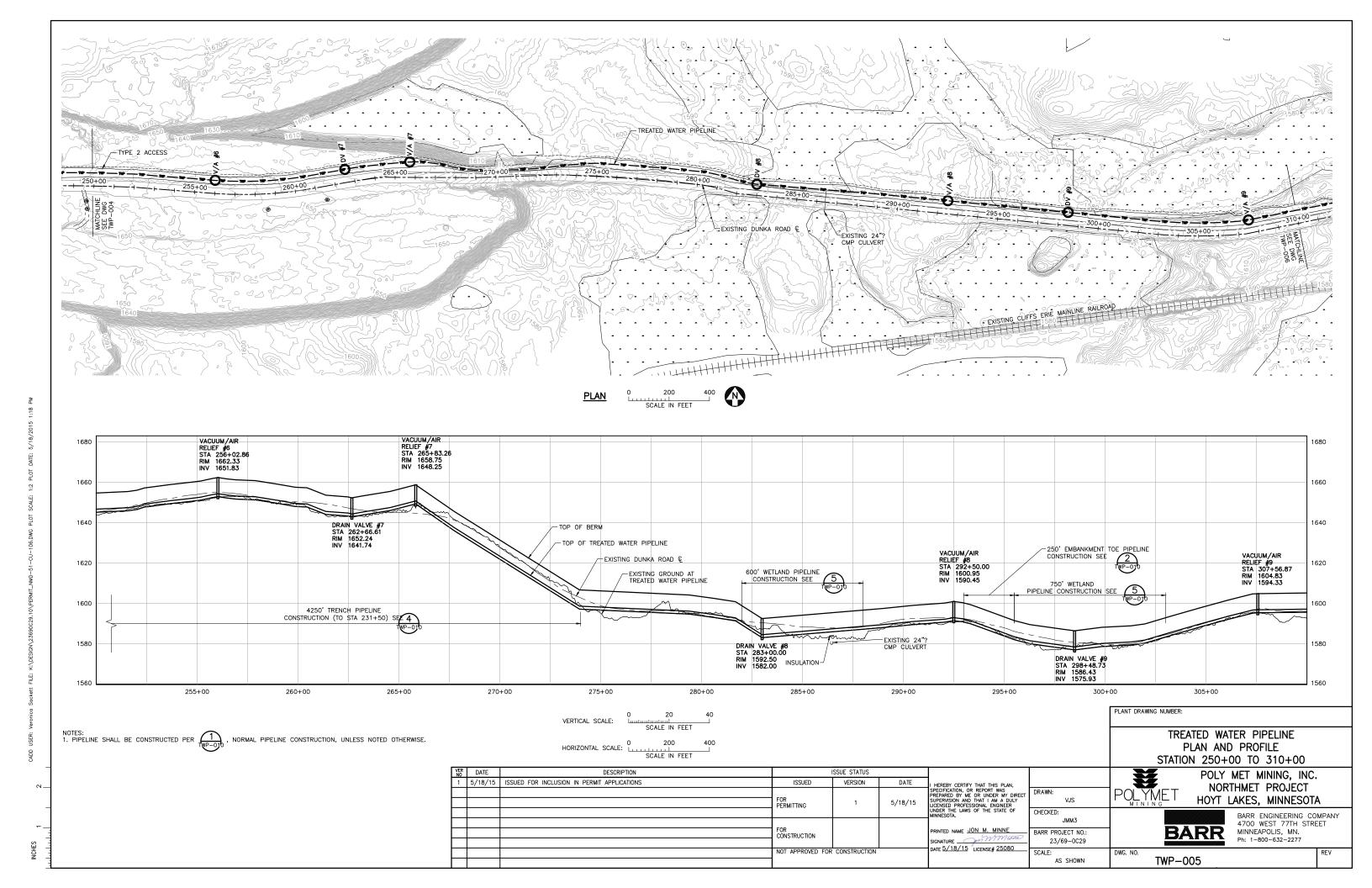
BARR ENGINEERING COMPANY 4700 WEST 77TH STREET MINNEAPOLIS, MN.

Ph: 1-800-632-2277

ME-004







- PROPOSED CENTRAL PUMPING STATION 5

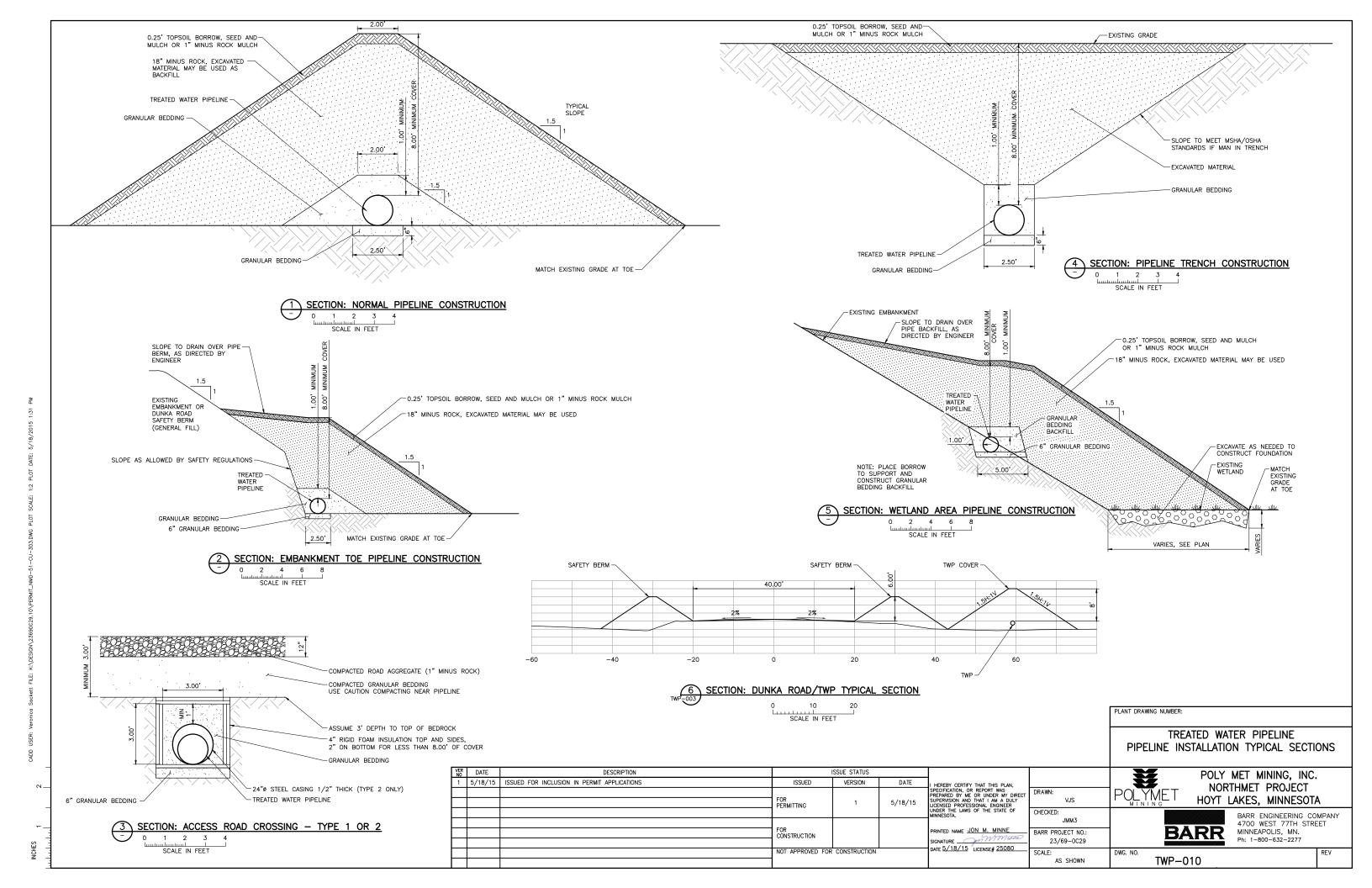
PIPE FROM CONSTRUCTION WATER BASIN

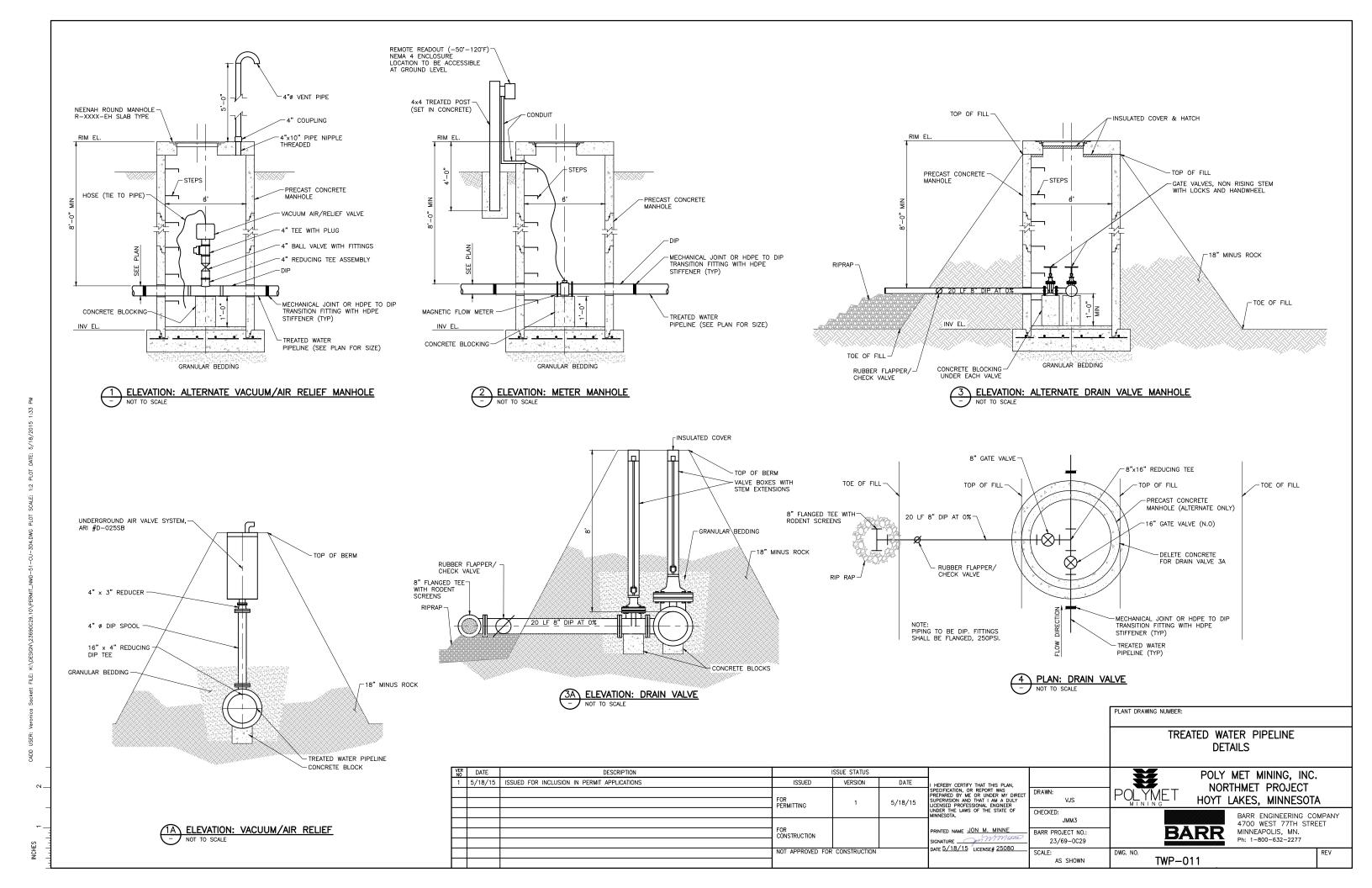
WASTE WATER TREATMENT FACILITY

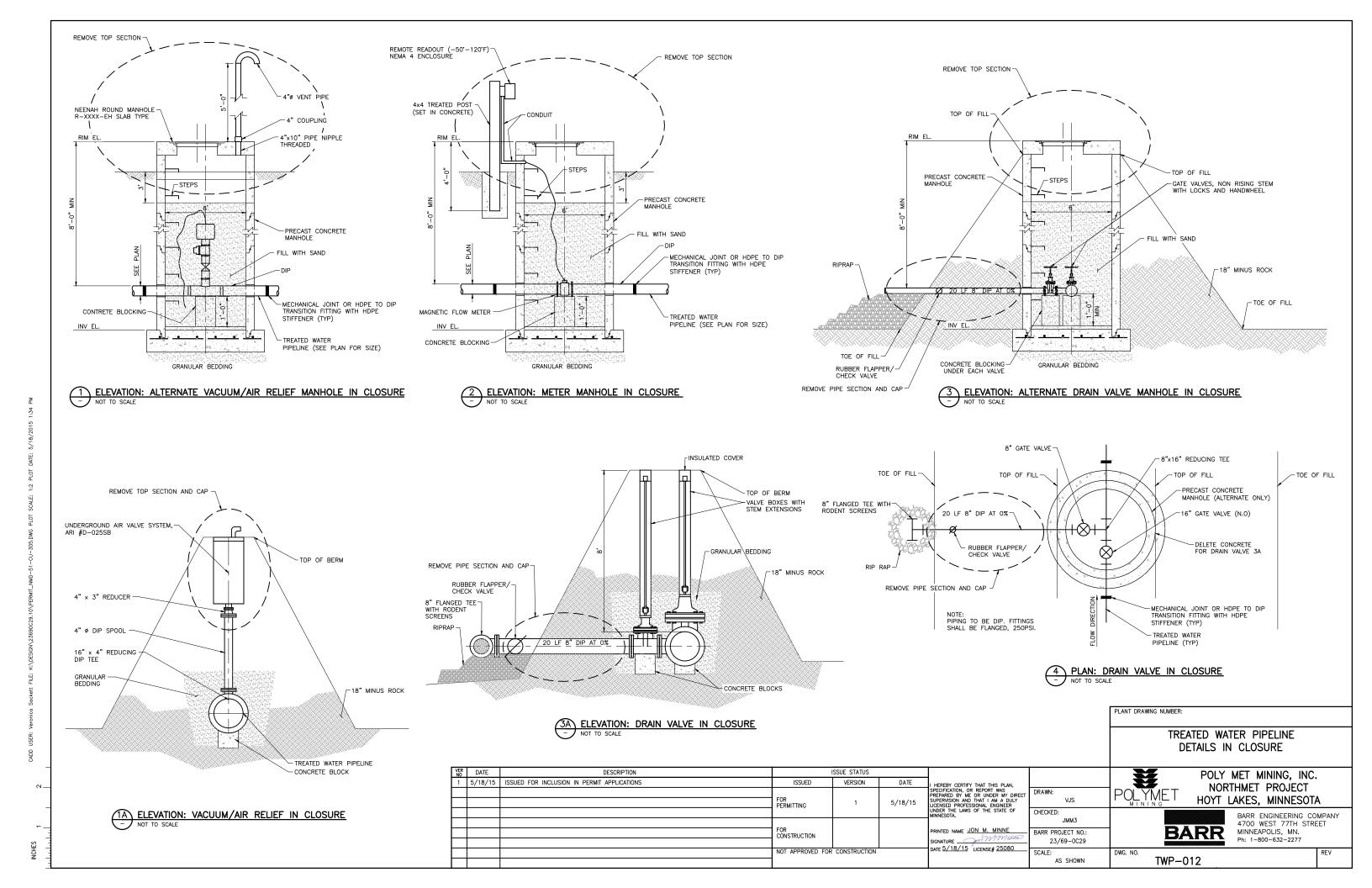
(CPS) SEE SHEET CPS-001

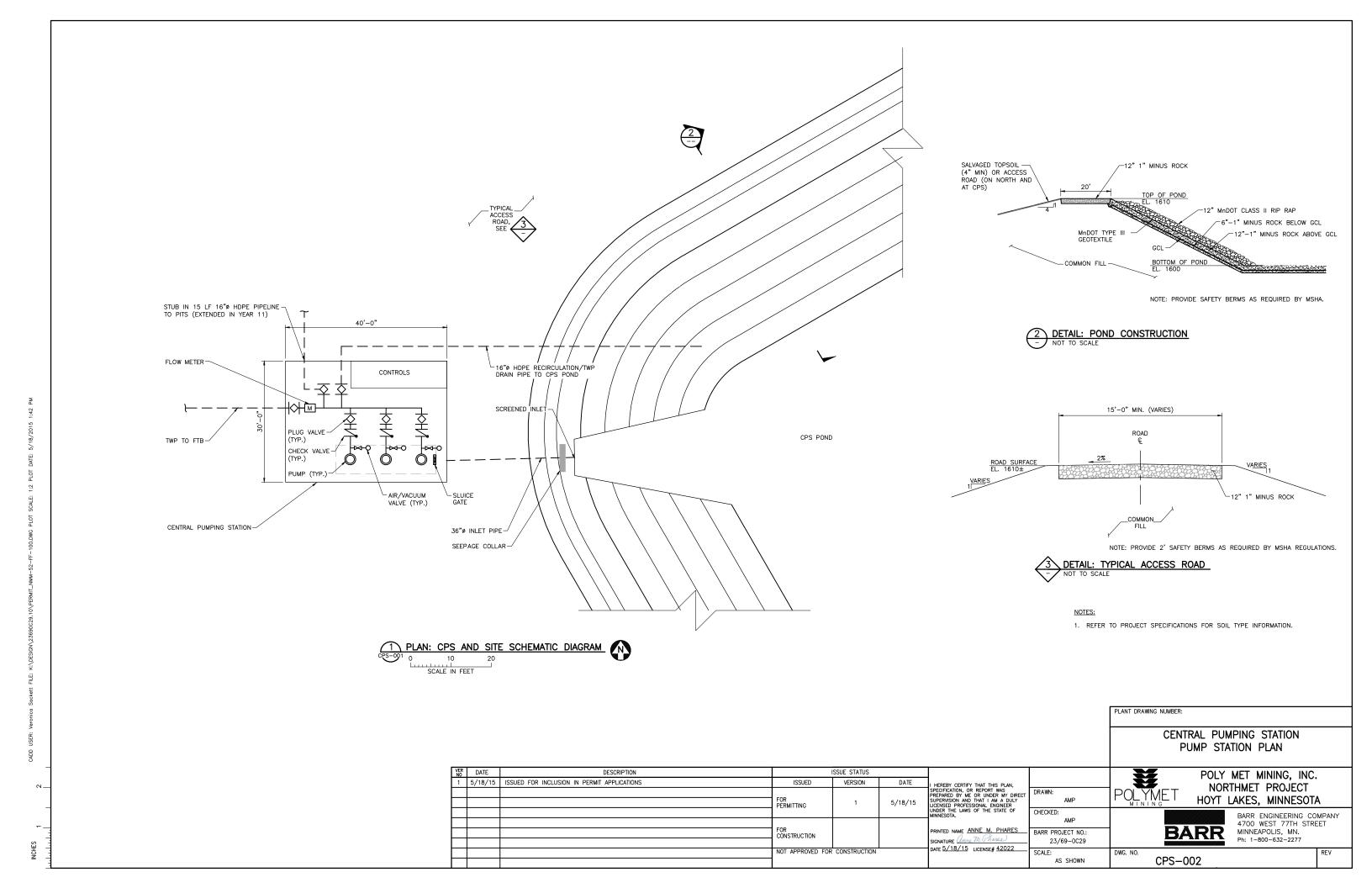
CONNECT TO CPS -DISCHARGE PIPE

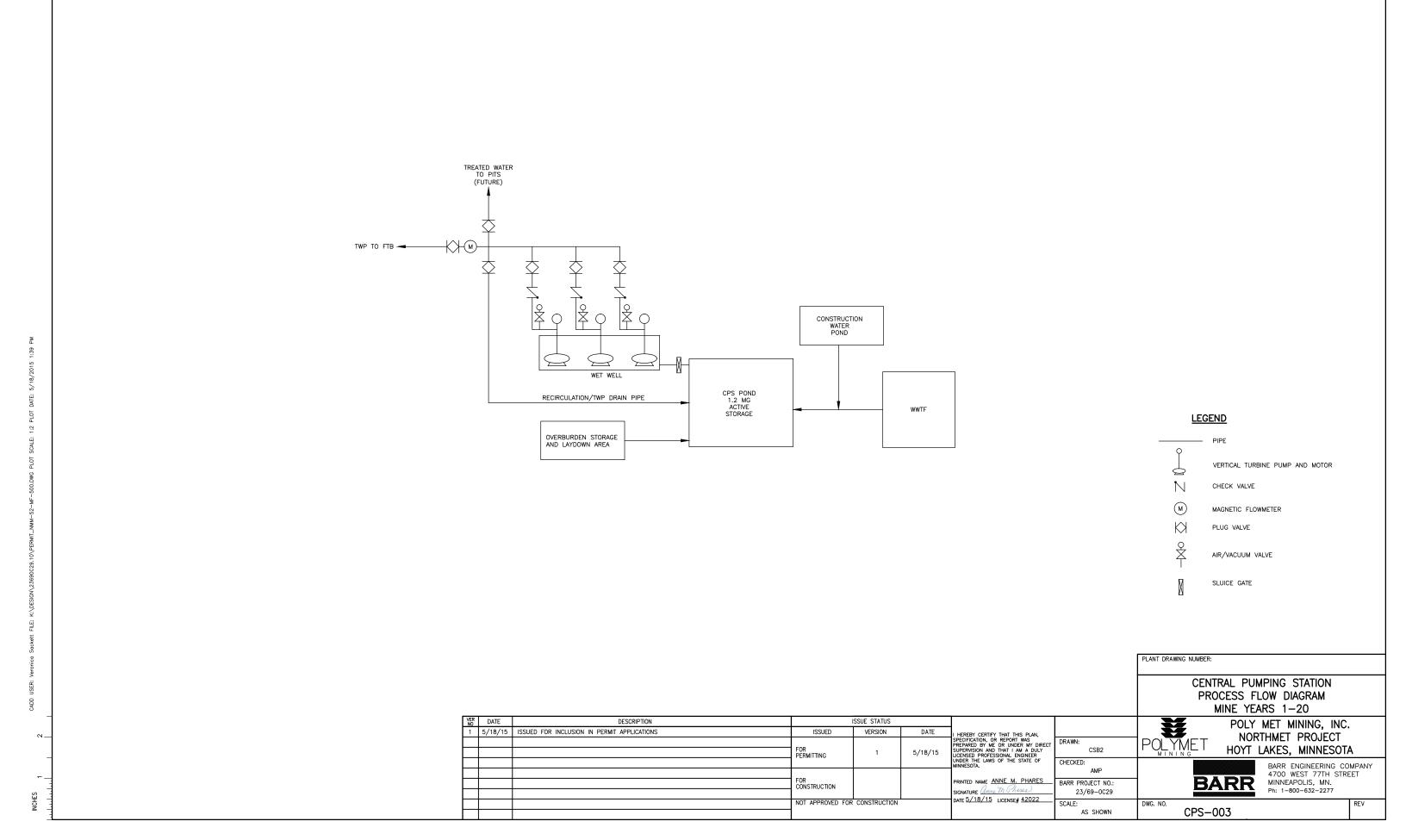
HAUL ROAD CENTERLINE

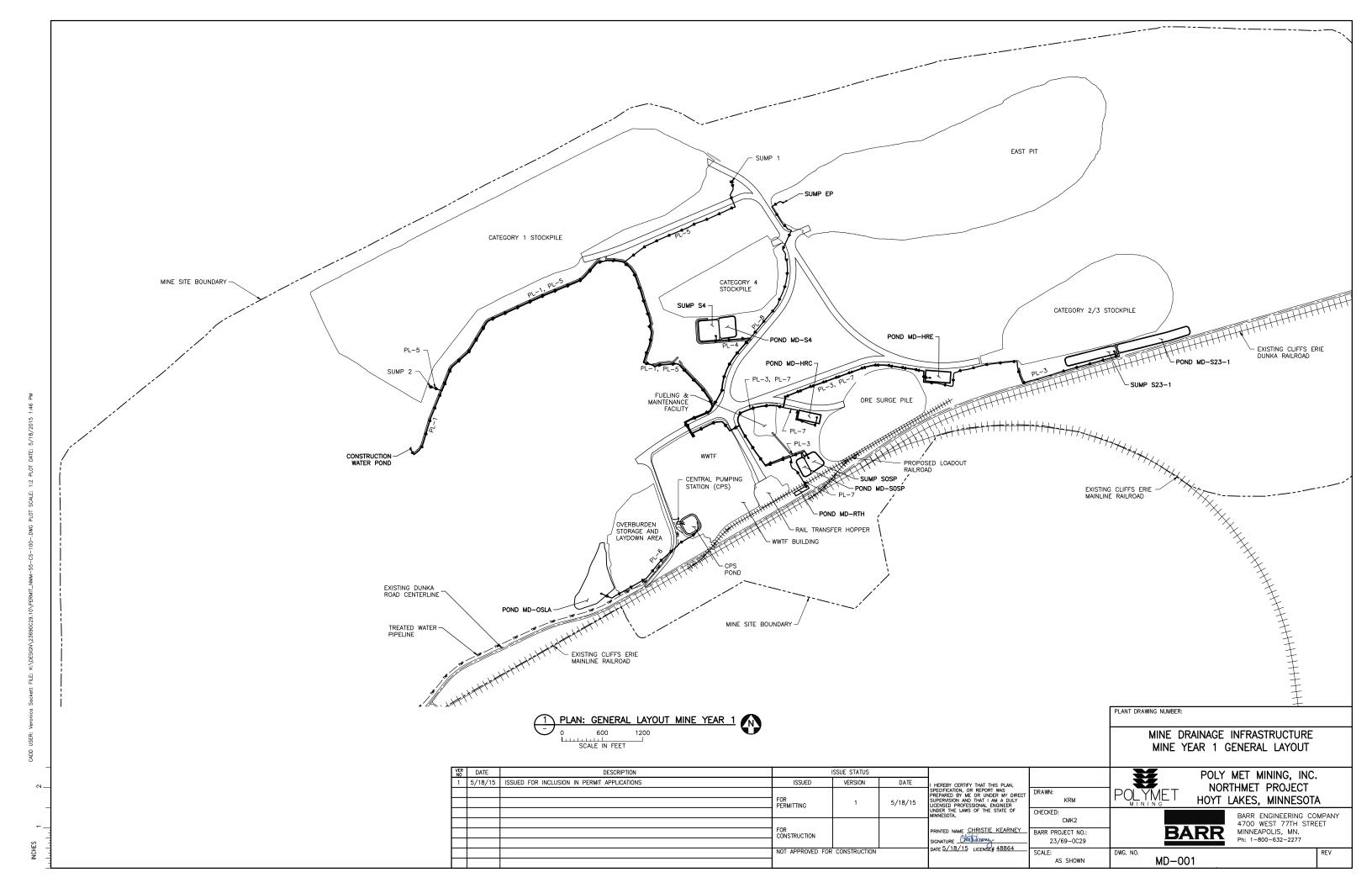


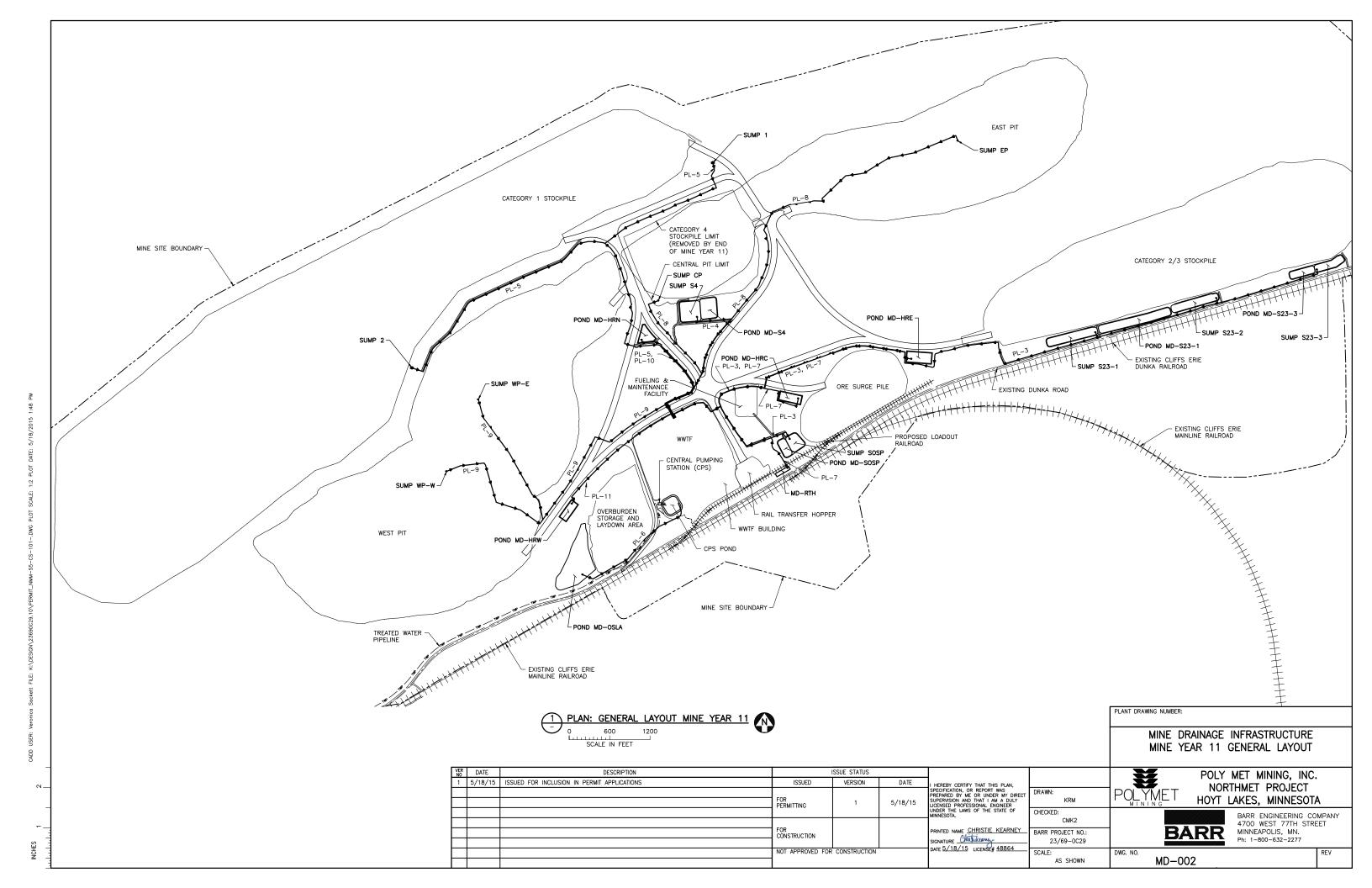


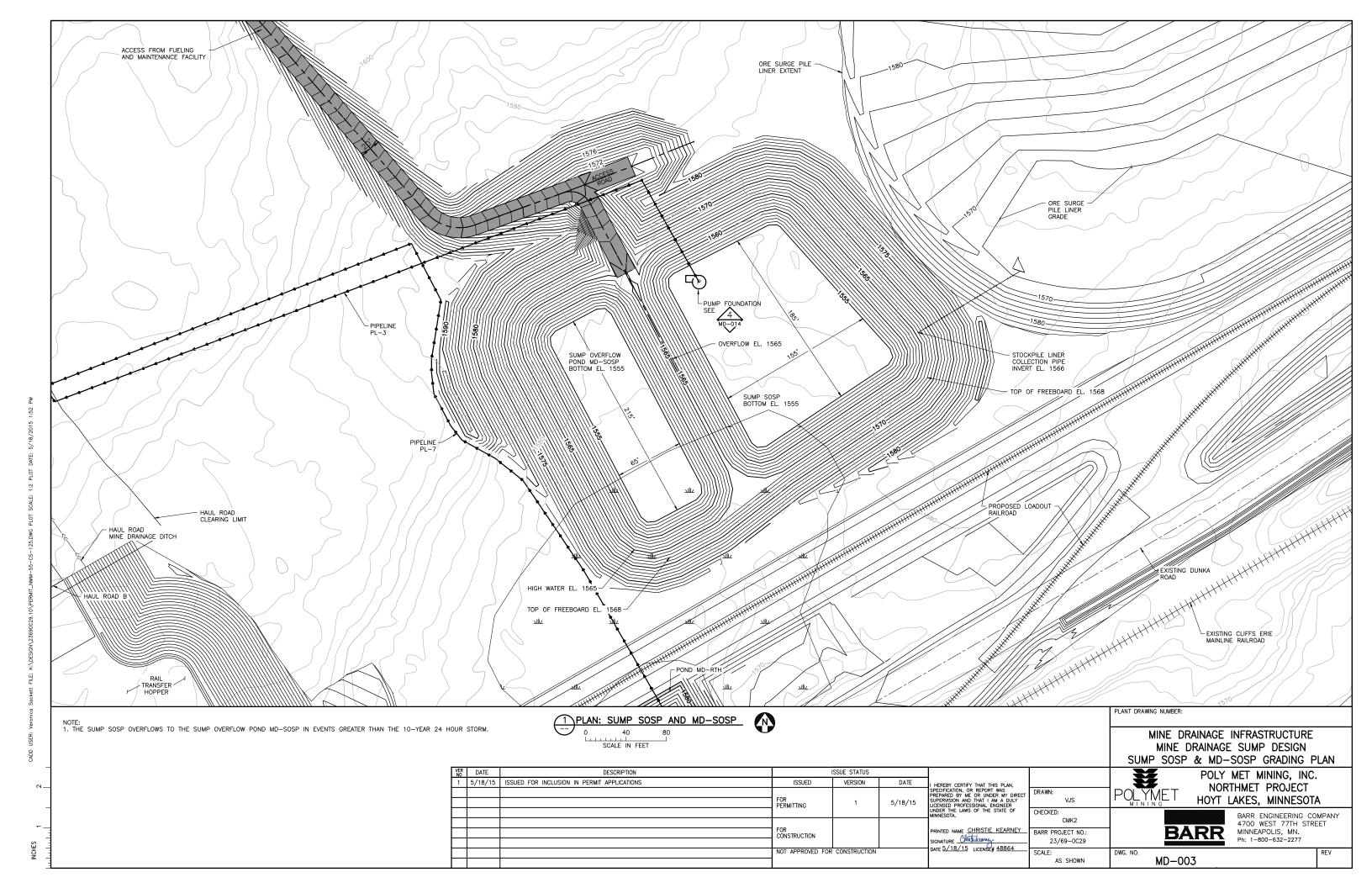


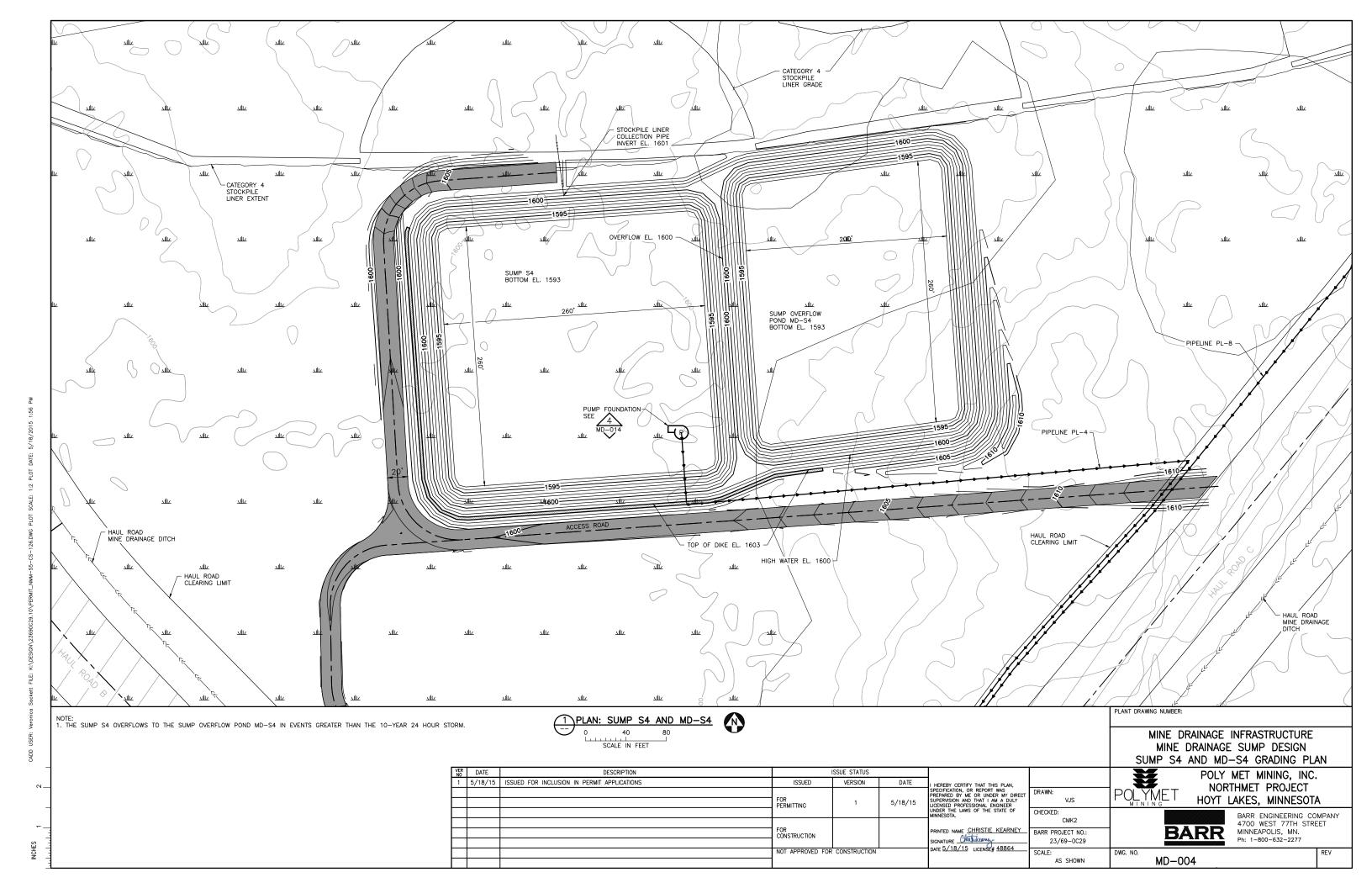


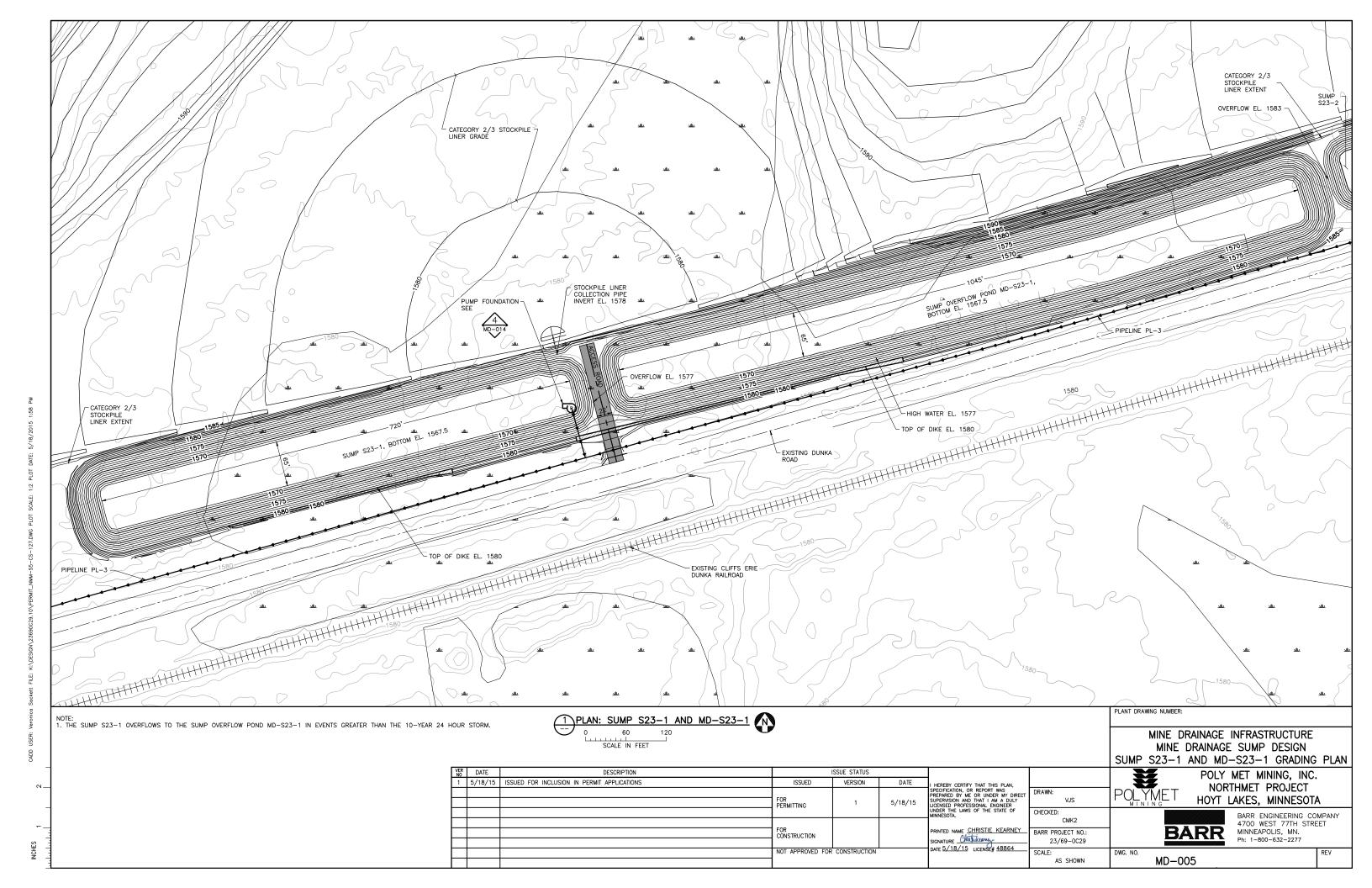


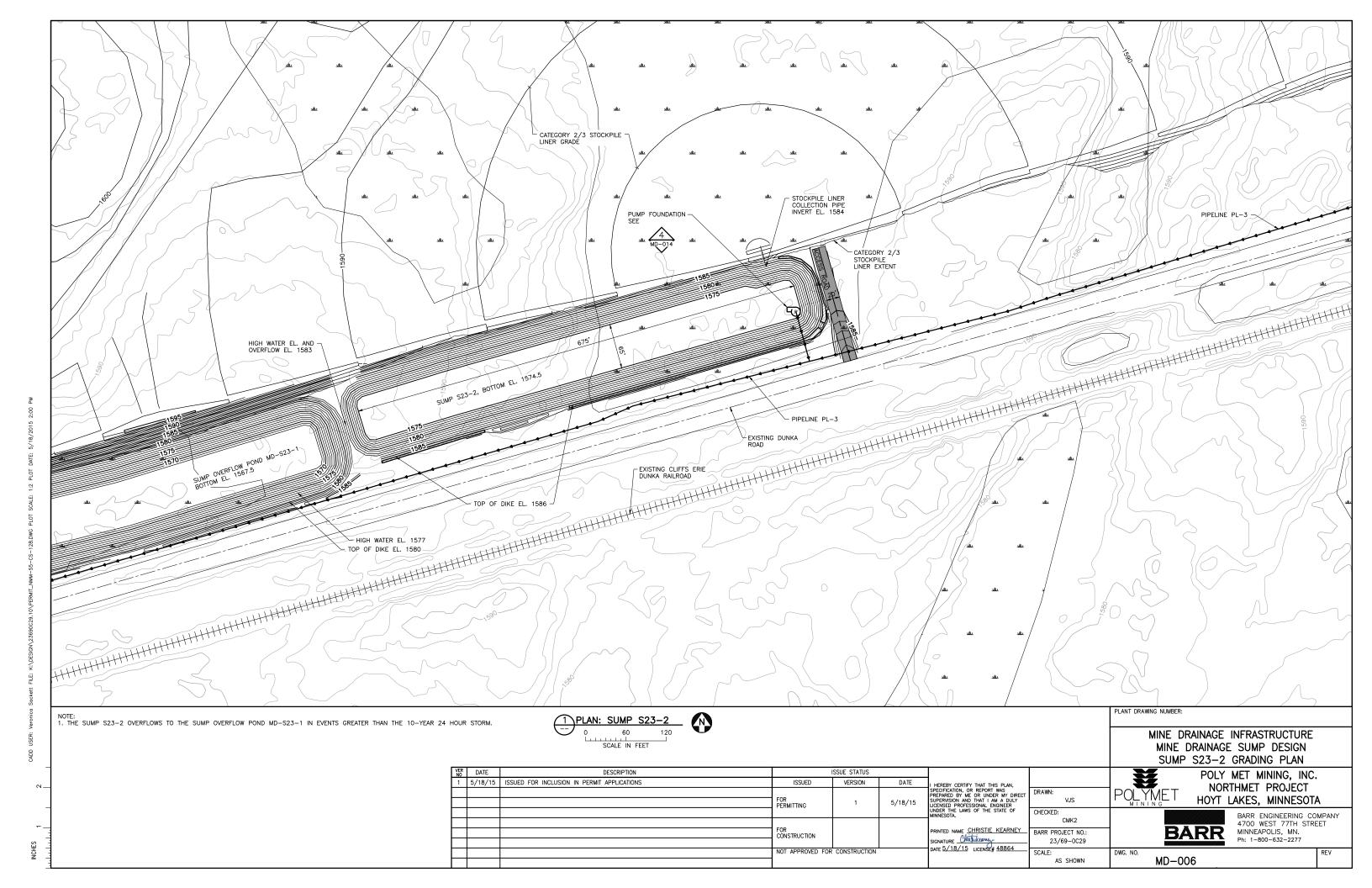


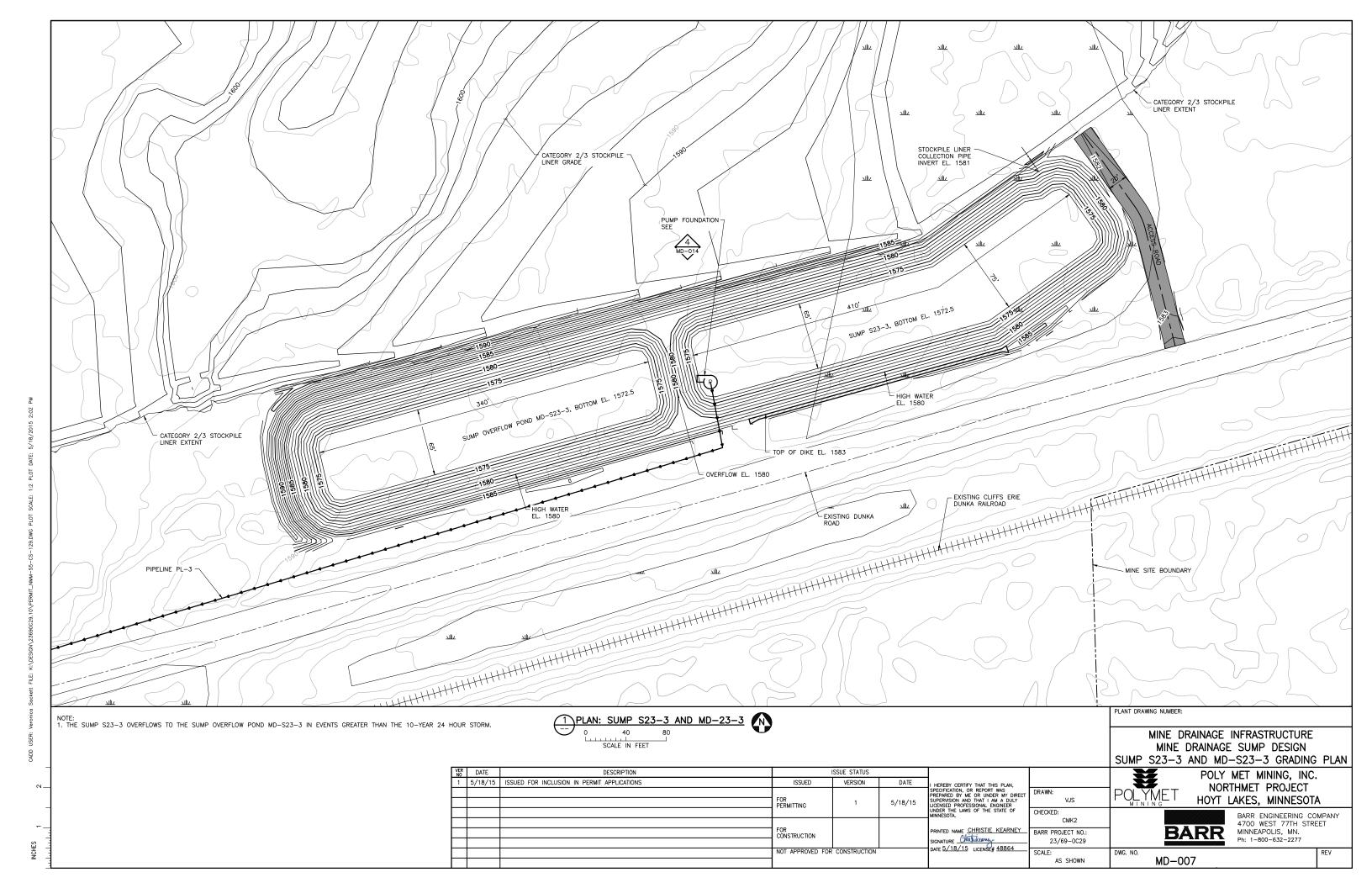


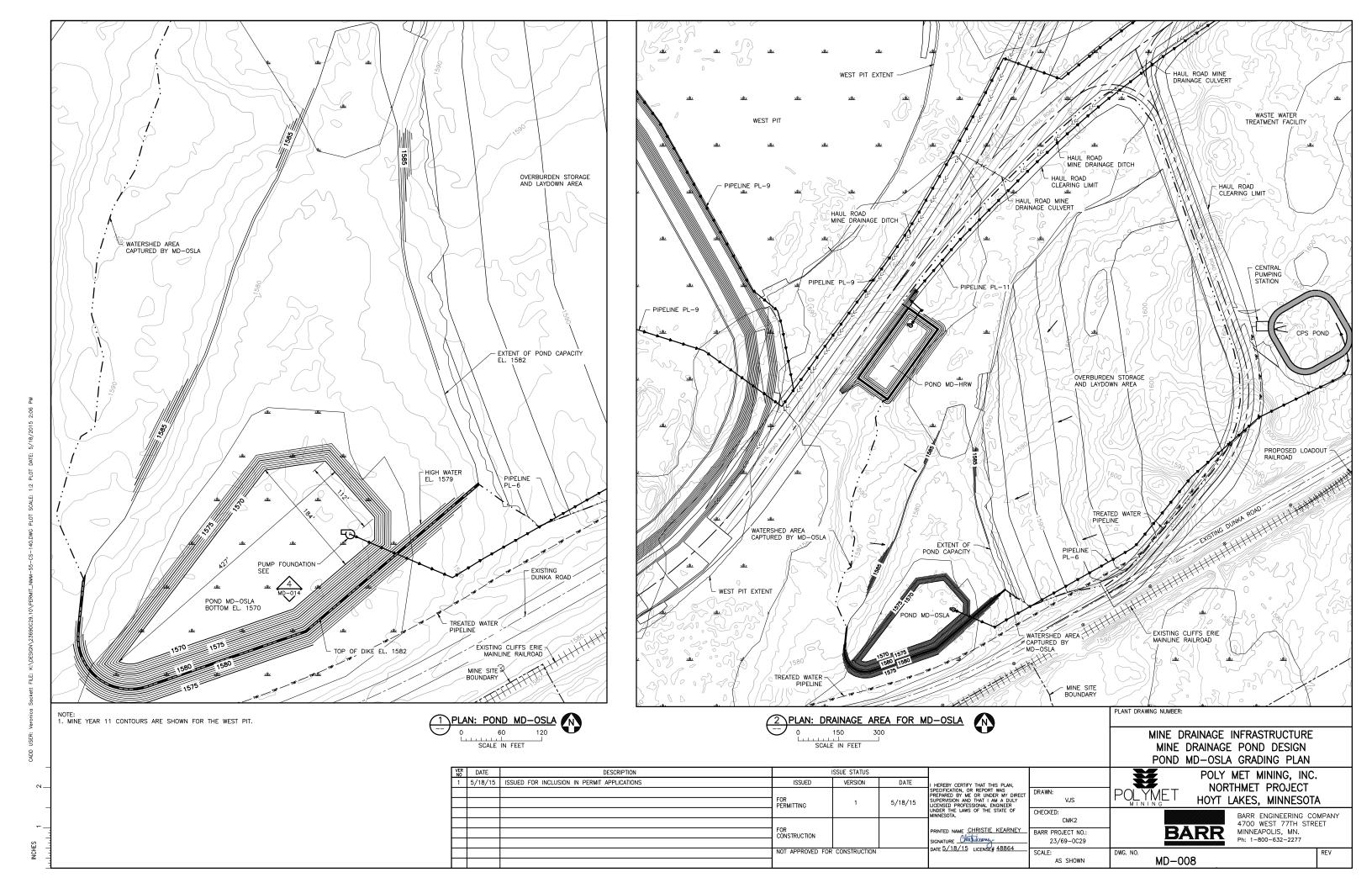


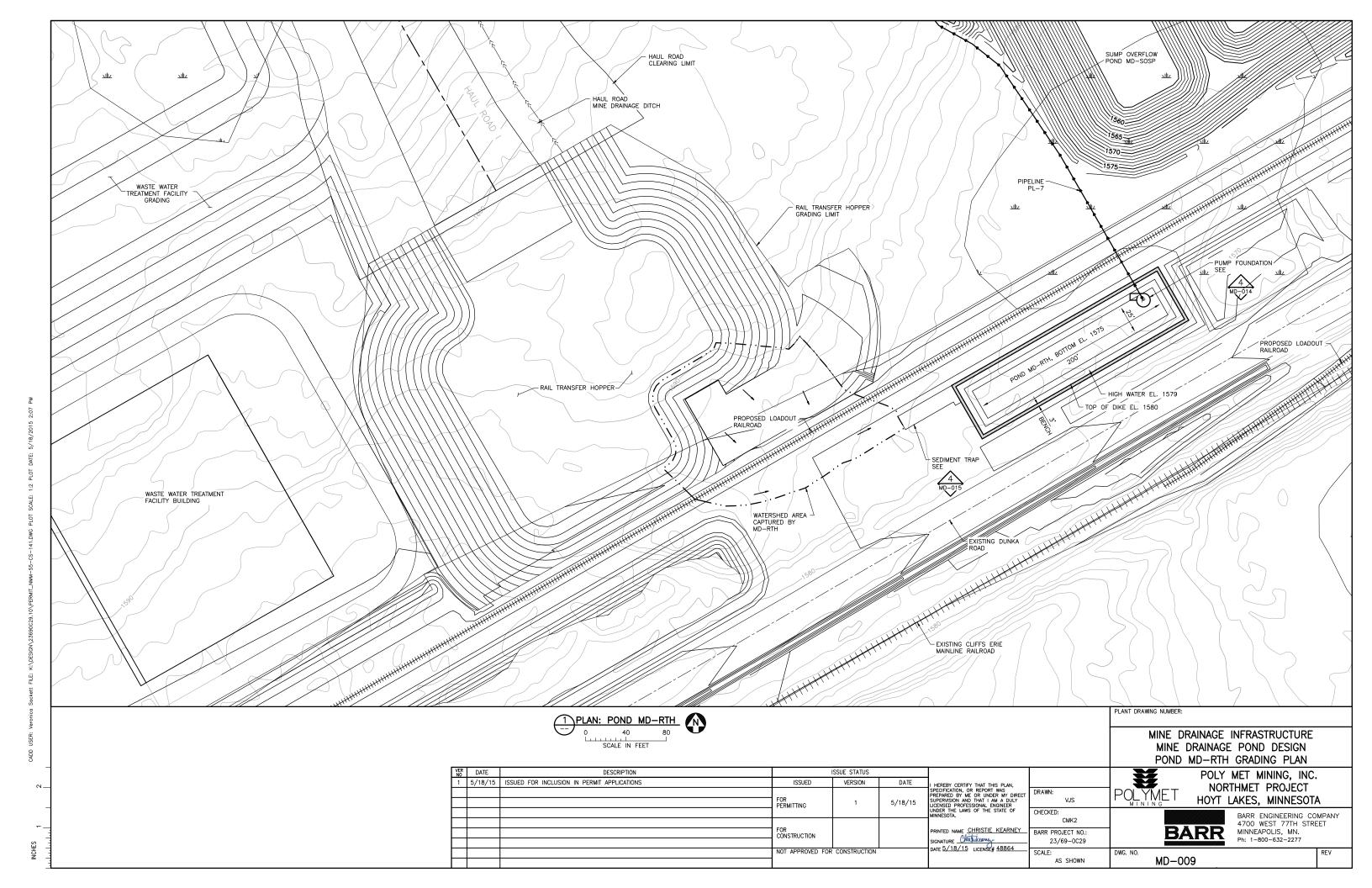


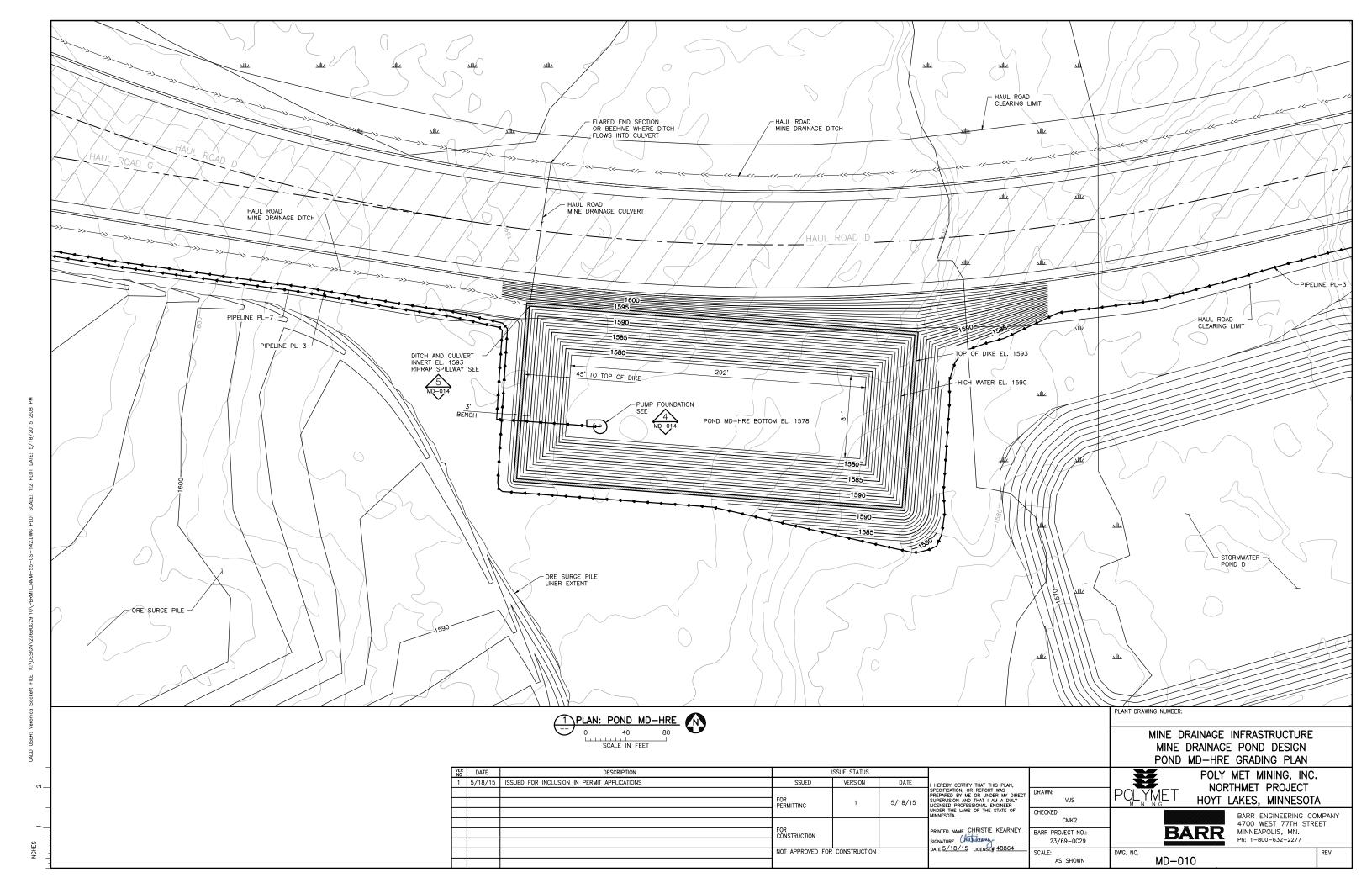


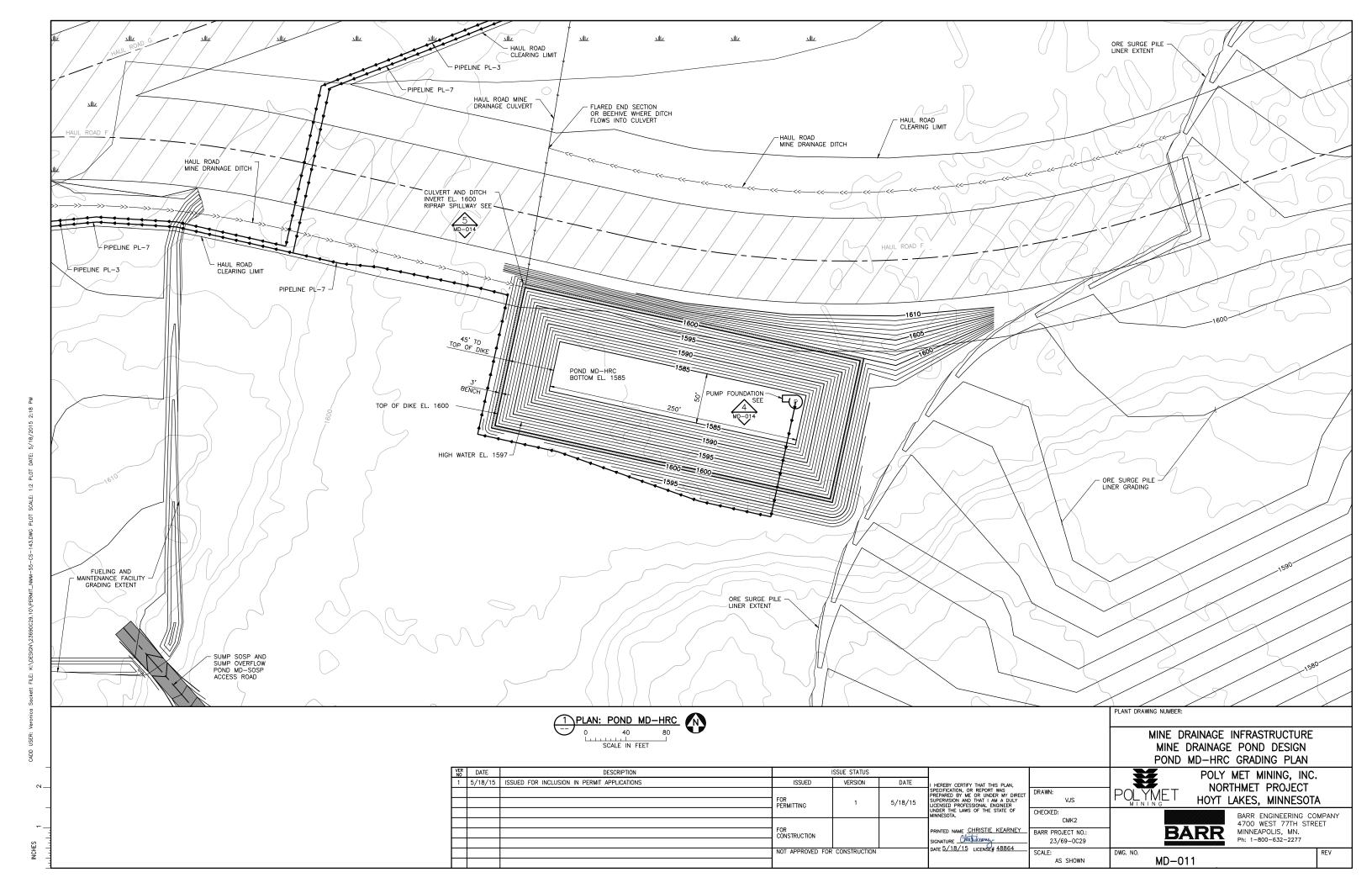


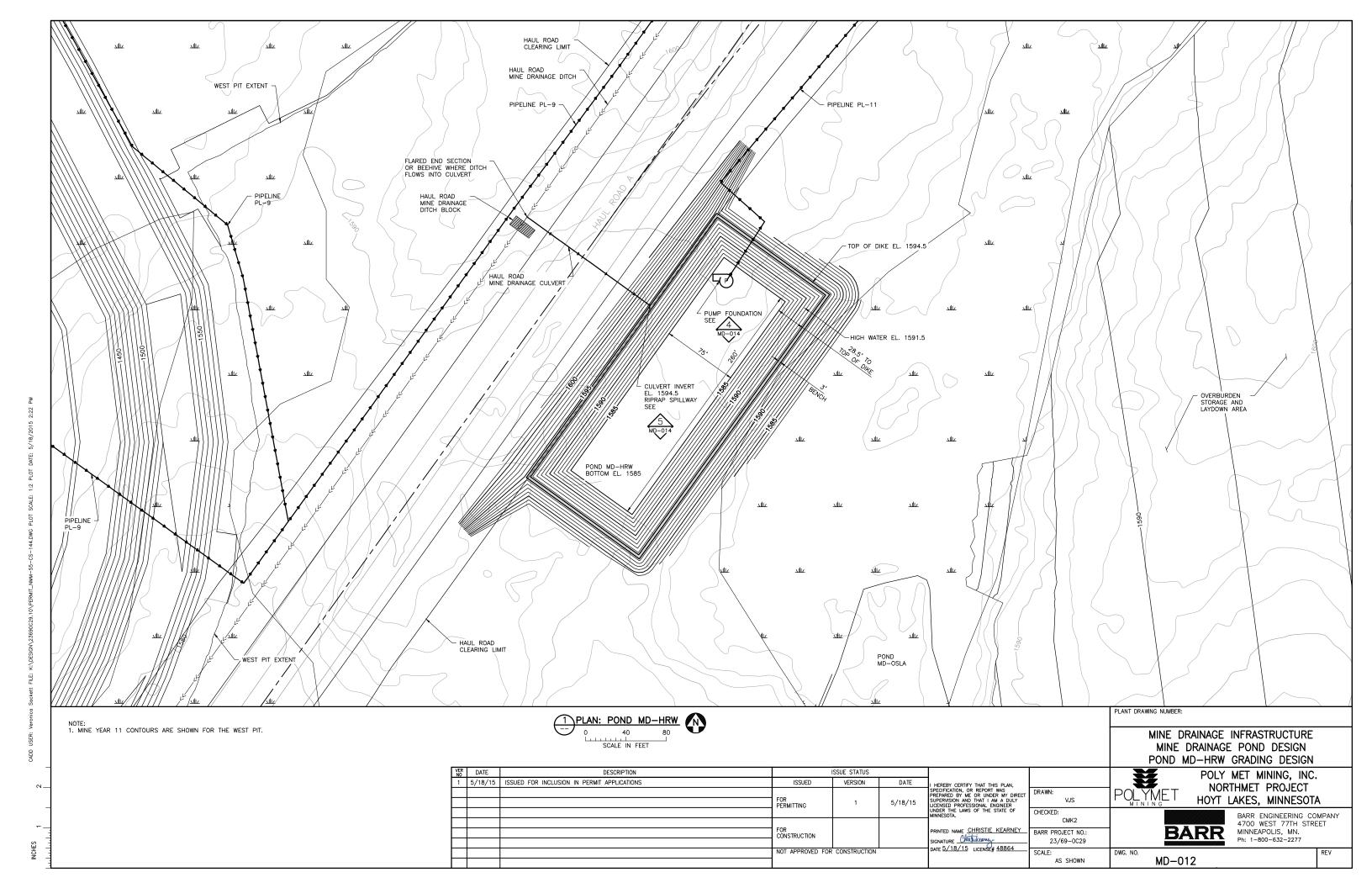


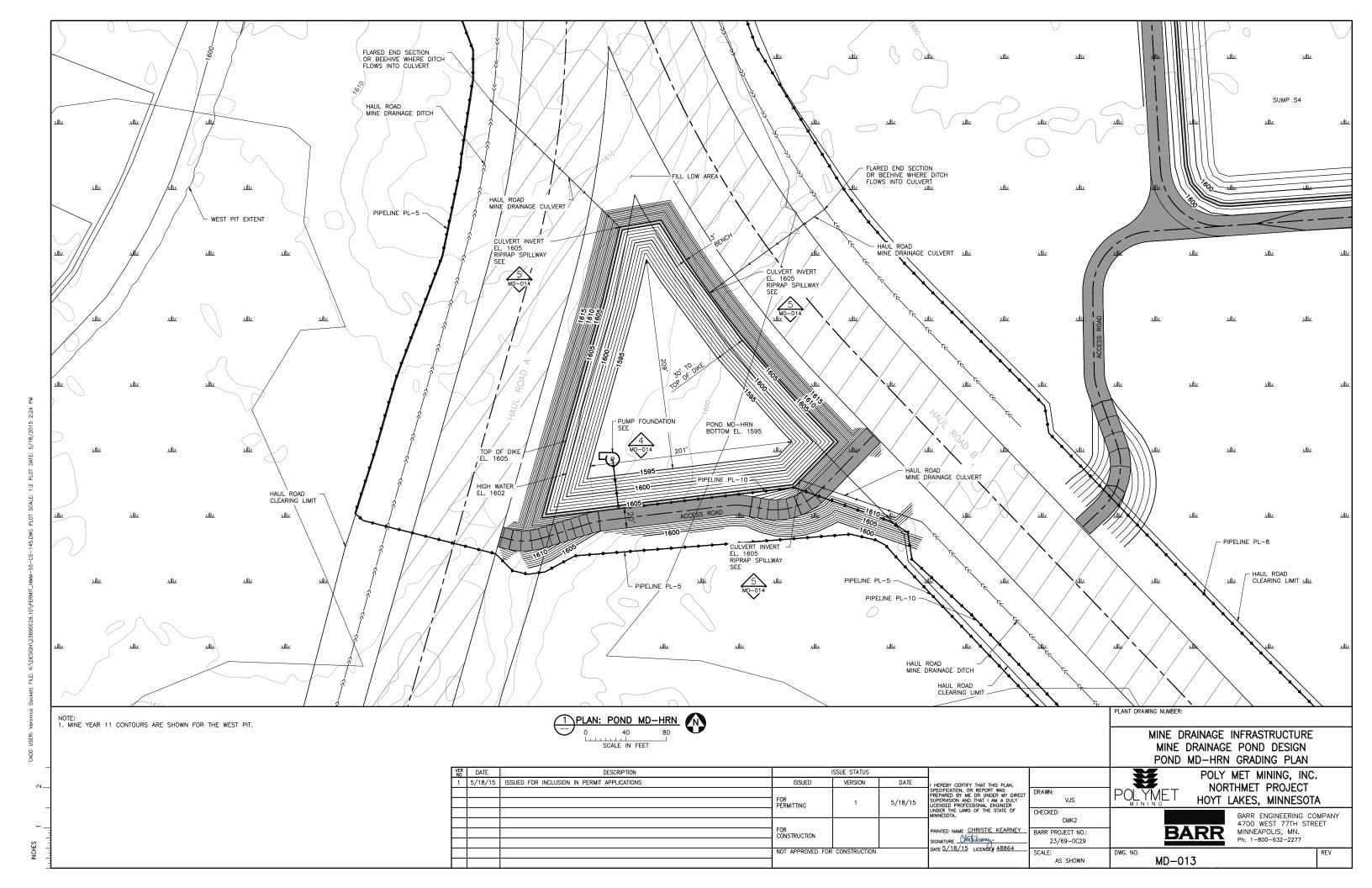


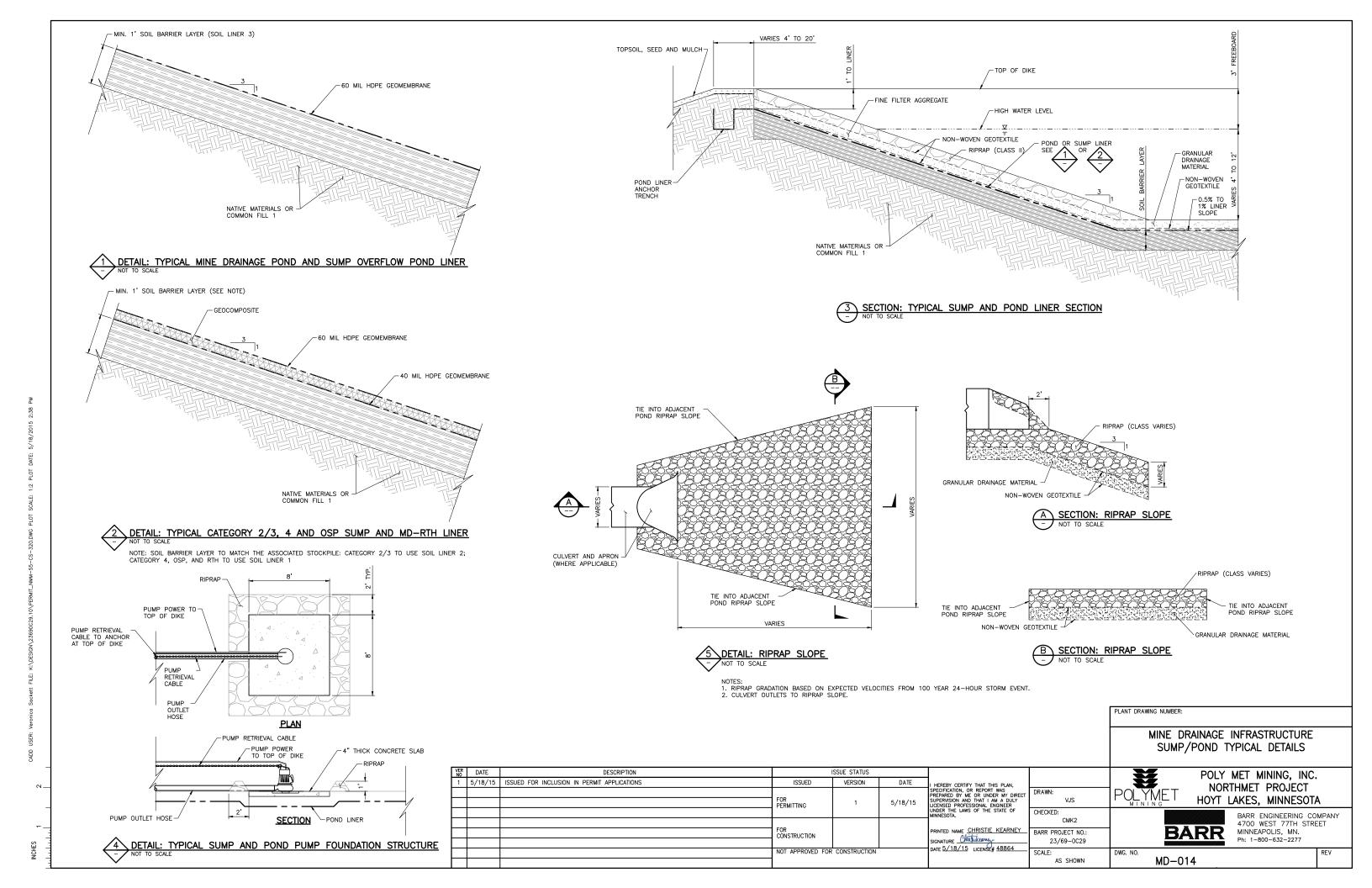


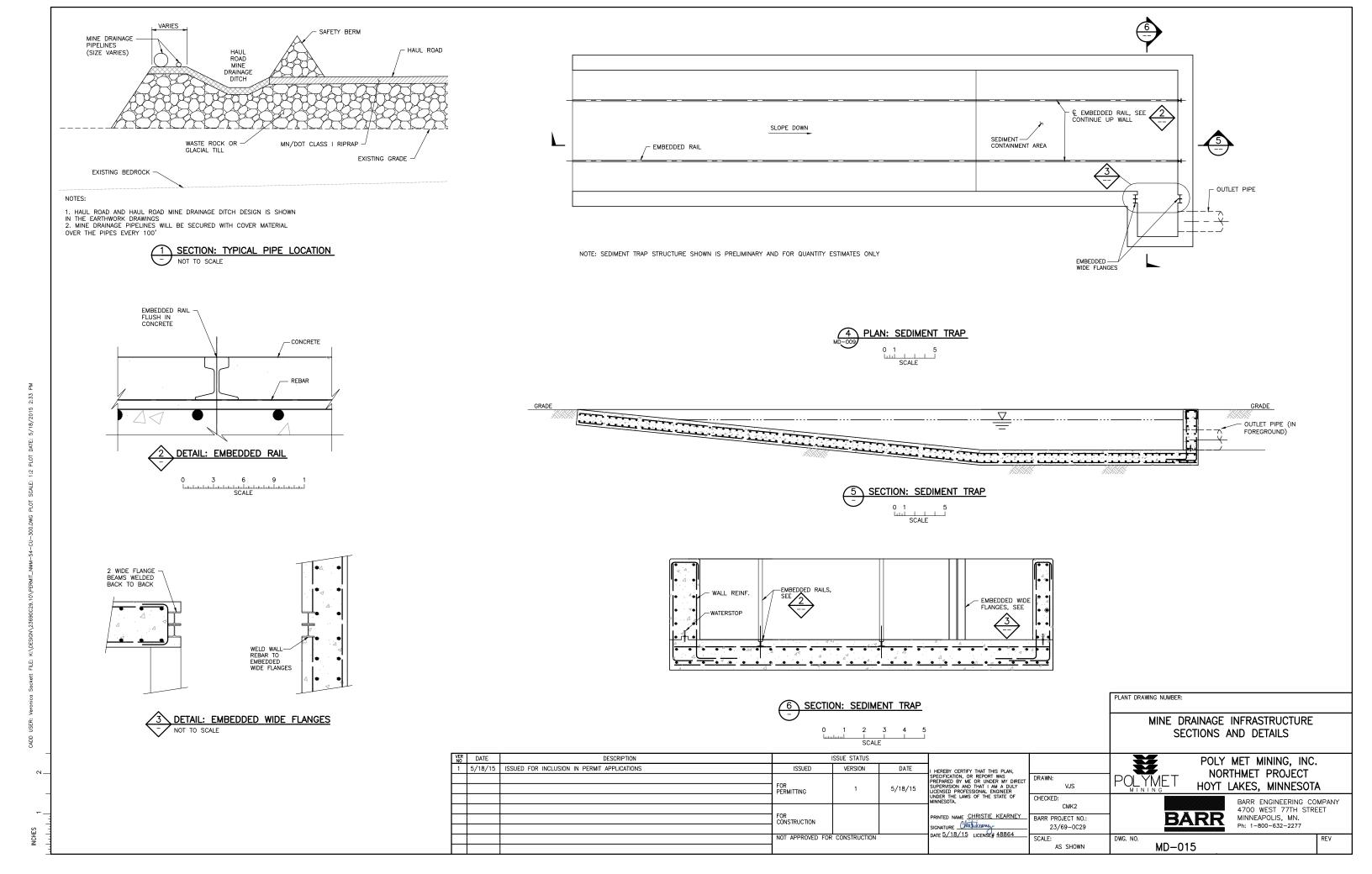


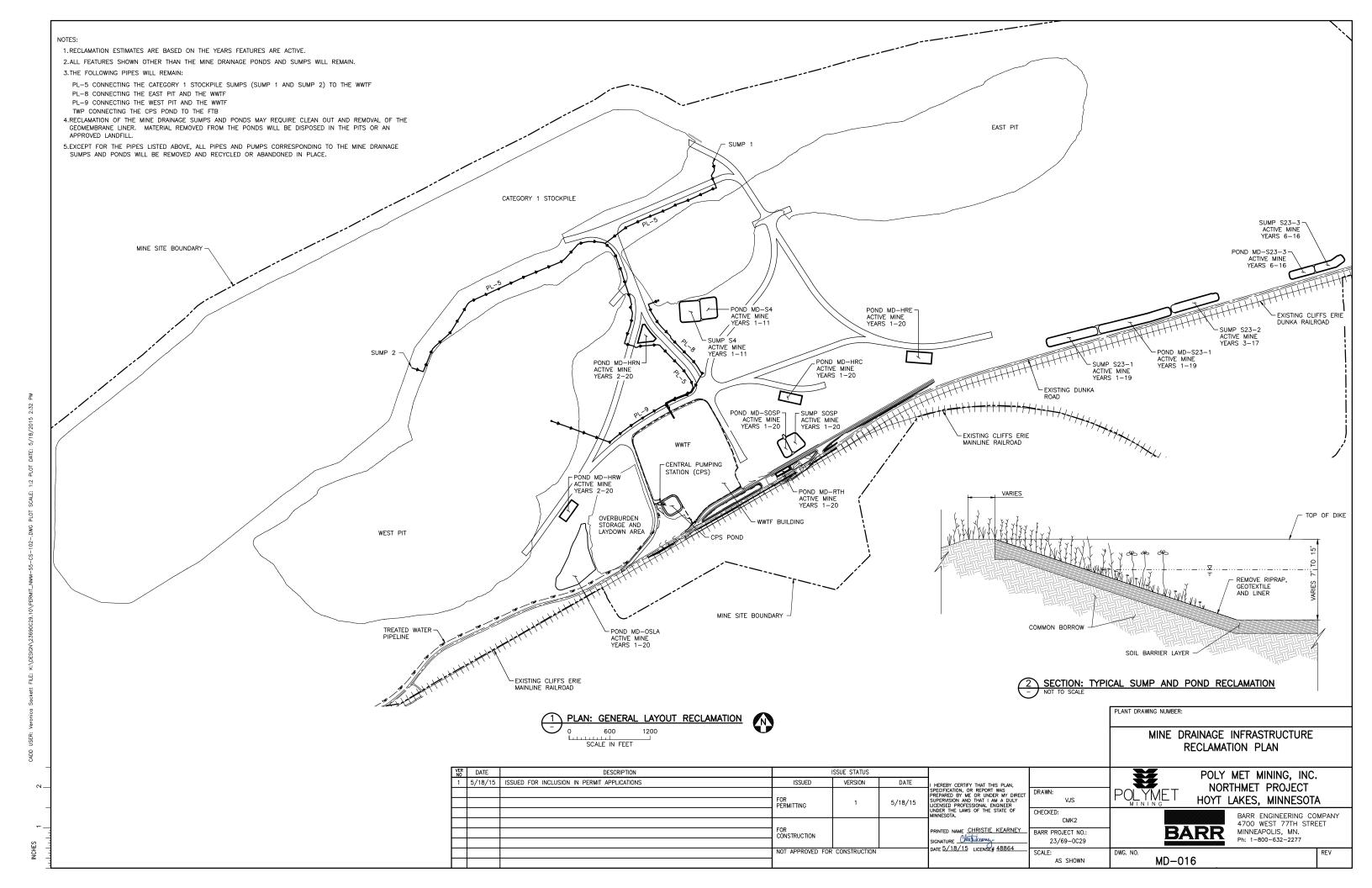












Attachment B

Stormwater Design Drawings

Errata Sheet

Poly Met Mining Inc. NorthMet Project

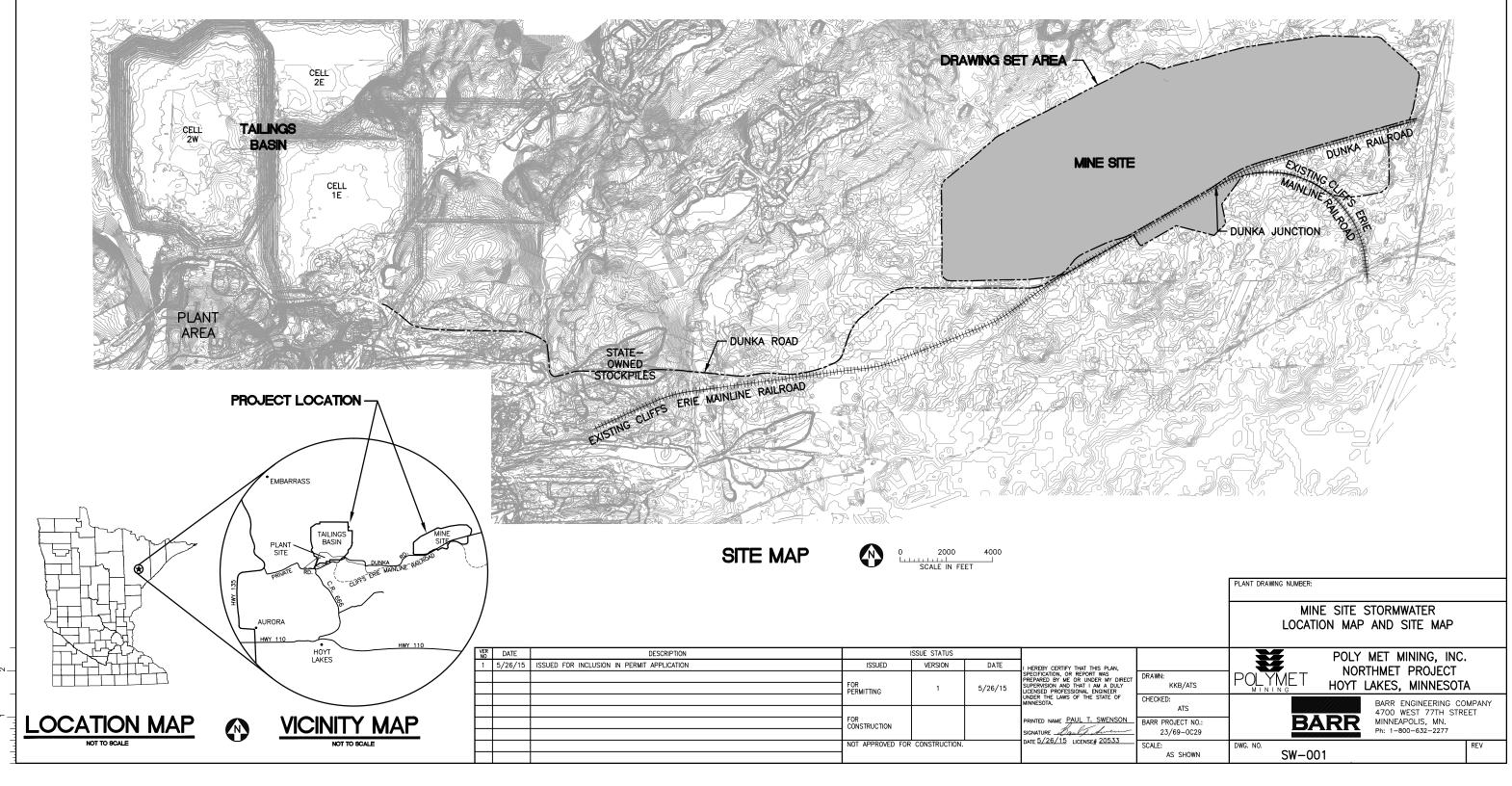
Permit Application Support Drawings: Mine Site Stormwater

May 2016

The table below lists changes that were identified during completion of the Construction Stormwater Pollution Prevention Plans (SWPPPs) and have not yet been incorporated in the attached permit application support drawings within this set. These changes and additional details developed during final design will be incorporated into the final design drawing set.

Drawing Sheet(s)	Change
Global change to all sheets, as needed	The terminology "mine drainage" as noted in these drawings has been changed to "mine water".
SW-003, SW-004, SW-005, SW-006, SW-031	A stormwater infiltration basin was added between the OSP & RTH to treat stormwater from an undisturbed area. This basin is shown in the Waste Water Treatment Facility drawings.
SW-003	Construction of the stormwater Ditch C West southeast of the Category 1 stockpile will begin during operations in Year 1. Therefore, it is not included in the Construction SWPPP during Year 0.
SW-003, SW-004, SW-005, SW-006, SW-031	Six stormwater infiltration basins and associated culverts were added near the WWTF and Dunka Road to meet the SWPPP requirements.

POLY MET MINING, INC. NORTHMET PROJECT PERMIT APPLICATION SUPPORT DRAWINGS MINE SITE STORMWATER HOYT LAKES, MINNESOTA



---1000----

----- WATER EDGE/CREEK CENTER LINE

EXISTING ROAD

----R/W---- RIGHT OF WAY

---- MINE SITE BOUNDARY

EXISTING STRUCTURES

- PROPERTY LINE

WETLAND BOUNDARY

--- OE --- EXISTING OVERHEAD ELECTRIC

--- UE --- EXISTING UNDERGROUND ELECTRIC

> EXISTING CULVERT

PROPOSED MINE DRAINAGE CULVERT

PROPOSED CONTOUR - MAJOR PROPOSED CONTOUR - MINOR

PROPOSED ACCESS ROADS

PROPOSED STORMWATER DRAIN

SURFACE DRAINAGE

NOTES

COORDINATE SYSTEM IS MINNESOTA STATE PLANE NORTH ZONE, NAD83.

2. ELEVATIONS ARE BASED ON MEAN SEA LEVEL (MSL), NAVD88.

3. EXISTING TOPOGRAPHIC INFORMATION SHOWN ON THE DRAWINGS WAS PREPARED BY AEROMETRIC, INC. FROM LIDAR DATA COLLECTED ON

4. CULVERT DIMENSIONS ARE PRELIMINARY. FINAL DIMENSIONS SHALL BE DETERMINED DURING FINAL DESIGN.

5. THE BEDROCK PROFILES SHOWN ON THESE DRAWINGS REPRESENT THE BEST AVAILABLE INFORMATION FOR PLANNING PURPOSES. THE BEDROCK SURFACE FROM WHICH THE PROFILES ARE EXTRACTED IS A THREE—DIMENSIONAL, MODELED SURFACE THAT RESULTED FROM DEDUCTING THE DEPTH TO BEDROCK IDENTIFIED ON LOGS OF BORINGS CONDUCTED AT THE MINE SITE FROM THE LIDAR TOPOGRAPHIC GROUND SURFACE MODEL. THE RESULTING DATA WAS THEN MODELED IN GIS SOFTWARE TO DEVELOP A THREE—DIMENSIONAL BEDROCK SURFACE. THE PROFILES SHOW SIGNIFICANT DETAIL IN LOCAL ELEVATIONS, WHICH MAY OR MAY NOT ACTUALLY EXIST. THE BEDROCK SURFACE PROFILES SHOULD BE TAKEN AS REPRESENTATIVE, BUT NOT NECESSARILY PRECISE.

ABBREVIATIONS

CATEGORY 1 WASTE ROCK STOCKPILE
CATEGORY 2/3 WASTE ROCK STOCKPILE
CENTERLINE
CENTRAL PUMPING STATION
THE MEDIAN PARTICLE DIAMETER OF A PARTICLESIZE
DISTRIBUTION; THE SIZE AT WHICH 50% OF THE
PARTICLES IN THE MATERIAL PARTICLE SIZE
DISTRIBUTION CURVE ARE SMALLER
ELEVATION
INVERT CATEGORY 1 STOCKPILE CATEGORY 2/3 STOCKPILE

CL CPS D50

III, IV, V

EL
INV
I, I, I
kV
LF
MP
NWL
RCP
O.C.
OSLA
PVI
SWPPP
TWP INVERI
ROMAN NUMERALS FOR RIPRAP CLASSIFICATION
KILOVOLT
LINEAR FEET
MINNESOTA POWER
NORMAL WATER LEVEL
REINFORCED CONCRETE PIPE
ON CENTER
OVERBURDEN STORAGE AND LANDOWN AREA

ON CENTER
OVERBURDEN STORAGE AND LAYDOWN AREA
POINT OF VERTICAL
STORMWAITER POLLUTION PREVENTION PLAN
TREATED WATER PIPELINE
WASTE WATER TREATMENT FACILITY

WWTF

(E) (W) EAST

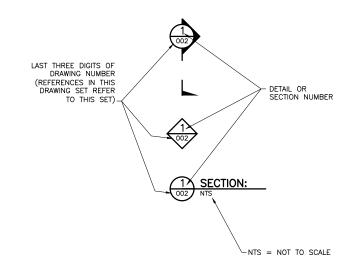
SHEET INDEX

SHEET NO. TITLE

GENERAL DRAWINGS

SW-001 STORMWATER LOCATION MAP AND SITE MAP	
SW-002 STORMWATER LEGEND AND SHEET INDEX	
SW-003 STORMWATER SITE DRAINAGE PLAN MINE YEAR 1	
SW-004 STORMWATER SITE DRAINAGE PLAN MINE YEAR 11	
SW-005 STORMWATER SITE DRAINAGE PLAN MINE YEAR 20	
SW-006 STORMWATER SHEET LOCATION MAP	
SW-007 STORMWATER TYPICAL DIKES AND DITCHES CROSS SECTIONS	
SW-008 STORMWATER SEDIMENTATION POND A GRADING PLAN AND DETAILS	
SW-009 STORMWATER SEDIMENTATION POND B GRADING PLAN AND DETAILS	
SW-010 STORMWATER SEDIMENTATION POND C-EAST GRADING PLAN AND DETAILS	
SW-011 STORMWATER SEDIMENTATION POND C-WEST GRADING PLAN AND DETAILS	
SW-012 STORMWATER SEDIMENTATION POND D GRADING PLAN AND DETAILS	
SW-013 STORMWATER NORTH DIKE AND DITCH PLAN AND PROFILE STATION 10+00N - 38+50N	
SW-014 STORMWATER NORTH DIKE AND DITCH PLAN AND PROFILE STATION 38+50N - 66+50N	
SW-015 STORMWATER NORTH DIKE AND DITCH PLAN AND PROFILE STATION 66+50N - 94+00N	
SW-016 STORMWATER NORTH DIKE AND DITCH PLAN AND PROFILE STATION 94+00N - 122+00N	
SW-017 STORMWATER NORTH DIKE AND DITCH PLAN AND PROFILE STATION 122+00N - 144+00I	
SW-018 STORMWATER NORTH DIKE AND DITCH PLAN AND PROFILE STATION 146+00N - 162+00I	
SW-019 STORMWATER NORTH DIKE AND DITCH PLAN AND PROFILE STATION 162+00N - 183+00I	4
SW-020 STORMWATER SOUTH DIKE AND DITCH PLAN AND PROFILE STATION 10+00S - 22+95S	
SW-021 STORMWATER DITCH B PLAN AND PROFILE STATION 0+00B - 17+50B	
SW-022 STORMWATER DITCH B PLAN AND PROFILE STATION 17+50B - 35+00B	
SW-023 STORMWATER DITCH C(E) PLAN AND PROFILE STATION 0+00C(E) - 19+64C(E)	
SW-024 STORMWATER DITCH C(W) PLAN AND PROFILE STATION 0+00C(W) - 18+00C(W)	
SW-025 STORMWATER DITCH C(W) PLAN AND PROFILE STATION 18+00C(W) - 34+50C(W)	
SW-026 STORMWATER DITCH C(W) PLAN AND PROFILE STATION 35+00C(W) - 55+00C(W)	
SW-027 STORMWATER DITCH C(W) PLAN AND PROFILE STATION 55+00C(W) - 75+00C(W)	
SW-028 STORMWATER DITCH C(W) PLAN AND PROFILE STATION 75+00C(W) - 94+60C(W)	
SW-029 STORMWATER DITCH C(W) PLAN AND PROFILE STATION 95+00C(W) - 124+35C(W)	
SW-030 STORMWATER DITCH D PLAN AND PROFILE STATION 0+00D - 27+00D	
SW-031 STORMWATER DIKES, DITCHES, AND PONDS CLOSURE PLAN	

DRAWING NUMBERING



MINE SITE STORMWATER

PLANT DRAWING NUMBER:

POLYME

LEGEND AND SHEET INDEX

)	DATE	DESCRIPTION	ISSUE STATUS				i
	5/26/15	ISSUED FOR INCLUSION IN PERMIT APPLICATION	ISSUED	VERSION	DATE	I HEREBY CERTIFY THAT THIS PLAN.	
			FOR PERMITTING	1		SPECIFICATION, OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER	DRAWN: KKB/ATS
						UNDER THE LAWS OF THE STATE OF MINNESOTA.	CHECKED: ATS
			FOR CONSTRUCTION			PRINTED NAME PAUL T. SWENSON SIGNATURE Baulf Sum	BARR PROJECT NO.: 23/69-0C29
			NOT APPROVED FOR	CONSTRUCTION.		DATE 5/26/15 LICENSE# 20533	SCALE:
							AS SHOWN

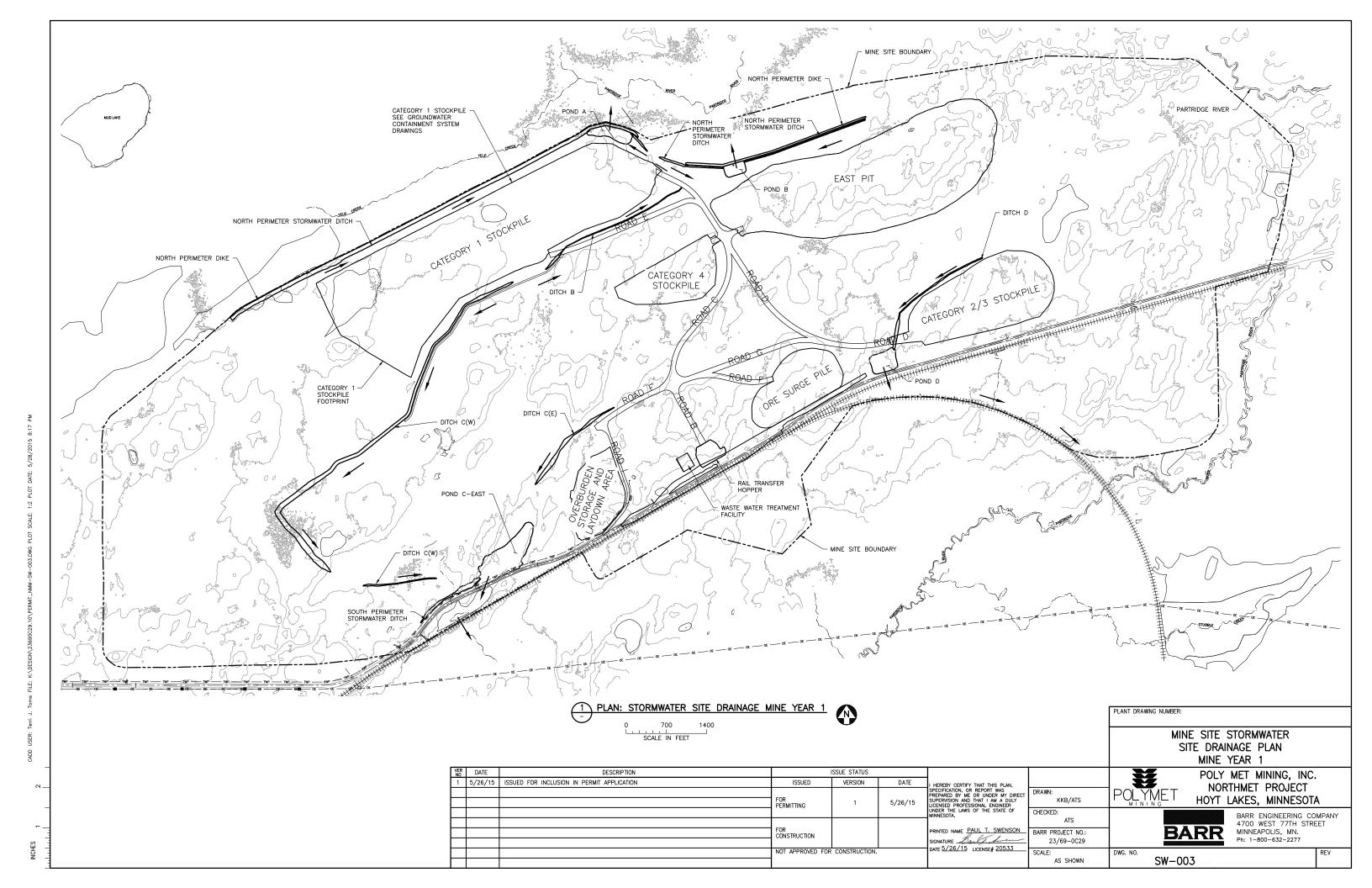
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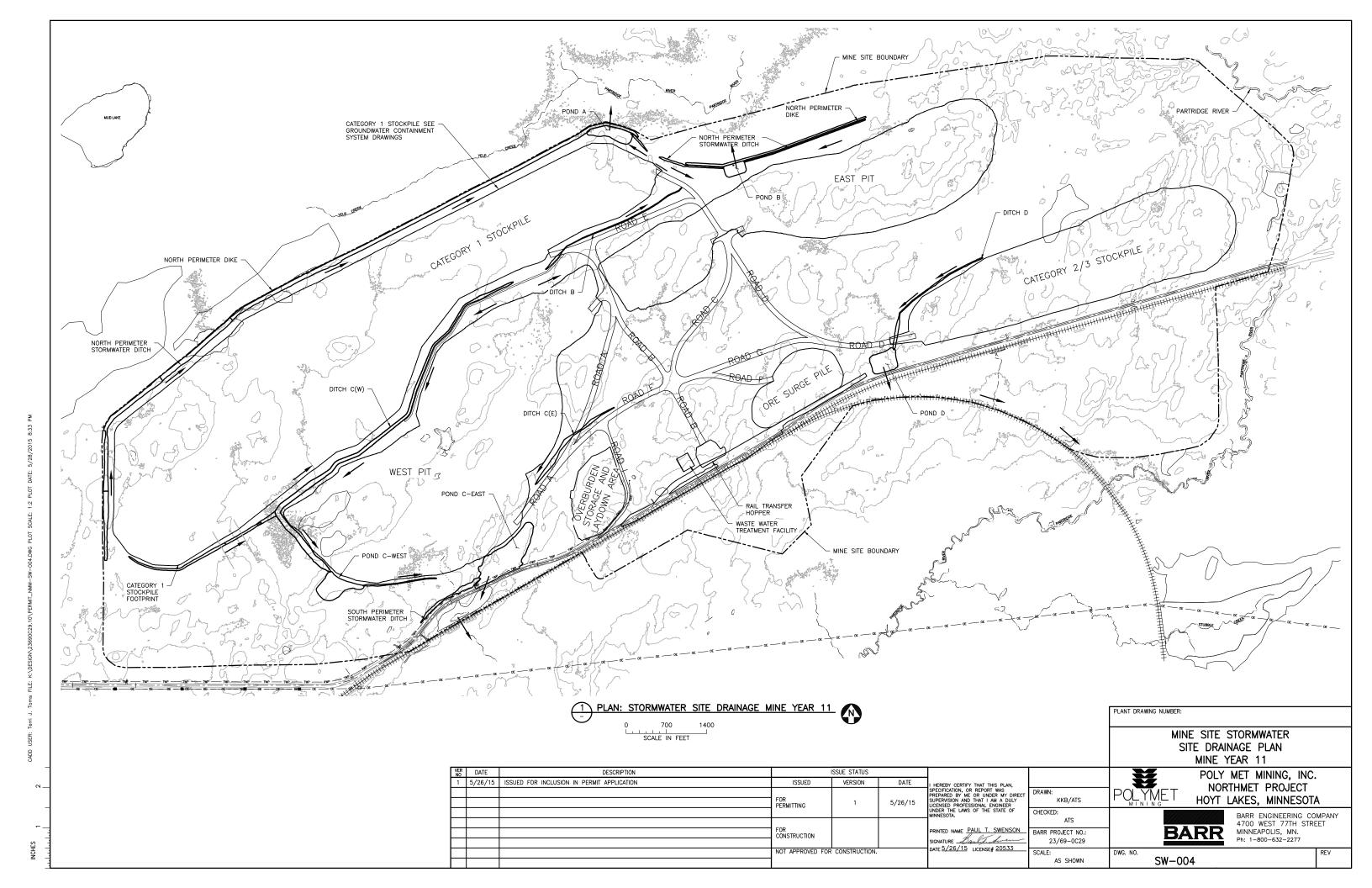
POLY MET MINING, INC. NORTHMET PROJECT

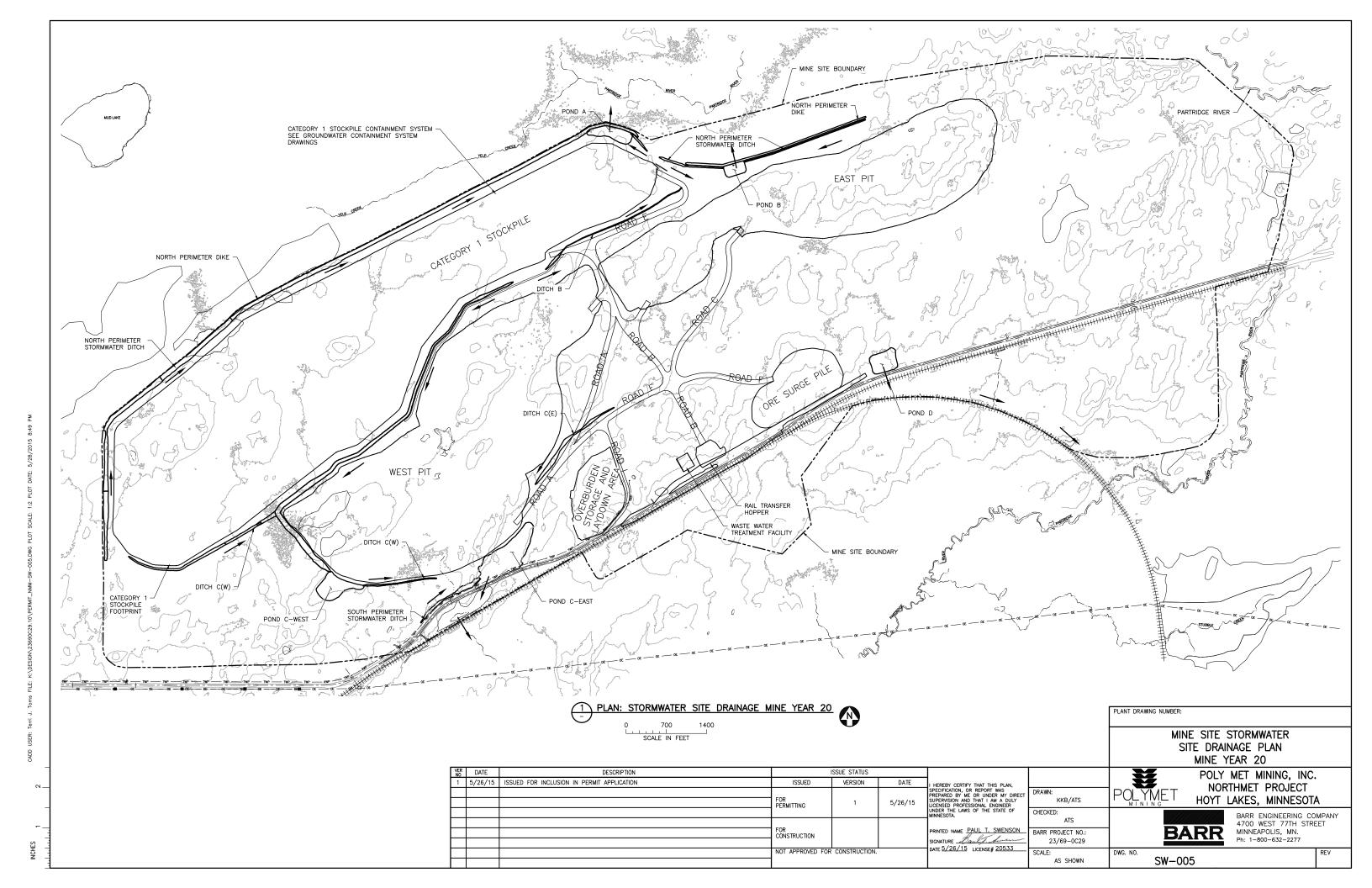
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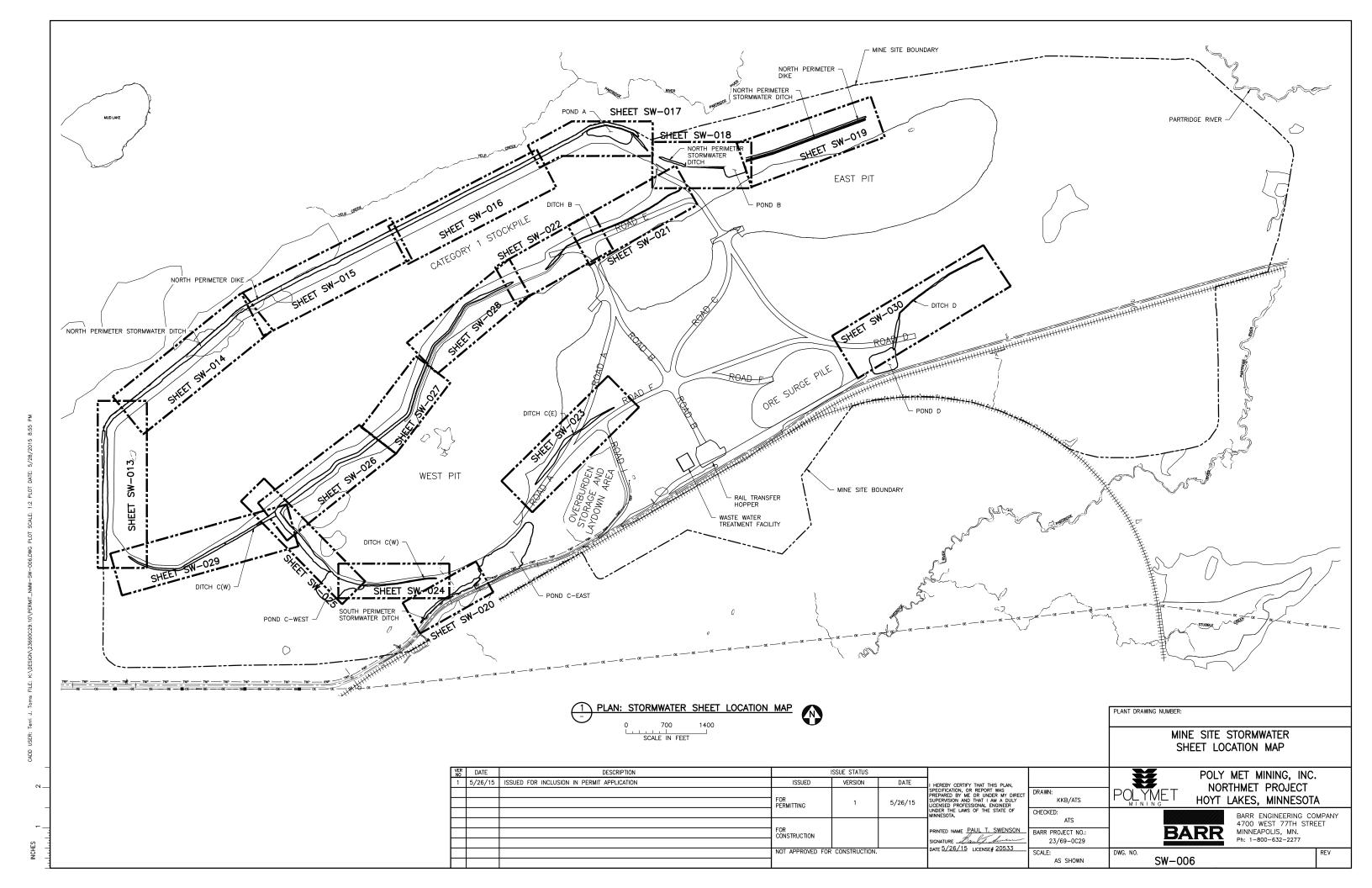
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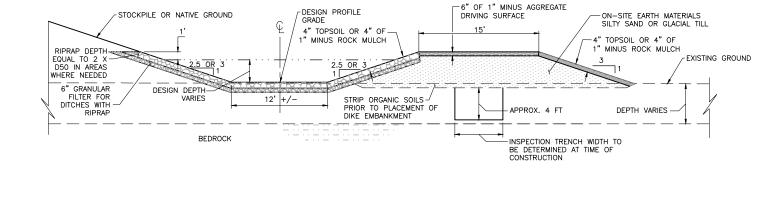
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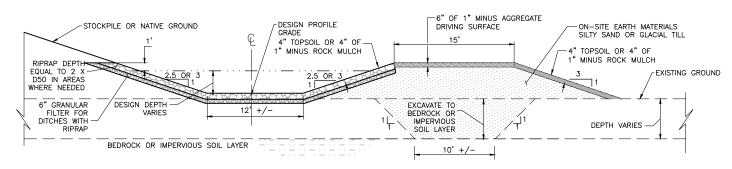






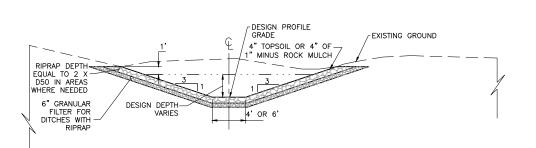






SECTION: TYPICAL PERIMETER DIKE AND DITCH WITHOUT SEEPAGE CUTOFF TRENCH





3 SECTION: TYPICAL INTERIOR DITCH

SCALE IN FEET

- 1. DITCHES NOT REQUIRING RIPRAP WILL BE CONSTRUCTED WITH NATIVE SOILS AND EITHER 4" TOPSOIL OR 4" OF 1" MINUS ROCK MULCH TO DESIGN PROFILE GRADE.
- 2. RESTORE ALL DISTURBED AREAS NOT STABILIZED WITH RIPRAP IN ACCORDANCE WITH THE SWPPP.
- 3. DESIGN DEPTH ESTABLISHED IN FINAL DESIGN OR IN FIELD. TOP OF DIKE TO FLOWLINE OF DITCH SHALL PROVIDE 1 FOOT OF FREEBOARD FROM DESIGN DEPTH.
- 4. INSPECTION TRENCH TO BE CONSTRUCTED ALONG ENTIRE LENGTH OF PERIMETER DIKES WHERE NON-ORGANIC
- 5. PERIMETER DITCH AND DIKE ALONG THE NORTH SIDE OF THE CATEGORY 1 STOCKPILE (STATION 10+00 TO 143+53) SHALL HAVE 2.5H:1V SIDE SLOPES. ALL OTHER DITCHES AND DIKES SHALL HAVE SIDE SLOPES OF 3H:1V.

MINE SITE STORMWATER TYPICAL DIKES AND DITCHES CROSS SECTIONS

BARR

SW-007

POLY MET MINING, INC. NORTHMET PROJECT

HOYT LAKES, MINNESOTA

MINNEAPOLIS, MN.

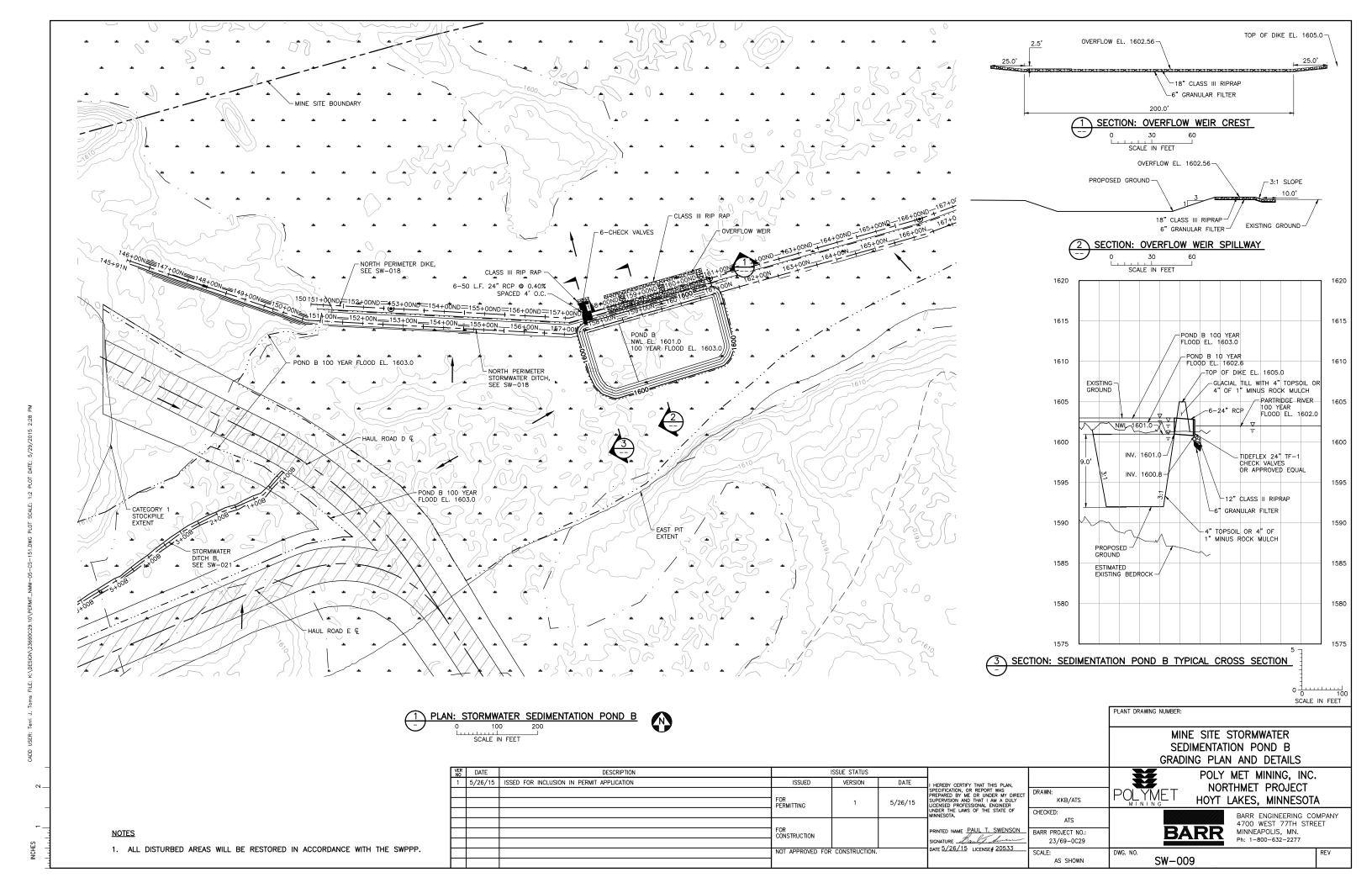
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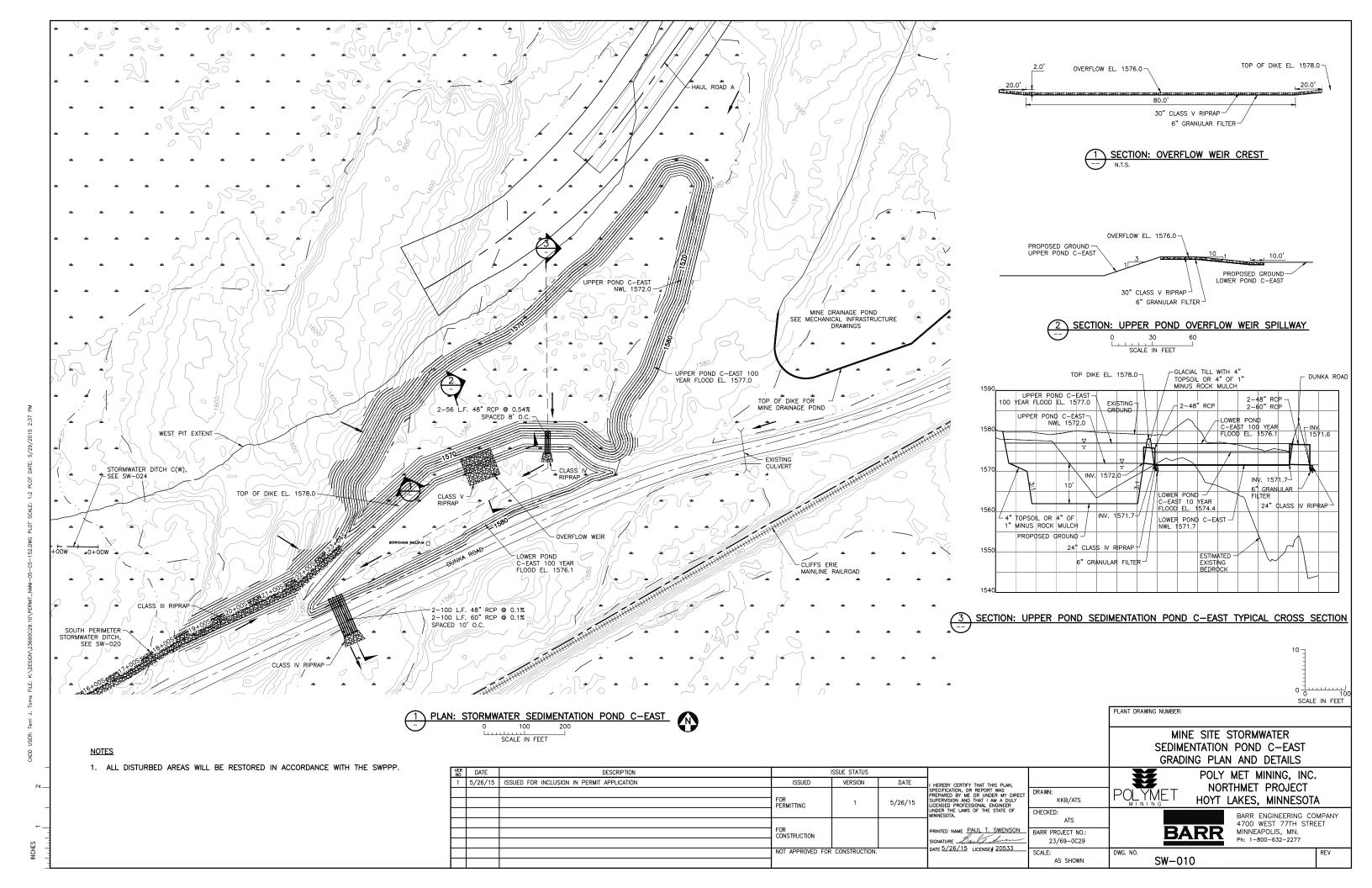
BARR ENGINEERING COMPANY 4700 WEST 77TH STREET

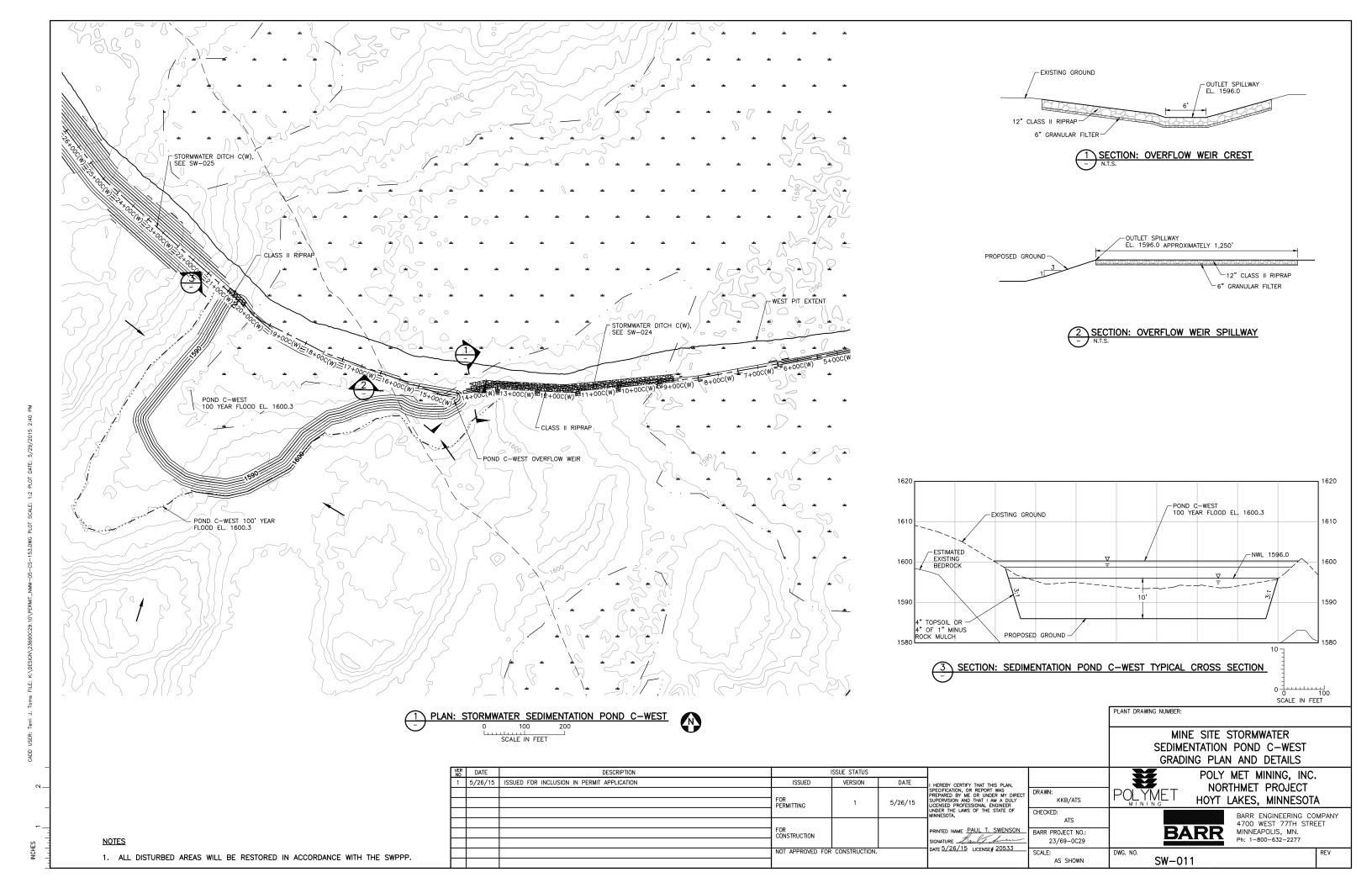
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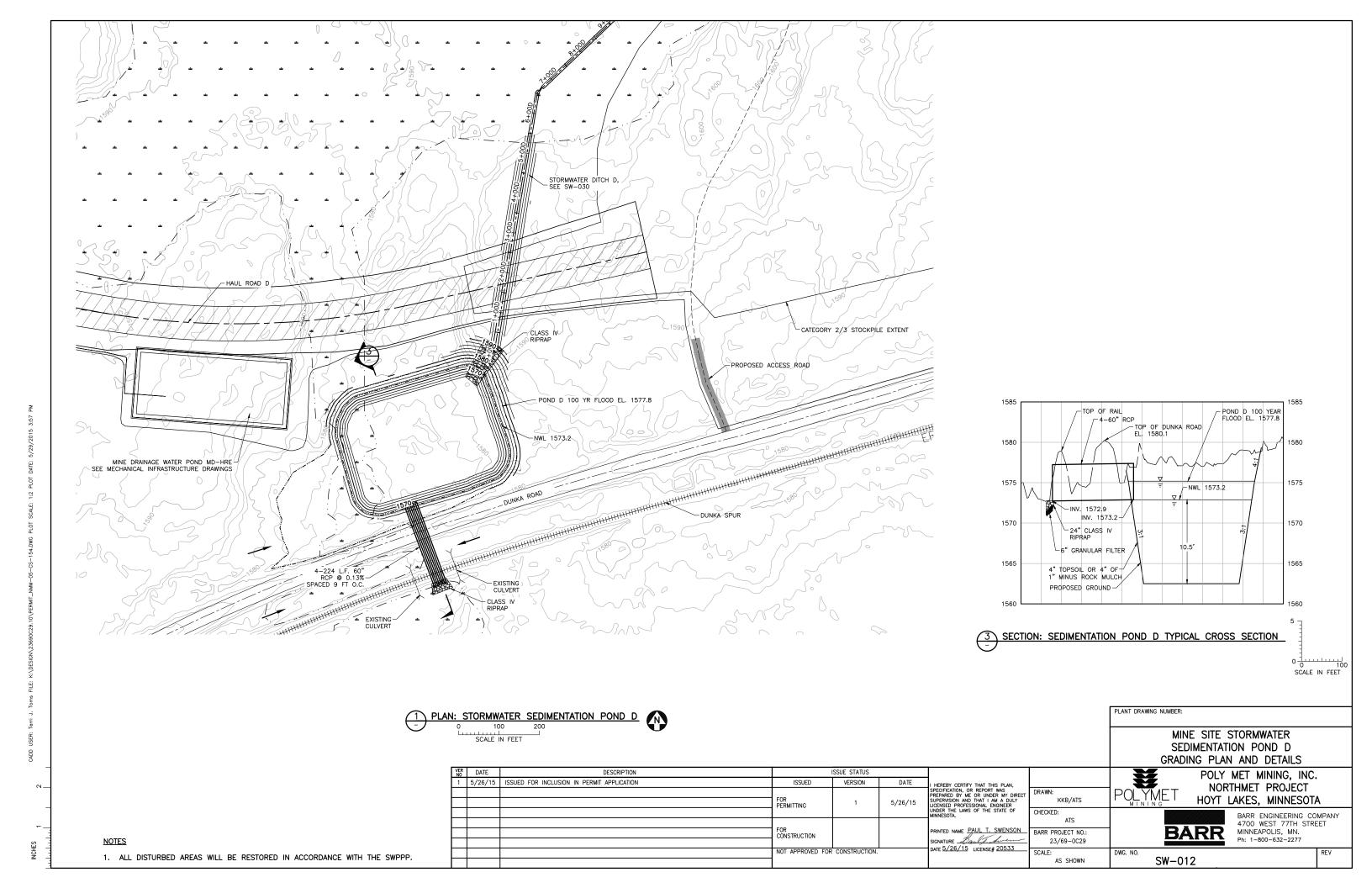
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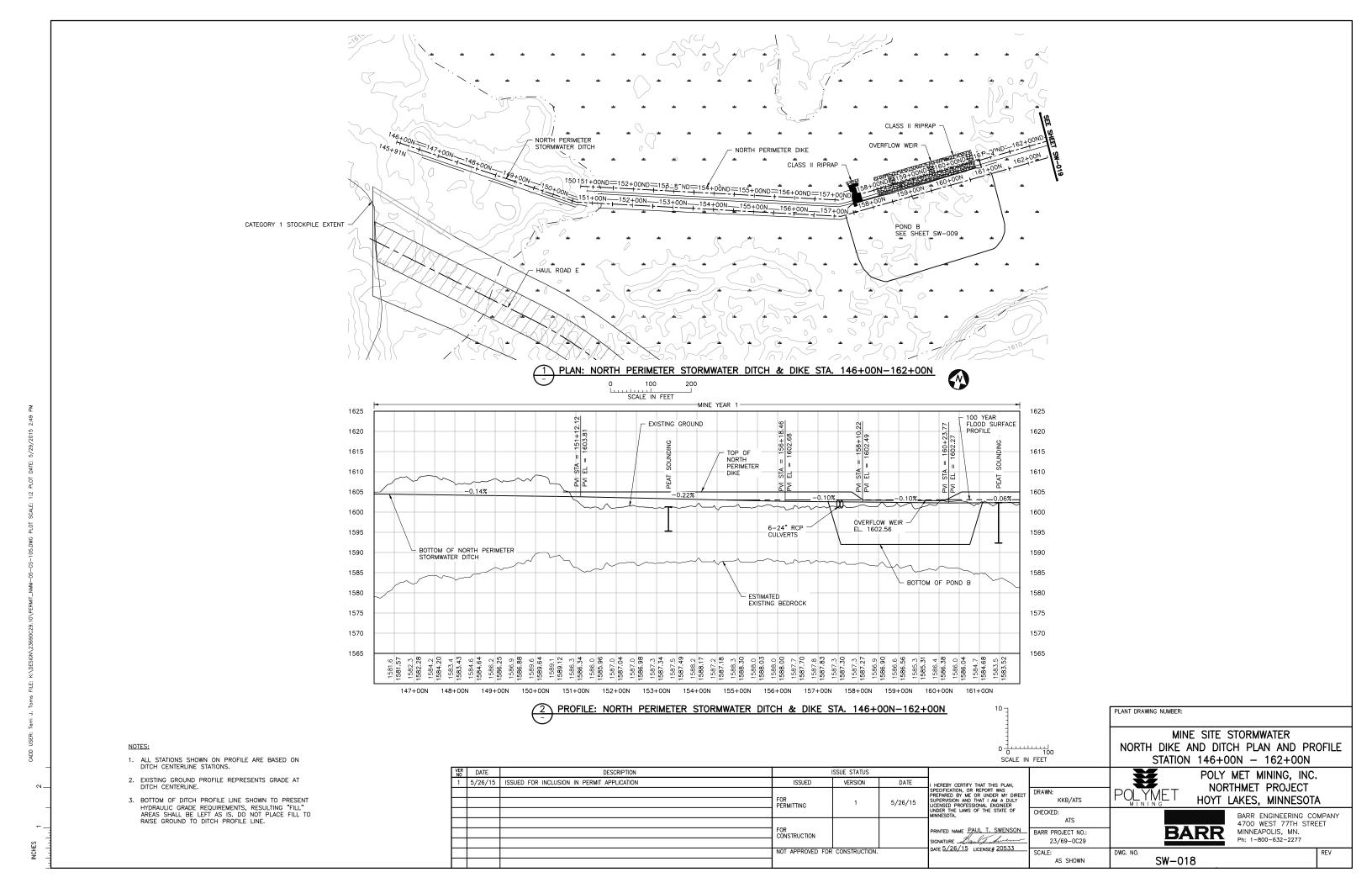
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			FOR PERMITTING	1	5/26/15	SPECIFICATION, OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER	DRAWN: KKB/ATS
			1 EKWITTING			UNDER THE LAWS OF THE STATE OF MINNESOTA.	CHECKED:
]				ATS
			FOR CONSTRUCTION			PRINTED NAME PAUL T. SWENSON	BARR PROJECT NO.:
						SIGNATURE Baulf Summer DATE 5/26/15 LICENSE# 20533	23/69-0029
			NOT APPROVED FOR CONSTRUCTION.			DATE 3/ 26/ 15 LICENSE# 20533	SCALE:
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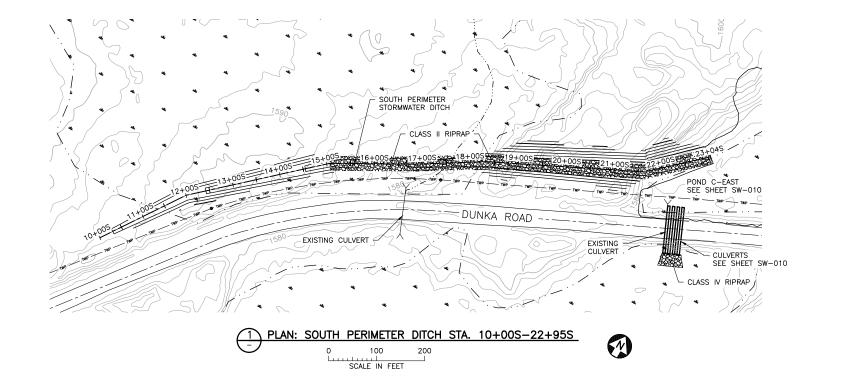


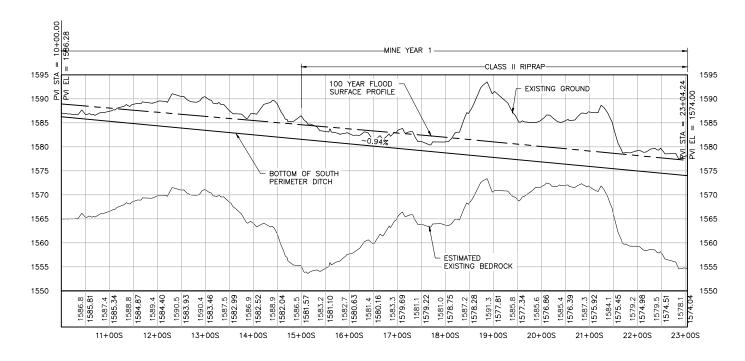












NOTES:

- ALL STATIONS SHOWN ON PROFILE ARE BASED ON DITCH CENTERLINE STATIONS.
- 2. EXISTING GROUND PROFILE REPRESENTS GRADE AT DITCH CENTERLINE.

PROFILE: SOUTH PERIMETER DITCH STA. 10+00S-22+95S

SCALE IN FEET

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Γ	VER D	DATE	DESCRIPTION	ISSUE STATUS				
Γ	1 5/	²⁶ /15	ISSUED FOR INCLUSION IN PERMIT APPLICATIONS	ISSUED	VERSION	DATE	I HEREBY CERTIFY THAT THIS PLAN.	
F				FOR PERMITTING	1	E /00 /1E	SPECIFICATION, OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER	DRAWN: KKB/ATS
Γ				1 EKIMITTING			UNDER THE LAWS OF THE STATE OF MINNESOTA.	CHECKED:
								ATS
				FOR CONSTRUCTION		l	PRINTED NAME PAUL T. SWENSON	BARR PROJECT NO.:
L							SIGNATURE Daul J. Surum DATE 5/26/15 LICENSE# 20533	23/69-0C29
				NOT APPROVED FOR	CONSTRUCTION.	INSTRUCTION.	DATE 3/20/13 LICENSE# 20333	SCALE:
Г								AS SHOWN

MINE SITE STORMWATER SOUTH DIKE AND DITCH PLAN AND PROFILE STATION 10+00S - 22+95S

POLYME1

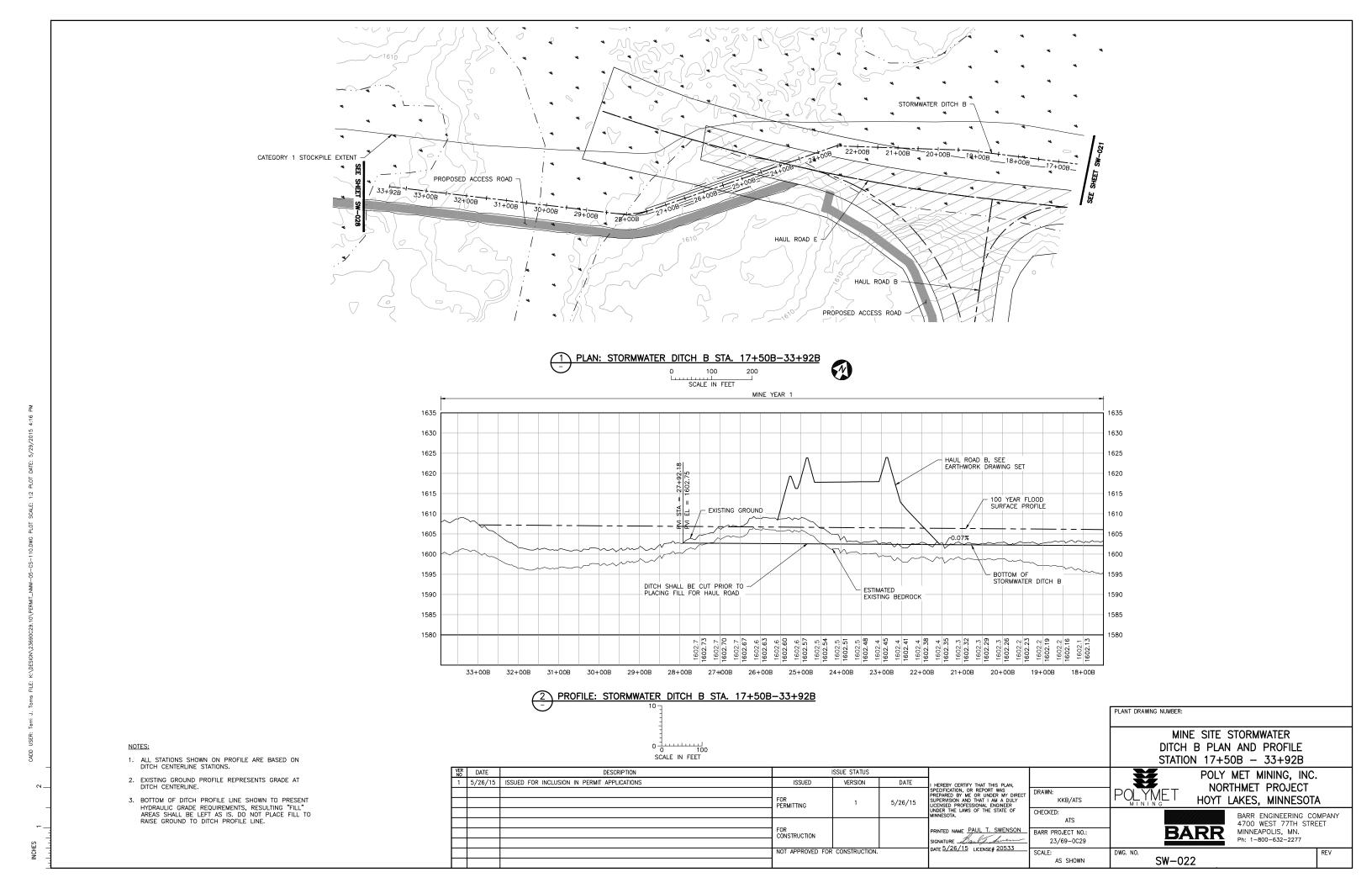
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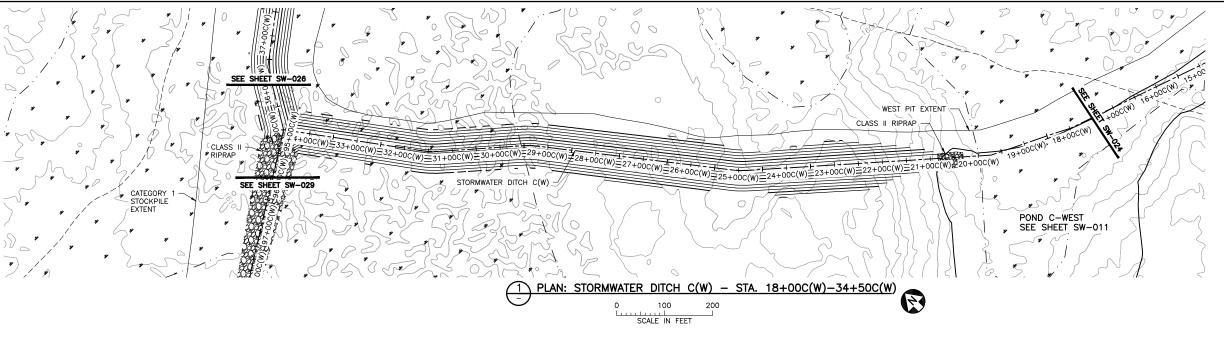
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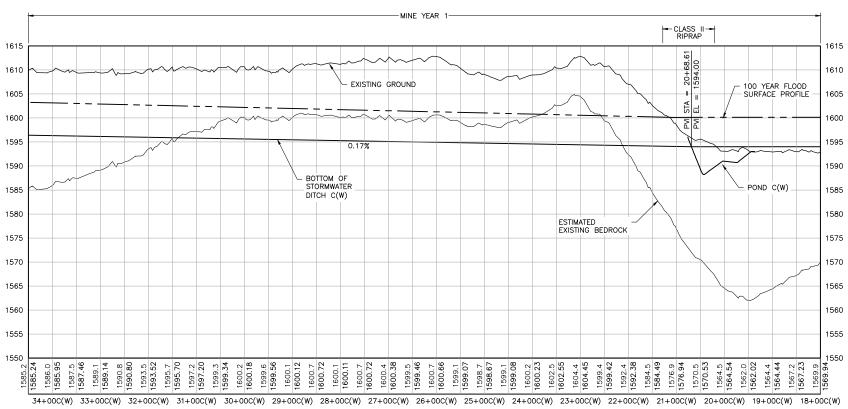
BARR

BARR ENGINEERING COMPANY 4700 WEST 77TH STREET MINNEAPOLIS, MN.

Ph: 1-800-632-2277 SW-020







PROFILE: STORMWATER DITCH C(W) - STA. 18+00C(W)-34+50C(W)

SCALE IN FEET

MINE SITE STORMWATER DITCH C(W) PLAN AND PROFILE STATION 18+00C(W) - 34+50C(W)

VER DATE DESCRIPTION ISSUE STATUS 5/26/15 ISSUED FOR INCLUSION IN PERMIT APPLICATION HEREBY CERTIFY THAT THIS PLAN, PECIFICATION, OR REPORT WAS REPARED BY ME OR UNDER MY DIRECT UPERVISION AND THAT I AM A DULY ICENSED PROFESSIONAL ENGINEER INDER THE LAWS OF THE STATE OF INNERSTATE. ISSUED VERSION DATE FOR PERMITTING 5/26/15 CHECKED: FOR CONSTRUCTION RINTED NAME PAUL T. SWENSON BARR PROJECT NO .: SIGNATURE Bault Swen DATE 5/26/15 LICENSE# 20533 NOT APPROVED FOR CONSTRUCTION.

POLY MET MINING, INC. NORTHMET PROJECT POLYME HOYT LAKES, MINNESOTA

PLANT DRAWING NUMBER:

KKB/ATS

23/69-0029

AS SHOWN

BARR

SW-025

BARR ENGINEERING COMPANY 4700 WEST 77TH STREET MINNEAPOLIS, MN. Ph: 1-800-632-2277

NOTES:

ALL STATIONS SHOWN ON PROFILE ARE BASED ON DITCH CENTERLINE STATIONS.

2. EXISTING GROUND PROFILE REPRESENTS GRADE AT DITCH CENTERLINE.

3. BOTTOM OF DITCH PROFILE LINE SHOWN TO PRESENT

HYDRAULIC GRADE REQUIREMENTS, RESULTING "FILL"
AREAS SHALL BE LEFT AS IS. DO NOT PLACE FILL TO
RAISE GROUND TO DITCH PROFILE LINE.

